GENERAL STRUCTURAL NOTES

01. NOTES AND DETAILS ON THE STRUCTURAL DRAWINGS TAKE PRECEDENCE OVER THESE STANDARD STRUCTURAL NOTES. TYPICAL DETAILS SHALL BE USED WHENEVER APPLICABLE. REFER TO SPECIFICATIONS FOR INFORMATION NOT COVERED WITHIN THESE NOTES OR DRAWINGS.

02. THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS, ELEVATIONS. AND SITE CONDITIONS BEFORE STARTING WORK, AND THE ENGINEER / ARCHITECT SHALL BE IMMEDIATELY NOTIFIED, IN WRITING, OF ANY DISCREPANCIES.

03. IN NO CASE SHALL DIMENSIONS BE SCALED FROM PLANS, SECTIONS, OR DETAILS ON THE STRUCTURAL DRAWINGS.

04. ALL OMISSIONS AND CONFLICTS BETWEEN THE VARIOUS ELEMENTS OF THE DRAWINGS AND / OR SPECIFICATIONS SHALL BE BROUGHT TO THE ATTENTION OF, AND RESOLVED WITH, THE

05. WHERE A CONSTRUCTION DETAIL IS NOT SHOWN OR NOTED, THE DETAIL SHALL BE THE SAME AS FOR OTHER SIMILAR WORK. ANY QUESTIONS OF SUCH DETAILS SHOULD BE CONSULTED WITH THE

ENGINEER OR ARCHITECT OF RECORD.

PERMISSION FROM THE ENGINEER OF RECORD

ENGINEER BEFORE PROCEEDING WITH ANY WORK SO INVOLVED.

06. THE CONTRACTOR SHALL DETERMINE THE LOCATION OF UTILITY SERVICES IN THE AREA TO BE EXCAVATED, BEFORE BEGINNING EXCAVATION

07. NO PIPES, DUCTS, SLEEVES, CHASES, ETC. SHALL BE PLACED IN SLABS, BEAMS, OR WALLS, NOR SHALL ANY STRUCTURAL MEMBER BE CUT FOR PIPES, DUCTS, ETC. WITHOUT EXPRESSED WRITTEN

08. ALL MATERIAL AND WORKMANSHIP SHALL CONFORM TO THE REQUIREMENTS OF THE 2010 CBC.

09. THE CONTRACTOR SHALL ASSUME SOLE AND COMPLETE RESPONSIBILITY FOR JOB SITE CONDITIONS DURING THE COURSE OF CONSTRUCTION OF THIS PROJECT, INCLUDING SAFETY OF ALL PERSONS AND PROPERTY. THIS REQUIREMENT SHALL APPLY CONTINUOUSLY AND NOT BE LIMITED TO NORMAL WORKING HOURS. THE CONTRACTOR SHALL DEFEND, INDEMNIFY, AND HOLD THE ENGINEER FREE AND HARMLESS OF ALL CLAIMS, DEMANDS AND / OR ALL LIABILITY. REAL OR ALLEGED, IN CONNECTION WITH THE PERFORMANCE OF WORK ON THIS PROJECT, EXCEPT FOR LIABILITY ARISING FROM THE NEGLIGENCE OF THE ENGINEER OF RECORD.

10. VERIFICATION OF A CALIFORNIA REGISTERED CIVIL ENGINEER IS NEEDED AND RETAINED TO DESIGN ALL TEMPORARY BRACING. SHORING, AND SUPPORT REQUIREMENTS DURING CONSTRUCTION.

11. THE CONTRACTOR / OWNER / DESIGNER MUST SUBMIT MATERIAL ALTERNATIVES TO THE ENGINEER OF RECORD PRIOR TO USE AND POSSIBLE PURCHASE TO AVOID UNNECESSARY COST TO CONTRACTOR / OWNER / DESIGNER. SUCH ENGINEERING REVIEW IS CONSIDERED SEPARATE ENGINEERING FEES UNDER ON-SITE ENGINEERING COSTS.

12. PLANS MUST BE APPROVED BY BUILDING DEPARTMENT PRIOR TO CONSTRUCTION.

FOUNDATION

01. THE SOILS REPORT RECOMMENDATIONS SHALL BE COMPLIED WITH BY THE CONTRACTOR.

02 SOILS INFORMATION: PREPARED BY: C.W. LA MONTE COMPANY, INC. DATED: AUGUST 2,2013 PROJECT NO: 13 6275 VERTICAL BEARING PRESSURE:

LATERAL BEARING PRESSURE:

ACTIVE PRESSUR

2000 PSF 35 PSF 350 PSF/F

NOTE: WHEN NO SOILS REPORT IS COMPLETED, FOOTINGS ARE DESIGNED ON A ASSUMED ALLOWABLE SOIL PRESSURE. IT IS STRONGLY RECOMMENDED THAT SOILS TESTING BY A LICENSED GEOTECHNICAL ENGINEER BE PERFORMED TO VERIFY SOIL BEARING CAPACITY AND STABILITY.

03. ALL SITE SOIL WORK SHALL BE DONE UNDER THE DIRECT OBSERVATION OF THE SOILS ENGINEER.

THAT ANY COMPACTION SITE SOIL WORK COMPLIES WITH ALL OF THE RECOMMENDATIONS AND CONCLUSIONS OF THE SOILS REPORT.

04. SOILS ENGINEER SHALL VERIFY IN WRITING TO THE ENGINEER

05. THE FINISH EXCAVATION FOR FOUNDATIONS SHALL BE NEAT & TRUE TO LINE.

06. FOUNDATION EXCAVATIONS SHALL BE KEPT FREE OF LOOSE MATERIAL AND STANDING WATER AND SHALL BE CHECKED AND APPROVED BY THE SOILS ENGINEER BEFORE THE PLACEMENT OF ANY CONCRETE.

07. THE TESTING LAB SHALL SUBMIT COMPACTION REPORTS FOR ALL FILL TO THE ENGINEER BEFORE REQUESTING FOUNDATION INSPECTION. ALL LOOSE SOIL AND FILL DIRT, INCLUDING BACKFILL BEHIND RETAINING WALLS, SHALL BE COMPACTED TO AT LEAST 90% OF MAXIMUM DENSITY, OR GREATER AS REQUIRED BY THE SOILS

08. ALL FOOTINGS MUST BEAR IN COMPETENT NATURAL GROUND.

CONCRETE MASONRY

01. CONCRETE MASONRY WALLS SHALL HAVE A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF f'm =1500 PSI.

02. CONCRETE MASONRY UNITS SHALL BE MEDIUM WEIGHT UNITS CONFORMING TO ASTM C 90, GRADE N-I, WITH MAX LINEAR SHRINKAGE OF 0.06 % (1900 PSI MINIMUM).

03. MORTAR SHALL BE TYPE "S", CONFORMING TO CBC TABLE 2103.8(1) (MINIMUM 1800 PSI AT 28 DAYS).

04. GROUT SHALL BE COMPOSED OF THE FOLLOWING RATIO BY VOLUME: 1-PART PORTLAND CEMENT, 3-PARTS SAND, 2-PARTS PEA GRAVEL, AND SUFFICIENT WATER FOR POURING WITHOUT SEGREGATION OF GROUT CONSTITUENTS (MIN. COMPRESSIVE STRENGTH OF 2000 PSI AT 28 DAYS).

05. ALL CELLS CONTAINING REINFORCING STEEL OR EMBEDDED ITEMS & ALL CELLS IN RETAINING WALLS AND WALLS BELOW GRADE SHALL BE SOLID GROUTED.

06. ALL HORIZONTAL REINFORCEMENT SHALL BE PLACED IN BOND

07. WHEN GROUTING IS STOPPED FOR ONE HOUR OR LONGER, HORIZ. CONSTRUCTION JOINTS SHALL BE FORMED BY STOPPING THE GROUT POUR 1-1/2" BELOW TOP OF THE UPPERMOST UNIT.

08. ALL BOND BEAM BLOCK SHALL BE "DEEP CUT" UNITS.

BEAM OR LINTEL BEAM UNITS.

09. PROVIDE INSPECTION AND CLEANOUT HOLES AT BASE OF GROUT LIFT FOR VERTICAL CELLS HAVING GROUT LIFTS IN EXCESS OF 5'-O"

10. ALL GROUT SHALL BE CONSOLIDATED WITH A MECHANICAL

VIBRATOR. 11. ANCHOR BOLTS MUST BE SET WITH TEMPLATES AND HELD IN PLACE PRIOR TO GROUTING. PROVIDE AT LEAST ONE INCH OF GROUT BETWEEN ANCHOR BOLT AND MASONBY

FASTENING SCHEDULE (U.O.N.)

DIAPHRAGM SHEATHING NAILS OR OTHER APPROVED SHEATHING CONNECTORS SHALL BE DRIVEN SO THAT THEIR HEAD OR CROWN IS FLUSH WITH THE SURFACE OF THE SHEATHING.

NAILING a,m

16d AT 24" O.C

2-16d FACE NAIL

3-16d

10d^dOR 8d^e

NO. 11 GAGE ROOFING NAIL

NO. 11 GAGE ROOFING NAIL IT

NO. 16 GAGE STAPLE

NO. 16 GAGE STAPLE

20d AT 32" O.C. FACE NAIL AT TOP AND

TABLE 2304.9.1 (CBC 2010)

JOISTS TO SILL OR GIRDER. TOENAIL BRIDGING TO JOIST, TOENAIL EACH END 3. 1" x 6" SUBFLOOR OR LESS TO EACH JOIST, FACE NAI . WIDER THAN 1" x 6" SUBFLOOR TO EACH JOIST, FACE NAIL 2-16d 2" SUBFLOOR TO JOIST OR GIRDER, BLIND AND FACE NAIL 6d AT 16" O.C. SOLE PLATE TO JOIST OR BLOCKING, TYPICAL FACE NAIL SOLE PLATE TO JOIST OR BLOCKING, AT BRACED WALL PANELS 3-16d PER 16 7. TOP PLATE TO STUD, END NAIL 4-8d. TOE NAIL OR 8. STUD TO SOLE PLATE 9. DOUBLE STUDS, FACE NAIL 10. DOUBLED TOP PLATES, TYPICAL FACE NAIL DOUBLE TOP PLATES, LAP SPLICE 2-16d AT 16" O.C 8-16d 1. BLOCKING BETWEEN JOISTS OR RAFTERS TO TOP PLATE, TOENAIL 2. RIM JOIST TO TOP PLATE, TOENAIL 13. TOP PLATES, LAPS AND INTERSECTIONS, FACE NAIL 2-16d 16d @ 16" ALONG EACH EDGE 14. CONTINUOUS HEADER, TWO PIECES

5. CEILING JOISTS TO PLATE, TOENAIL . CONTINUOUS HEADER TO STUD, TOENAIL CEILING JOISTS, LAP OVER PARTITIONS, FACE NAI B. CEILING JOISTS TO PARALLEL RAFTERS, FACE NAIL 9. RAFTER TO PLATE, TOENAIL 0. 1" DIAGONAL BRACE TO EACH STUD AND PLATE, FACE NAIL 1 1" x 8" SHEATHING OR LESS TO EACH BEARING FACE NAIL

22. WIDER THAN 1" x 8" SHEATHING TO EACH BEARING, FACE NAIL 23. BUILT-UP CORNER STUDS 24. BUILT-UP GIRDER AND BEAMS

BOTTOM STAGGERED ON OPPOSITE SIDES 2-20d FACE NAIL AT ENDS AND AT EACH SPLICE 2-16d AT FACH BEARING 26. COLLAR TIE TO RAFTER, FACE NAIL 3-10d TOE NAIL OR 27. JACK RAFTER TO HIP 28. ROOF RAFTER TO 2x RIDGE BEAM, TOE OR FACE NAIL

29. JOIST TO BAND JOIST, FACE NAIL 30. LEDGER STRIP, FACE NAIL 1. WOOD STRUCTURAL PANELS AND PARTICLEBOARD. SUBFLOOR, ROOF AND WALL SHEATHING (TO FRAMING) 1/2" AND LESS

8d^dOR 6d^e 19/32"-3/4" 1 1/8"-1 1/4" 10d^dOR 8d^e SINGLE FLOOR (COMBINATION SUBFLOOR-UNDERLAYMENT TO FRAMING): 3/4" AND LESS

1 1/8"-1 1/4" 2. PANEL SIDING (TO FRAMING) 1/2" OR LESS 3. FIBERBOARD SHEATHING: 9

25/32"

34. INTERIOR PANELING

on center at edges, 6 inches at intermediate supports for roof sheathing.

p. Fasteners spaced 4 inches on center at edges, 8 inches at intermediate supports.

For SI: 1 inch = 25.4 mm

a. Use common nails except where otherwise stated. b. Nails spaced at 6 inches on center at edges, 12 inches at intermediate supports except 6 inches at supports where spans are 48 inches or more. For nailing of wood structural panel and particleboard diaphragms and shear walls, refer to Section 230 Nails for wall sheathing are permitted to be common, box or casing.

c. Common or deformed shank

e. Deformed shank. f. Corrosion-resistant siding or casing nail. g. Fasteners spaced 3 inches on center at exterior edges and 6 inches on center at intermediate supports.

h. Corrosion-resistant roofing nails with 7/16-inch-diameter head and 11/2-inch length for 1/2-inch sheathing and 1 3/4-inch length for 25/32-inch sheathing. sistant staples with nominal 7/16-inch crown and 1 1/8-inch length for 1/2-inch sheathing and 1 1/2-inch length fo 25/32-inch sheathing. Panel supports at 16 inches (20 inches if strength axis in the long direction of the panel, unless otherwise

j. Casing or finish nails spaced 6 inches on panel edges, 12 inches at intermediate supports. k. Panel supports at 24 inches. Casing or finish nails spaced 6 inches on panel edges, 12 inches at intermediate supports.

I. For roof sheathing applications, 8d nails are the minimum required for wood structural panels. m. Staples shall have a minimum crown width of 7/16 inch. n. For roof sheathing applications, fasteners spaced 4 inches on center at edges, 8 inches at intermediate supports.

SILL BOLTING REQUIREMENTS

02. EMBEDDED A MINIMUM OF 7" INTO FIRST POUR ON TWO POUR SYSTEMS.

03. BOLTS AT CONCRETE CURBS TO BE EMBEDDED 7" MINIMUM INTO POUR

06. MINIMUM TWO ANCHOR BOLTS PER SILL PLATE SECTION WITH ONE BOLT

LOCATED NO MORE THAN 12" OR LESS THAN 4" FROM EACH END OF EACH PIECE.

04. 3" X 3" X 0.229" PLATE WASHERS TO BE USED ALL ANCHOR BOLTS.

01. BOLTS TO BE A 307, 5/8" Ø X 10".

05. MAXIMUM ANCHOR BOLTS SPACING 48" OC.

EXCLUDING CURB.

o. Fasteners spaced 4 inches on center at edges, 8 inches at intermediate supports for subfloor and wall sheathing and 3 inches

2. ALL SPECIFIED FASTENERS MUST BE INSTALLED ACCORDING TO THE INSTRUCTIONS OF THE MOST RECENT EDITION OF "WOOD STRONG-TIE, INC. INCORRECT FASTENER QUANTITY, SIZE, TYPE, MATERIAL, OR FINISH MAY CAUSE THE CONNECTION TO FAIL. 16d FASTENERS ARE COMMON NAILS (8GA x 3 1/2") AND CANNOT BE REPLACED WITH 16d SINKERS (9GA x 3 1/4") FOR FULL LOAD VALUE

3. ALL CONNECTORS TO BE FULLY NAILED OR BOLTED PER

THAN A MAXIMUM OF 1/16" LARGER THAN THE BOLT DIAMETER (PER THE 2005 NDS 11.1.2).

5. INSTALL ALL SPECIFIED FASTENERS BEFORE LOADING THE CONNECTION.

7. FOR CONNECTORS AND FASTENERS IN PRESERVATIVE-TREATED AND FIRE-RETARDED WOOD; USE HOT DIPPED ZINC-COATED GALVANIZED STEEL, STAINLESS STEEL, SILICON BRONZE, OR COPPER CONNECTIONS.

B. WELDING GALVANIZED STEEL MAY PRODUCE HARMFUL FUMES; FOLLOW PROPER WELDING PROCEDURES AND SAFETY PRECAUTIONS. WELDING SHOULD BE IN ACCORDANCE WITH A.W.S. STANDARDS. UNLESS OTHERWISE NOTED, SIMPSON CONNECTORS CANNOT BE WELDED.

9. PNEUMATIC NAIL TOOLS MAY BE USED TO INSTALL CONNECTORS, PROVIDED THE CORRECT QUANTITY AND TYPE OF NAILS ARE PROPERLY INSTALLED IN THE NAIL HOLES. TOOLS WITH NAIL HOLE-LOCATING MECHANISMS SHOULD BE USED. FOLLOW THE MANUFACTURER'S INSTRUCTIONS AND USE THE APPROPRIATE SAFETY EQUIPMENT. AS OVER DRIVING NAILS MAY REDUCE ALLOWABLE LOADS, OVER DRIVING MUST BE AVOIDED.

10. JOIST SHALL BEAR COMPLETELY ON THE CONNECTOR SEAT, AND THE GAP BETWEEN THE JOIST END AND THE HEADER SHALL NOT EXCEED 1/8" PER ASTM D1761 TEST STANDARDS.

11. FOR HOLDOWNS, ANCHOR BOLT NUTS SHOULD BE FINGER-TIGHT PLUS 1/3 TO 1/2 TURN WITH A WRENCH, WITH CONSIDERATION GIVEN TO POSSIBLE FUTURE WOOD SHRINKAGE. CARE SHOULD BE TAKEN TO NOT OVER-TORQUE THE NUT AS THIS MAY PRELOAD THE HOLDOWN.

STRUCTURAL WOOD

GRADE #2

GRADE #1

GRADE #1

GRADE #2

GRADE #2

03. PLYWOOD SHALL BE APA RATED SHEATHING AND SHALL

04. CUTTING, NOTCHING, OR DRILLING OF STUDS SHALL BE

AND/OR PER CBC SECTION 2308.9.10 OR 2308.9.11.

SHALL BE PRESSURE TREATED DOUGLAS FIR.

PERMITTED ONLY AS DETAILED OR APPROVED BY THE ENGINEER

05. CUTTING, NOTCHING, OR DRILLING OF BEAMS OR JOISTS SHALL

07. ALL SILLS OR PLATES RESTING ON CONCRETE OR MASONRY,

08. ALL BOLT HEADS AND NUTS BEARING ON WOOD SHALL HAVE

STANDARD CUT WASHERS. ALL BOLT HOLES IN WOOD SHALL BE

THE END AND 4 DIAMETERS FROM THE EDGE OF THE MEMBER.

ETC., SHALL BE AS MANUFACTURED BY "SIMPSON COMPANY" OR

11. PROVIDE DOUBLE JOISTS UNDER ALL PARALLEL PARTITIONS.

12. WHERE POST OCCURS ON 2nd FLOOR, ADD THE SAME POST ON FIRST FLOOR AND BETWEEN FLOOR SHEATHING AND 1st FLOOR TOP

13. TOP PLATES OF ALL WOOD STUD WALLS TO BE 2-2x MINIMUM

(SAME WIDTH AS STUDS), LAP 48" (MINIMUM), WITH NOT LESS THAN

6-16d NAILS AT EACH LAP AND NOT MORE THAN 12" BETWEEN NAILS.

14. MOISTURE CONTENT OF WOOD AT TIME OF PLACING SHALL NOT

PREFABRICATED WOOD

MANUFACTURED BY BOISE CASCADE AND MEET THE REQUIREMENTS

2. JOISTS AND JOIST HANGERS SHALL BE DESIGNED IN ACCORDANCE

ANY SPECIAL LOADS SUCH AS MECHANICAL EQUIPMENT LOADS, WIND

AND SEISMIC LOADS. DESIGN SHALL INCLUDE BRIDGING, BRACING, OR

REVIEWED BY THE ENGINEER AND BUILDING DEPARTMENT BEFORE

OR ANY EQUIVALENT WITH WRITTEN APPROVAL FROM THE PROJECT

OR ANY EQUIVALENT WITH WRITTEN APPROVAL FROM THE PROJECT

USE 2.0E - 2800 DF GRADE LVL JOISTS, U.N.O. WHERE SOLID TIMBER IS BEING USED AT EXTERIOR OF FLOOR SYSTEM, MINIMUM WIDTH OF

OR ANY EQUIVALENT WITH WRITTEN APPROVAL FROM THE PROJECT

6. THE CONDITIONS OF USE FOR MANUFACTURED LUMBER MUST BE

ALL MANUFACTURED LUMBER EXPOSED TO THE WEATHER MUST BE

TREATED FOR EXTERIOR APPLICATION AS RECOMMENDED BY THE

VERIFY AND PROVIDE REQUIRED TREATMENT PER MANUFACTURE.

7. DO NOT CUT, NOTCH, DRILL, BORE, SHAVE, TAPER OR FOR ANY

REASON MODIFY PRE-ENGINEERED/MANUFACTURED STRUCTURAL

PARAMETERS SET FORTH BY THE MANUFACTURER OF THAT PRODUCT

GLUE-LAMINATED TIMBERS

ELEMENTS SUCH AS GLUE-LAMINATED MEMBERS, PARALAMS,

UNLESS SUCH MODIFICATIONS ARE WITHIN THE WRITTEN

MICROLAMS, I-JOISTS, AND OTHER SIMILAR TIMBER PRODUCTS

OR A LETTER OF CERTIFICATION FROM THE MANUFACTURER'S

ENGINEER WITH A SIGNED AND STAMPED DETAIL IS ISSUED AND

1. MANUFACTURE OF GLUED-LAMINATED TIMBERS SHALL BE IN

ARCHITECTURAL APPEARANCE GRADE, USING EXTERIOR GLUE,

COMBINATION SYMBOL 24F-V4 FOR SIMPLE SPANS AND 24F-V8 FOR

CONTINUOUS SPAN AND/OR CANTILEVERED MEMBERS. MEMBERS

NOT EXPOSED TO VIEW MAY BE INDUSTRIAL APPEARANCE GRADE.

3. GLUED-LAMINATED TIMBERS SHALL BE FABRICATED IN A PLANT

WITH AN APPROVED QUALITY CONTROL SYSTEM, LICENSED BY THE

4. MANUFACTURER OF GLUED-LAMINATED TIMBERS SHALL STAMP

CERTIFICATE TO THE BUILDING INSPECTION DEPARTMENT AND

5. CONTRACTOR SHALL SUBMIT SHOP DRAWINGS FOR REVIEW

6. PRESSURE TREAT FABRICATED GLUED LAMINATED MEMBERS,

EXPOSED TO WEATHER, IN ACCORDANCE WITH AITC 109-98. THE

7. FOR STRUCTURAL GLUE LAMINATED TIMBER MEMBERS, AN AITC

CONFORMANCE ISSUED BY A CURRENT ICC APPROVED QUALITY

TREATMENT SHALL BE APPROVED BY THE SUPPLIER AND THE

CERTIFICATION OF CONFORMANCE OR A CERTIFICATE OF

CONTROL AGENCY, MUST BE SUBMITTED TO THE BUILDING

INSPECTION ORGANIZATION AND SHALL SUBMIT AN AITC INSPECTION

MEMBERS WITH A QUALITY MARK OF A QUALIFIED CENTRAL

AUTHORIZED BY THE PROJECT ENGINEER OF RECORD AND

APPROVED BY THE CITY OF SAN DIEGO BUILDING OFFICIAL.

2. GLUED-LAMINATED TIMBERS SHALL BE DOUGLAS FIR

CONFORMANCE WITH ANSI/AITC A190.1-2002.

ENGINEER BEFORE INSTALLATION.

ENGINEER OF RECORD.

BEFORE FABRICATION AND ERECTION.

INSPECTOR PRIOR TO INSTALLATION.

DRY WHEREAS THE MOISTURE CONTENT REMAINS BELOW 16%.

MANUFACTURER. IT IS THE CONTRACTORS RESPONSIBILITY TO

ICC ES ESR-1040

ICC ES ESR-1040

WITH THE LOAD AND DEFLECTION CRITERIA PROVIDED INCI LIDING

SHORING REQUIRED DURING AND AFTER CONSTRUCTION.

CALCULATIONS AND DRAWINGS SIGNED BY A CALIFORNIA

" I " JOISTS CAN BE FROM ANY OF THE FOLLOWING

4. VERSA-LAM "LVL" JOISTS CAN BE FROM THE FOLLOWING

5. VERSA-LAM, "LVL" BEAMS CAN BE FROM THE FOLLOWING

USE 2.0E - 3100 DF GRADE LVL BEAMS, U.N.O.

REGISTERED CIVIL ENGINEER SHALL BE SUBMITTED TO AND

PREFABRICATED WOOD JOISTS SHALL BE BCI JOISTS AS

DRILLED 1/32" DIAMETER LARGER THAN NOMINAL BOLT DIAMETERS.

09. BOLTS IN WOOD SHALL NOT BE LESS THAN 7 DIAMETERS FROM

10. ALL FRAMING ANCHORS, POST CAPS, BASES, HANGERS, STRAPS,

WHICH IS IN CONTACT WITH EARTH OR RESTING ON FOUNDATIONS,

BE PERMITTED ONLY AS DETAILED OR APPROVED BY THE ENGINEER

06. ALL NAILING SHALL CONFORM TO NAILING SCHEDULE, EXCEPT AS

CONFORM TO COMMERCIAL PRODUCTS STANDARD PS 1-09 WHERE

REQUIRED. NAIL WITH COMMON NAILS OR AS APPROVED IN WRITING

MATCH VERTICAL MEMBERS

8'-0" LONG: STANDARD GRADE

8'-1" TO 14'-0": GRADE #1

01. ALL WOOD MEMBERS SHALL BE DOUGLAS FIR (DF) OR LARCH 01. BAR REINFORCEMENT SHALL BE ASTM A615, GRADE 60 FOR #4 GRADE MARKED BY A RECOGNIZED GRADING AGENCY (WCLIB & AND LARGER AND GRADE 40 FOR #3 AND SMALLER.

02. WOOD GRADES

BEAMS:

HEADERS:

TOP PLATES

DECKING:

2 x 4 STUDS,

2 x 4 STUDS

A) FOR HORIZONTAL MEMBERS (U.N.O.)

B) FOR VERTICAL MEMBERS (U.N.O.)

2 x 6 & LARGER STUDS:

NOTED ON PLANS AND DETAILS.

ENGINEER APPROVED FOLIAL.

EXCEED 19 PERCENT.

OF ICC ES ESR-1336

FABRICATION.

MANUFACTURES: BOISE CASCADE

MANUFACTURES:

BOISE CASCADE

MANUFACTURES:

BOISE CASCADE

ENGINEER OF RECORD.

ENGINEER OF RECORD.

MEMBER TO BE 1-1/4" LVL.

ENGINEER OF RECORD.

LEDGERS AND NAILERS: GRADE #2

JOISTS & RAFTERS:

02. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A185 AND SHALL BE LAPPED 12 INCHES MINIMUM.

REINFORCING STEEL

03. MINIMUM LAP SPLICES OF REINFORCING BARS SHALL BE AS FOLLOWS: A) CONCRETE: CLASS B AS DEFINED IN ACI 318-(CURRENT EDITION) UNLESS OTHERWISE NOTED ON PLANS.

04. REINFORCING DETAILING, BENDING, AND PLACING SHALL BE IN ACCORDANCE WITH THE CONCRETE REINFORCING STEEL INSTITUTE "MANUAL OF STANDARD PRACTICE", LATEST EDITION.

05. REINFORCING STEEL SHALL BE PROVIDED WITH THE FOLLOWING AMOUNTS OF COVER FOR CAST-IN-PLACE CONCRETE: A) CONCRETE DEPOSITED AGAINST EARTH: 3" B) CONCRETE SURFACE (FORMED) EXPOSED TO EARTH OR

#6 THROUGH #18 BARS: 2"

#5 BAR AND SMALLER: 1-1/2" 06. ALL REINFORCING STEEL, ANCHOR BOLTS, DOWELS, AND INSERTS SHALL BE WELL SECURED IN POSITION WITH WIRE POSITIONERS BEFORE PLACING CONCRETE OR GROUT.

REINFORCED CONCRETE

1. ALL PHASES OF WORK PERTAINING TO THE CONCRETE CONSTRUCTION SHALL CONFORM TO THE "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE" (ACI 318 LATEST APPROVED EDITION) WITH MODIFICATIONS AS NOTED IN THE DRAWINGS AND SPECIFICATIONS

02. CEMENT SHALL CONFORM TO ASTM C150, TYPE II, W/C RATIO = 0.45. AGGREGATES FOR NORMAL WEIGHT CONCRETE SHALL CONFORM TO ASTMC33.

03. READY-MIX CONCRETE SHALL BE MIXED AND DELIVERED IN ACCORDANCE WITH ASTM C94.

04. CONCRETE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS (U.N.O.) AS FOLLOWS: FOOTINGS: 2500 PSI GRADE BEAMS: 2500 PSI

05. ADMIXTURES MAY BE USED WITH PRIOR APPROVAL OF THE **ENGINEER**

SLAB ON GRADE: 2500 PSI

06. ADMIXTURES SHALL COMPLY WITH ASTM C494 AND BE OF A TYPE THAT INCREASES THE WORKABILITY OF THE CONCRETE, BUT SHALL NOT BE CONSIDERED TO REDUCE THE SPECIFIED MINIMUM CEMENT CONTENT (CALCIUM CHLORIDE SHALL NOT BE USED).

07. NO CONDUIT PLACED IN A CONCRETE SLAB SHALL HAVE AN OUTSIDE DIAMETER GREATER THAN 1/3 THE THICKNESS OF THE SLAB.

08. NO CONDUIT SHALL BE EMBEDDED IN A SLAB THAT IS LESS THAN 3-1/2" THICK. EXCEPT FOR LOCAL OFFSETS, MINIMUM CLEAR DISTANCE BETWEEN CONDUITS SHALL BE THREE DIAMETERS ON

9. PROJECTING CORNERS OF SLABS, BEAMS, WALLS, COLUMNS, ETC., SHALL BE FORMED WITH A 3/4" CHAMFER.

10. REFER TO DRAWINGS OF OTHER DISCIPLINES FOR MOLDS. GROOVES, CLIPS, ORNAMENTS, OR GROUNDS REQUIRED TO BE CAST

11. CONTRACTOR SHALL SUBMIT MIX DESIGNS FOR REVIEW BEFORE FABRICATION AND INSTALLATION.

CONNECTION HARDWARE

. UNLESS NOTED OTHERWISE, ALL HANGERS, STRAPS, HOLDOWNS, AND MISCELLANEOUS STRUCTURAL CONNECTION SHALL BE SIMPSON STRONG TIE OR AN ENGINEER APPROVED EQUAL. SIMPSON STRONG-TIE CONNECTORS ARE SPECIFICALLY REQUIRED TO MEET THE STRUCTURAL CALCULATIONS OF PLAN. BEFORE SUBSTITUTING ANOTHER BRAND, CONFIRM LOAD CAPACITY BASED UPON RELIABLE PUBLISHED TESTING DATA OR CALCULATIONS. THE ENGINEER OF RECORD SHOULD EVALUATE AND GIVE WRITTEN APPROVAL FOR SUBSTITUTION PRIOR TO INSTALLATION.

CONSTRUCTION CONNECTORS" PUBLISHED ANNUALLY BY SIMPSON UNLESS OTHERWISE SPECIFIED.

MANUFACTURES RECOMMENDATIONS. 4. BOLT HOLES SHALL BE AT LEAST A MINIMUM OF 1/32" AND NO MORE

6. SOME HARDENED FASTENERS MAY HAVE PREMATURE FAILURE IF EXPOSED TO MOISTURE. THE FASTENERS ARE RECOMMENDED TO BE USED IN DRY INTERIOR APPLICATIONS.

STRUCTURAL SHEATHING

1. ALL PLYWOOD SHALL BE CDX SHEATHING WITH EXTERIOR GLUE AND SHALL BEAR THE STAMP OF AN APPROVED TESTING AGENCY. LAY PLYWOOD WITH FACE GRAIN PERPENDICULAR TO SUPPORTS. USE A MINIMUM OF 4-PLY PLYWOOD. STAGGER JOINTS AND NAILS. ORIENTED STRAND BOARD (O.S.B.) WITH EQUIVALENT PROPERTIES MAY REPLACE PLYWOOD.

2. ROOF SHEATHING: 19/32" APA RATED SHEATHING SPAN RATING 40/20. UNBLOCKED WITH 10d COMMON NAILS AT 6" O.C. ALL BOUNDARIES AND SUPPORTED PANEL EDGES 12" O.C. FIELD UNLESS NOTED OTHERWISE.

3. FLOOR / DECK SHEATHING: 3/4" APA RATED STURDI-I-FLOOR SPAN RATING 24". UNBLOCKED WITH 10d COMMON NAILS AT 6"

O.C. ALL BOUNDARIES AND SUPPORTED PANEL EDGES, 12" O.C. FIELD UNLESS NOTED OTHERWISE.

4. SHEAR WALL SHEATHING: STRUCTURAL II PLYWOOD OR O.S.B. PANEL BLOCKED WITH THE NAILING PER SHEAR WALL SCHEDULE, U.N.O.

5. PLYWOOD PANELS TO BE SPACED AT 1/8" AT END AND EDGE JOINTS, FOR: ROOF, FLOOR, AND WALL CONSTRUCTION.

6. ALL DECKS SHALL BE SHEATHED WITH EXTERIOR GRADE PLYWOOD AND SCREWED.

SHEAR PANEL SCHEDULE.

SHEAR PANEL (1) (6)		ANCHOR BOLT	ALLOWABLE	
MARK	MATERIAL	NAILING O/C (5)	SPACING O/C (2) (4)	SHEAR (plf)
1	3/8" CDX PLYWOOD	8d @ 6" EN, 12" FN, 2x STUDS @ 16" O.C BLOCKED	@ 48"	220
2	3/8" CDX PLYWOOD	8d @ 4" EN, 12" FN, INTERMEDIATE 2x STUDS, @ EDGE NAILING TO ABUTTING PANELS 2x STUDS REQUIRED, FRAMING 16" O.C. BLOCKED	@ 32"	320
3	3/8" CDX PLYWOOD (3)	10d @ 3" EN, 12" FN, INTERMEDIATE 2x STUDS, @ EDGE NAILING TO ABUTTING PANELS 3x STUDS REQUIRED, FRAMING 16" O.C. BLOCKED	@ 24"	410
4	3/8" CDX PLYWOOD (3)	10d @ 2" EN, 12" FN, INTERMEDIATE 2x STUDS, @ EDGE NAILING TO ABUTTING PANELS 3x STUDS REQUIRED, FRAMING 16" O.C. BLOCKED	@ 16"	530
5	15/32" STRUCT I PLYWOOD (3)	10d @ 2" EN, 12" FN, INTERMEDIATE 2x STUDS, @ EDGE NAILING TO ABUTTING PANELS 3x STUDS REQUIRED, FRAMING 16" O.C. BLOCKED	@ 12"	870

(1) BLOCK ALL EDGES U.O.N. (2) USE 1/2 SPACING WHERE PLYWOOD APPLIED TO BOTH FACES OF WALL. (3) USE MINIMUM 3x: STUDS, SILL PLATES AND BLOCKING AT EDGE NAILING

(STAGGER NAILING) (4) 5/8 ANCHOR BOLTS

(5) COMMON NAIL (6) DOUBLE-SIDED WALLS REQUIRE 3X SILL PLATE AND AB @ 16" O.C

SHEAR TRANSFER SCHEDULE		2ND FLOOR WALL	1ST FLOOR WALL PERPENDICULAR TO JOISTS	1ST FLOOR WALL PARALLEL TO JOISTS			
SHEAR WALL	MATERIAL USED	SHEAR TRANSFER BOTTOM PLATE	SHEAR TRANSFER TOP PLATE	SHEAR TRANSFER TOP PLATE			
1	3/8" PLYWOOD	2x SILL W/ 16d@ 6" O.C.	LTP4 @12" O.C.	A35 @ 12" O.C.			
2	3/8" PLYWOOD	2x SILL W/ 16d @4" O.C.	LTP4 @12" O.C.	A35 @ 12" O.C.			
3	3/8" PLYWOOD	3x SILL W/ 16d @4" O.C.	LTP4 @12" O.C.	A35 @ 9" O.C.			
4	3/8" PLYWOOD	3x SILL W/ 1/4 x 6" SDS25500 @ 6" O.C.	LTP4 @ 8" O.C.	A35 @ 8" O.C.			
5	15/32" STRUCT I PLYWOOD	3x SILL, 3x RIM OR BLKG W/ 1/4x 6" SDS25500 @ 3" O.C.	LTP4 @ 8" O.C.	A35 @ 6" O.C.			

FOUNDATION NOTES

01. FOR TYPICAL FOOTING DETAILS, SEE SHEET S2.0.

02. FOR HOLDOWN DETAILS, SHEET S2.0. 03. VERIFY DEPTH OF FOOTING AND SIZE OF STEEL REINFORCEMENT WITH CIVIL ENGINEER OR SOILS ENGINEER PRIOR TO START OF CONSTRUCTION.

04. CONCRETE FOR FOOTING AND SLABS SHALL HAVE A MIN. 28 DAYS COMPRESSIVE STRENGTH 2500 psi (UON.) 05. CONTINUOUS CONCRETE FOOTINGS SHALL BE IN ACCORDANCE WITH THE

FOOTING SCHEDULE.

06. PROVIDE POST BASES AT POST WHERE SHOWN PER PLAN. 07. ANCHOR INTERIOR NON-SHEAR/NON-BEARING WALLS WITH HILTI "X-CP 72"

ANCHOR W/ WASHER 0.145 Ø x 2-7/8" LONG PER ICC-ESR-2379 08. SEE "SILL BOLTING REQUIREMENTS" FOR ANCHOR BOLT INFORMATION. 09. LAP SPLICE CONTINUOUS REINFORCING STEEL PER DETAIL 3/S2.0

TERMINATE REBARS AT FOOTING CORNERS PER 1/S2.0. 10. ALL HOLDOWNS COLUMNS BASES, ANCHORS BOLTS, POST ANCHORS, ETC. TO BE TIED IN PLACE AND VERIFIED BY ROUGH FRAMING CONTRACTOR PRIOR TO

CALLING FOR FOUNDATION INSPECTION. 11. THE INSPECTOR WILL RECHECK FOR EXPANSIVE SOILS AND / OR GRADING

REQUIREMENTS AT THE FIRST INSPECTION.

12. FURTHER NOTES SHOULD BE REVIEWED ON SHEET S-2.

PROJECT NOTES

01. SEE FLOOR AND ROOF FRAMING PLANS FOR FURTHER NOTES.

02. LOWER FLOOR: FLOOR FRAMING PLAN 03. UPPER FLOOR: ROOF FRAMING PLAN. 04. FOR LOWER FLOOR SHEARWALLS, FOUNDATION PLAN.

11. ROOF JOISTS: SEE ROOF FRAMING PLANS

WITHIN THE SCOPE OF THE PERMIT ISSUED.

05. FOR UPPER FLOOR SHEARWALLS, ROOF FRAMING PLAN. 06. POSTS USE 4 x 6 POSTS (UON.)

USE 6 x 6 POSTS FOR PLATE HEIGHTS ABOVE 14'-0" 07. KING POSTS: MIN 4 x 4 (U.O.N.) 08. WALL STUDS: USE 2 x 4 @ 16" OC UP TO 10'-0"

USE 2 x 6 @ 16" OC UP TO 16'-0". USE 2 x 8 @ 16" OC UP TO 21'-1 09. HEADERS: ARE TO BE IN ACCORDANCE WITH THE TYPICAL HEADER DETAIL. 10. CEILING JOISTS: SEE CEILING JOIST SCHEDULE

12. FLOOR SHEATHING: A. SEE FRAMING PLANS B. NAILING 10d COMMON OC. @ 6" BN, 6" EN, 12" FN 13. THE STRUCTURAL ENGINEER SHOULD BE NOTIFIED OF ANY PROBLEMS

STRUCTURAL DESIGN CRITERIA

VERTICAL DESIGN TYPICAL ROOF DEAD LOAD ROOF LIVE LOAD 20 PSF FLOOR DEAD LOAD 14.8 PSF FLOOR LIVE LOAD 40 PSF DECK DEAD LOAD 14 PSF DECK LIVE LOAD 40 PSF **EXTERIOR WALL WEIGHT** 18 PSF INTERIOR WALL WEIGHT

ASCE 7-05 SEISMIC FACTOR DETERMINATION

Ss= 1.577 g 0.2 sec. response S1= 0.616 g 1.0 sec response SITE CLASSIFICATION= D

> Sm1= 0.923

SEISMIC DESIGN CATEGORY= D

RESISTING SYSTEM BASIC SEISMIC FORCE RESISTING SYSTEM= LIGHT-FRAMED WALLS SHEATHED W/ WOOD

SHEAR PANELS.

REDUNDANCY FACTOR: 6.5 (SW) REDUNDANCY FACTOR=1.0 Ct= 0.020

AMERICAN CONCRETE INSTITUTE

EXPANSION

FLUSH BEAM

FOUNDATION

FLOOR JOIST

GYPSUM BOARD GLUE LAM BEAM

FOOTING

HEADER

INSULATION

AMERICAN INSTITUTE OF STEEL CONSTRUCTION

AMERICAN SOCIETY OF TESTING MATERIALS

x = 0.75

Ta= 0.256

hn= 30.0 ft

TL= 8.000 sec

MAS.

MASONRY

SIMPSON STRONG WALL

TOP AND BOTTOM

TOP OF CONCRETE TOP OF FOOTING

UNLESS NOTED OTHERWISE

UNLESS OTHERWISE NOTED

BOISE CASCADE VERSA-LAM

WELDED WIRE REINFORCEMENT

TOP OF PIER

TOP OF STEEL

VERTICAL

TOP OF WALL

SIMPSON STEEL STRONG WALL

STRUCTURAL ABBREVIATIONS

Cs(ASD) = 1.62

MATERIAL ARCHITECTURAL PLANS MAXIMUM MACHINE BOLT BOISE CASCADE "I" JOIST MECHANICAL MINIMUM BLOCKING MISCELLANEOUS BOUNDARY NAII NATURAL GRADE BOTTOM OF PIEF NOT TO SCALE ON CENTER POUNDS PER LINEAR FOOT CEILING JOIST POUNDS PER SQUARE FOOT CONSTRUCTION POST TENSION PRESSURE TREATED DOUGLAS FIR CONTINUOUS RIDGE BEAM REFERENCE DIAMETER REINFORCEMENT DIMENSION SCHED SCHEDULE SPEC'S SPECIFICATION EDGE NAIL STRUCTURAL SELECT EACH WAY

CHECKED BY: CKP

SAN DIEGO, CA 92109 PHONE: 858-605-0937

FAX: 858-605-1414

H

DESIGNER:

REVISIONS

DATE REV REVISION

DRAWING STATUS

☐ INFORMATION DRAWINGS. (NOT FOR CONSTRUCTION) PRELIMINARY DRAWINGS (NOT FOR CONSTRUCTION) (NOT FOR CONSTRUCTION)

☐ PLAN CHECK DRAWINGS.

CONTENT STRUCTURAL NOTES

DRAWN BY: CR/CKP

DATE: SEPTEMBER 20, 2013

☐ CONSTRUCTION DOCUMENTS.

1. FABRICATION AND ERECTION OF STRUCTURAL STEEL SHALL BE IN ACCORDANCE WITH THE AISC SPECIFICATIONS FOR STRUCTURAL STEEL BUILDINGS (360-05 & 341-05) WHERE THE STRUCTURAL STEEL IS EXPOSED, FABRICATION AND ERECTION SHALL ALSO BE IN ACCORDANCE WITH AISC CODE OF STANDARD PRACTICE FOR ARCHITECTURALLY EXPOSED STRUCTURAL STEEL.

2. STRUCTURAL STEEL SHALL CONFORM TO ASTM DESIGNATION AS INDICATED BELOW (U.N.O.).
PAINTING OF STEEL IS NOT REQUIRED STRUCTURALLY. VARIATION IN THE YIELD STRENGTH BELOW
THE SPECIFIED MINIMUM VALUE IS NOT ALLOWED:
ALL W SHAPES (U.N.O.)
A992, (A193, GRADE 65 WHERE NOTED)

ALL OTHER STRUCTURAL SECTIONS
PLATES
STRUCTURAL TUBING
PIPE COLUMNS
ANCHOR BOLTS
ASECTIONS
A5
ANCHOR BOLTS

STAIR CHANNELS, PLATES, SHAPES, BARS

SECTIONS

A572, GRADE 50 (A36 WHERE NOTED)

A572, GRADE 50 (GRADE 42 IF ≥ 4" THICK)

A500, GRADE B (Fy=46ksi)

A53, GRADE B (Fy=42ksi)

F1554 GRADE 36

A36 / A36M U.N.O.

3. MEMBERS IN THE SEISMIC FORCE RESISTING SYSTEM INCLUDE BRACED FRAME COLUMNS, BRACED FRAME BEAMS, BRACES, DRAG BEAMS, MOMENT FRAME BEAMS, AND MOMENT FRAME COLUMNS.

4. HEAVY STRUCTURAL SECTIONS INCLUDE, ASTM A6 GROUP 3 SHAPES WITH FLANGES 1|4" THICK OR THICKER, ASTM A6 GROUP 4 AND 5 SHAPES, AND PLATES THAT ARE 1|4" THICK OR THICKER IN BUILT-UP CROSS-SECTIONS.

5. HEAVY STRUCTURAL SECTIONS AND ALL BRACED FRAME COLUMNS SHALL BE SUPPLIED WITH TESTING IN ACCORDANCE WITH ASTM A6 SUPPLEMENTARY REQUIREMENT S5 AND SHALL HAVE A MINIMUM CHARPY V-NOTCH (CVN) TOUGHNESS OF 20 FT/LBS AT 70 DEGREES FAHRENHEIT

6. HIGH STRENGTH BOLTS SHALL BE INSTALLED IN ACCORDANCE WITH THE CURRENT (DESIGN) EDITION OF AISC SPECIFICATIONS FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS. HIGH STRENGTH BOLTS SHALL BE BEARING TYPE WITH THREADS EXCLUDED FROM THE FROM THE SHEAR PLANES (I.E. A325-X) UNLESS NOTED OTHERWISE. SLIP CRITICAL BOLTS SHALL BE USED FOR MOMENT AND DRAG CONNECTIONS.

7. ALL BOLTED CONNECTIONS SHALL HAVE A MINIMUM OF TWO BOLTS UNLESS SHOWN OTHERWISE. MINIMUM SIZE OF BOLTS FOR STRUCTURAL STEEL CONNECTIONS SHALL BE 7/8" DIA. EXCEPT WHEN OTHERWISE SHOWN OR NOTED.

8. WHEN FABRICATING BEAMS PLACE NATURAL CAMBER UP.

9. ALL FLANGE STIFFENER PLATES SHALL BE MADE FROM BI-DIRECTIONALLY ROLLED STEEL PLATE.

10. CONDITION OF STEEL ON DELIVERY SHALL BE PER THE CURRENT (DESIGN) EDITION OF AISC SPECIFICATIONS FOR STRUCTURAL STEEL BUILDINGS.

11. SEE ARCHITECTURAL PLANS FOR DETAILS OF FIREPROOFING.

12. ALL HOLES SHALL BE STANDARD DIAMETER U.N.O.

13. INSTALL BOLTS/EXPANSION ANCHORS IN CONCRETE AND FIELD VERIFY AS-BUILT LOCATIONS PRIOR TO FABRICATION OF BOLT HOLES IN NEW STEEL MEMBERS. AT CONTRACTOR OPTION, OVERSIZED HOLES AND WELDED PLATE WASHERS CAN BE USED IN LIEU OF STANDARD DIAMETER HOLES.

14. PROVIDE FILLS AT SPLICES OF PARTS HAVING MORE THAN |1" DIFFERENCE IN THICKNESS.

15. PROVIDE BEVELED WASHERS ON ALL CONNECTIONS WHERE SLOPE SURFACE EXCEEDS 1:20.

16. SPLICE MEMBERS ONLY WHERE INDICATED.

17. STRESSES AT ALL TIMES SHALL BE LESS THAN DESIGN AND ALLOWABLE STRESSES. THE FULL DESIGN AND LOAD CARRYING CAPACITY OF THE STEELWORK SHALL NOT BE IMPAIRED DUE TO FABRICATION, SHIPMENT, OR ERECTION PROCEDURES, THROUGHOUT THE COMPLETE PROCESS. THE STABILITY OF ALL INDIVIDUAL MEMBERS AND ASSEMBLIES SHALL BE MAINTAINED.

18. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE CONTROL OF ALL ERECTION PROCEDURES AND SEQUENCES WITH RELATION TO TEMPERATURE DIFFERENTIALS AND WELD SHRINKAGE. CONTRACTOR TO SUBMIT WELDING SEQUENCE PROCEDURE FOR APPROVAL BY SEOR.

19. ALL BASE PLATE TO BE FULLY GROUTED AND REACHES 28 DAYS STRENGTH PRIOR TO LOADING.

20. ALL ANCHOR BOLTS IN BASE PLATES WITH OVERSIZED HOLES SHALL HAVE 3-1/2 IN. SQ. WASHER PLATES UNO. THICKNESS OF WASHER PLATES SHALL BE 0.375 TIMES THE DIAMETER OF THE ANCHOR BOLTS. PLATE WASHERS SHALL BE WELDED TO BASE PLATE WITH 5/16" FILLET WELD ON ALL SIDES.

21. ANCHOR BOLTS TO HAVE DOUBLE NUT ON EMBEDDED SIDE U.N.O.

STRUCTURAL STEEL WELDING NOTES

1. ALL WELDING SHALL BE IN STRICT CONFORMANCE WITH THE 2010 EDITION OF AWS D1.1, THE 2009 D1.8 SEISMIC SUPPLEMENT, AND THE CALIFORNIA BUILDING CODE.

2. ALL WELDING ELECTRODES (FILLER METAL) SHALL BE E7XXX (70 KSI) OR E70TXXXX (70 KSI) AS REQUIRED FOR INTENDED USE, U.N.O., AND SHALL BE LOW HYDROGEN TYPES.

3. ALL WELDS SHALL HAVE A FILLER METAL WITH A MINIMUM CHARPY V-NOTCH TOUGHNESS OF 20 FT/LBS AT -20 DEGREES FAHRENHEIT AND 40 FT/LBS @ 70 DEGREES FAHRENHEIT CERTIFY CONFORMANCE TO CHARPY V-NOTCH TOUGHNESS REQUIREMENTS WITH TESTS BY AN INDEPENDENT TESTING LABORATORY.

4. LENGTHS OF WELDS ARE EFFECTIVE LENGTHS AS SPECIFIED IN THE CALIFORNIA BUILDING CODE. WHERE LENGTH OF WELD IS NOT SHOWN IT SHALL BE FULL LENGTH OF JOINT. ALL BUTT WELDS SHALL BE FULL PENETRATION, UNLESS NOTED OTHERWISE.

5. ALL SHOP WELDS SHALL BE PERFORMED BY A LICENSED FABRICATOR.

6. ALL WELDERS SHALL BE QUALIFIED FOR THE WORK THEY WILL BE DOING & SHALL HAVE CERTIFICATIONS CURRENT.

7. FACES OF FILLET WELDS EXPOSED TO VIEW SHALL HAVE AS-WELDED SURFACES THAT REQUIRE NO FINISHING OR GRINDING EXCEPT WHERE CLEARANCES OR FIT OF OTHER ITEMS MAY SO NECESSITATE.

8. ALL PARTIAL AND FULL PENETRATION WELDS WHICH ARE EXPOSED TO VIEW SHALL BE IN ACCORDANCE WITH AISC COMMENTARY ON AESS.

9. CLEAN GROOVE PREPARATION THERMAL CUTS BY GRINDING.

10. WELDS SHALL BE TERMINATED AT THE END OF A JOINT IN A MANNER THAT WILL ENSURE SOUND WELDS. WHENEVER NECESSARY THIS SHALL BE DONE BY USE OF EXTENSION BARS AND WELD TABS, PER AWS D1.1-9 SECTION 5.31, ALIGNED IN SUCH A MANNER TO PROVIDE AN EXTENSION OF THE JOINT PREPARATION.

11. THE CONTRACTOR SHALL SUBMIT ALL WELDING PROCEDURE SPECIFICATIONS (WPS) TO BE USED ON THE PROJECT PER THE CURRENT (DESIGN) EDITION OF AWS D1.1. FOR ALL WPS THAT ARE NOT PREQUALIFIED PER AWS D1.1, THE SUPPORTING PROCEDURE QUALIFICATION RECORD (PQR) SHALL ALSO BE SUBMITTED WITH THE WPS. THE WPS SHALL INCLUDE ALL MANUFACTURER'S DATA SHEETS FOR ALL WELDING MATERIALS TO BE USED. THE DATA SHEETS SHALL DESCRIBE THE PRODUCTS, LIMITATIONS OF USE, RECOMMENDED WELDING PARAMETERS, AND STORAGE AND EXPOSURE REQUIREMENTS.

12. THE CONTRACTOR SHALL PROVIDE PLANS SHOWING THE SEQUENCE OF WELDING PLANNED TO MINIMIZE LOCKED IN STRESSES AND DISTORTION IN THE STEEL FRAMING.

13. LOW HYDROGEN SMAW ELECTRODES SHALL BE RECEIVED AND STORED IN THE ORIGINAL, UNDAMAGED MANUFACTURER PACKAGING, UNTIL READY FOR USE. WHEN WELDING IS TO BE SUSPENDED FOR MORE THAN 8 HOURS, ELECTRODES SHALL BE REMOVED FROM THE MACHINES AND STORED IN AN ELECTRODE WIRE OVEN MAINTAINED AT A TEMPERATURE BETWEEN 250 DEGREES AND 550 DEGREES OR AS RECOMMENDED BY THE MANUFACTURER. ELECTRODES NOT CONSUMED WITHIN 24 HOURS OF ACCUMULATED EXPOSURE OUTSIDE CLOSED OR HEATED STORAGE SHALL NOT BE USED. FLUX CORED ELECTRODES SHALL BE RECEIVED AND STORED PER AWS D1.1.

14. BACKING BAR FOR THE BOTTOM FLANGE OF BEAM TO COLUMN CONNECTIONS TO BE REMOVED. FOLLOWING REMOVAL OF BACKING, THE ROOT PASS SHALL BE BACKGOUGED TO SOUND WELD METAL AND BACKWELDED UNTIL FLUSH OR WITH SLIGHT REINFORCEMENT.

WHERE WELD TABS ARE USED, THEY SHALL BE REMOVED & THE SURFACE SHALL BE GROUND SMOOTH

TO A SURFACE ROUGHNESS NOT TO EXCEED 500 MICROINCHES.

15. MINIMUM PREHEAT AND INTERPASS TEMPERATURES SHALL BE PROVIDED FOR ALL WELDS, INCLUDING TACK WELDS, IN ACCORDANCE WITH AWS D1.1 AND D1.8, THE MAXIMUM PREHEAT AND MAXIMUM INTERPASS TEMPERATURE PERMITTED IS 550 DEGREES FAHRENHEIT, MEASURED AT A DISTANCE OF 1" FROM THE POINT OF ARC INITIATION.

1. STRUCTURAL OBSERVATION IS REQUIRED FOR THIS PROJECT IN ACCORDANCE WITH CBC 1710. STRUCTURAL OBSERVATION IS THE VISUAL OBSERVATION OF THE STRUCTURAL SYSTEM BY A LICENSED DESIGN PROFESSIONAL FOR GENERAL CONFORMANCE TO THE APPROVED CONSTRUCTION DOCUMENTS.

2. STRUCTURAL OBSERVATION DOES NOT WAIVE THE RESPONSIBILITY FOR THE REQUIRED INSPECTIONS BY THE LOCAL BUILDING OFFICIAL.

3. THE OWNER SHALL EMPLOY A LICENSED DESIGN PROFESSIONAL TO PERFORM STRUCTURAL OBSERVATION SITE VISITS, AND TO ISSUE ALL STRUCTURAL OBSERVATION REPORTS.

4. THE DESIGN ENGINEER SHALL IDENTIFY THE REQUIRED STRUCTURAL OBSERVATION SITE VISITS ON THE STRUCTURAL OBSERVATION

5. THE REQUIRED SITE VISIT SHALL AT A MINIMUM INCLUDE THE FOLLOWING: 5.1. OBSERVATION OF THE FOUNDATION SYSTEM PRIOR TO FINAL CONCRETE POUR.

CONSTRUCTION OF SAID CHANGES

5.2. OBSERVATION OF BUILDING FRAMING PRIOR TO CALLING FOR THE LOCAL BUILDING OFFICIAL "COMPLETE FRAMING INSPECTION"; AND, 5.3. FINAL OBSERVATION OF THE COMPLETE STRUCTURE.

6. THE STRUCTURAL OBSERVER SHALL PREPARE A STRUCTURAL OBSERVATION REPORT FOR EACH STAGE OF CONSTRUCTION

7. ALL STRUCTURAL OBSERVATION REPORTS, REGARDLESS OF FORM USED, SHALL INCLUDE THE THE LICENSE STAMP & SIGNATURE OF

THE STRUCTURAL OBSERVER RESPONSIBLE FOR THE PROJECT.

8. EACH STRUCTURAL OBSERVATION REPORT SHALL BE GIVEN TO THE OWNER OR OWNER'S REPRESENTATIVE, PROJECT CONTRACTOR,

9. THE CONTRACTOR SHALL RESOLVE ALL DEFICIENCIES & THE FINAL STRUCTURAL OBSERVATION REPORT ISSUED PRIOR TO FINAL INSPECTION OR ACCEPTANCE OF STRUCTURAL WORK BY THE BUILDING INSPECTOR.

10. THE FINAL STRUCTURAL OBSERVATION REPORT SHALL STATE THAT THE STRUCTURAL SYSTEM CONFORMS TO THE APPROVED CONSTRUCTION DOCUMENTS & THAT ALL PREVIOUSLY OBSERVED DEFICIENCIES HAVE BEEN CORRECTED.

11. FINAL INSPECTION OR OTHER ACCEPTANCE OF THE STRUCTURAL SYSTEM BY THE CHIEF BUILDING OFFICIAL, OR DESIGNEE, WILL NOT OCCUR UNTIL THE FINAL STRUCTURAL OBSERVATION REPORT IS RECEIVED.

12. THE LICENSED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE SHALL PREPARE ALL CONSTRUCTION DOCUMENT CHANGES RELATING TO THE STRUCTURAL SYSTEMS. REVIEW & APPROVAL OF SUCH CHANGES BY THE CHIEF BUILDING OFFICIAL, OR DESIGNEE, SHALL BE OBTAIN BY THE DESIGN PROFESSIONAL AND/OR CONTRACTOR PRIOR TO INSTALLATION AND/OR

STRUCTURAL OBSERVATION SCHEDULE

BASED ON REQUIRE S	THE TRU	TED BY THE DESIGN ENGINEER, AND INCLUDED OPROJECT SCOPE, PLEASE IDENTIFY THE ELEMEN CTURAL OBSERVATION. SPECIFY THE INTERVAL OUTURAL OBSERVATION WILL BE PERFORMED.	TS AND/OR CONNECTIONS THAT
I TYPE I TOTAL TOT			SCHEDULED INTERVAL OR STAGE OF CONSTRUCTION
NO		FOOTINGS, SLAB FOUNDATION, ANCHORS	
FOUNDATION		MAT FOUNDATION, PRESTRESSED CONC. SLAB	
		CAISSON, PILE, GRADE BEAM	
D _T		OTHER:	
		CONCRETE	
AR		MASONRY	
SHEAR		WOOD OR MANUFACTURED SHEAR PANELS	
		STEEL MOMENT OR BRACED FRAME	
IES		CONCRETE MOMENT FRAME	
FRAMES		MASONRY WALL FRAME	
		OTHER:	
MS		CONCRETE	
RAG		STEEL DECK	
DIAPHRAGMS		WOOD	
		OTHER:	·
FINAL		FINAL OBSERVATION REPORT	

ADDITIONAL SPECIAL INSPECTION NOTES

1. THE SPECIAL INSPECTOR MUST BE CERTIFIED BY THE LOCAL BUILDING OFFICIAL, DEVELOPMENT SERVICES, IN THE CATEGORY OF WORK REQUIRED TO HAVE SPECIAL INSPECTION.

2. FABRICATOR MUST BE REGISTERED AND APPROVED BY THE LOCAL BUILDING OFFICIAL, DEVELOPMENT SERVICES, FOR THE FABRICATION OF MEMBERS AND ASSEMBLIES ON THE PREMISES OF THE FABRICATOR'S SHOP.

3. FABRICATOR SHALL SUBMIT AN 'APPLICATION TO PERFORM OFF-SITE FABRICATION' TO THE INSPECTION SERVICES DIVISION FOR APPROVAL PRIOR TO COMMENCEMENT OF FABRICATION.

4. FABRICATOR SHALL SUBMIT A 'CERTIFICATE OF COMPLIANCE FOR OFF-SITE FABRICATION' TO THE INSPECTION SERVICES DIVISION PRIOR TO

ERECTION OF FABRICATED ITEMS AND ASSEMBLIES.

5. A PROPERTY OWNER'S FINAL REPORT FORM OF WORK REQUIRED TO HAVE SPECIAL INSPECTION, TESTING AND STRUCTURAL OBSERVATIONS MUST BE COMPLETED BY THE PROPERTY OWNER, PROPERTY OWNER'S AGENT OF RECORD, ARCHITECT OF RECORD, OR ENGINEER OF RECORD

6. THE SPECIAL INSPECTIONS IDENTIFIED ON PLANS ARE, IN ADDITION TO, AND NOT A SUBSTITUTE FOR, THOSE INSPECTIONS REQUIRED TO BE PERFORMED BY A BUILDING INSPECTOR.

AND SUBMITTED TO THE INSPECTION SERVICES DIVISION.

7. NOTICE TO THE APPLICANT/OWNER/OWNER'S AGENT/ARCHITECT OR ENGINEER OF RECORD: BY USING THIS PERMITTED CONSTRUCTION DRAWINGS FOR CONSTRUCTION/INSTALLATION OF THE WORK SPECIFIED HEREIN, YOU AGREE TO COMPLY WITH THE REQUIREMENTS OF THE LOCAL BUILDING OFFICIAL FOR SPECIAL INSPECTIONS, AND THOSE REQUIREMENTS REQUIRED BY THE CALIFORNIA CONSTRUCTION CODES.

8. NOTICE TO THE CONTRACTOR / BUILDER / INSTALLER / SUB-CONTRACTOR / OWNER-BUILDER: BY USING THIS PERMITTED CONSTRUCTION DRAWINGS FOR CONSTRUCTION/ INSTALLATION OF THE WORK SPECIFIED HEREIN, YOU ACKNOWLEDGE AND ARE AWARE OF, THE REQUIREMENTS CONTAINED IN THE STATEMENT OF SPECIAL INSPECTIONS. YOU AGREE TO COMPLY WITH THE REQUIREMENTS OF THE LOCAL BUILDING OFFICIAL FOR SPECIAL INSPECTIONS, STRUCTURAL OBSERVATIONS, CONSTRUCTION MATERIAL TESTING AND OFF-SITE FABRICATION OF BUILDING COMPONENTS, CONTAINED IN THE STATEMENT OF SPECIAL INSPECTIONS AND, AS REQUIRED BY CALIFORNIA CONSTRUCTION CODES.

1. PROVIDE SPECIAL INSPECTIONS IN ACCORDANCE WITH THE BELOW NOTED SECTION OF THE 2010 CALIFORNIA BUILDING CODE.

2. WHERE SPECIAL INSPECTION IS REQUIRED, IT SHALL BE PERFORMED BY A REGISTERED DEPUTY INSPECTOR EMPLOYED BY THE OWNER AND APPROVED BY THE GOVERNING JURISDICTION. COPIES OF THE INSPECTION REPORTS SHALL BE SUBMITTED TO THE BUILDING DEPARTMENT AND ENGINEER FOR REVIEW.

3. A CERTIFICATE OF SATISFACTORY COMPLETION OF WORK REQUIRING SPECIAL INSPECTION, MUST BE COMPLETED AND SUBMITTED TO THE INSPECTION SERVICES DIVISION

4. THESE ITEMS ARE IN ADDITION TO THE BUILDING DEPARTMENT REQUIREMENTS.

5. THE SPECIAL INSPECTOR MUST BE APPROVED BY THE LOCAL JURISDICTION.6. THE TESTING LABORATORY MUST BE APPROVED BY THE LOCAL JURISDICTION.

7. A PROPERTY OWNER'S FINAL REPORT OF WORK REQUIRING SPECIAL INSPECTION MUST BE COMPLETED BY THE PROPERTY OWNER, PROPERTY OWNER'S AGENT OF RECORD, ARCHITECT OF RECORD, OR ENGINEER OF

RECORD AND SUBMITTED TO THE INSPECTION SERVICES DIVISION.

8. THE SPECIAL INSPECTIONS IDENTIFIED ARE IN ADDITION TO THOSE REQUIRED BY SEC.108 OF THE BUILDING CODE, AS AMENDED. SPECIAL INSPECTION IS NOT A SUBSTITUTE FOR INSPECTION BY A LOCAL JURISDICTION

DESCRIPTION AND LOCATION

9. SOILS SPECIAL INSPECTION SHALL BE PERFORMED BY THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE (SOILS ENGINEER OR GEOTECHNICAL ENGINEER OF RECORD), WHO HAS PREPARED

1704.7

DESIGN SPECIAL INSPECTOR APPROVED NAME & NUMBER (INITIAL &

THE APPROVED GEOTECHNICAL INVESTIGATION REPORT.

INSPECTION CODE SECTION

A. SHEAR WALL HOLDOWNS 1707.3

B. SHEAR WALL NAILING TYPE 2, 3, 4, 5 1707.3

C. DRAG STRUTS 1707.3

E. EPOXY GROUTED ANCHOR BOLTS	1704 13
	1701.10
F. MASONRY CONSTRUCTION	1704.5
G. CONCRETE CONSTRUCTION	1704.4
	1704.0
H. STEEL CONSTRUCTION	1704.3

10. ITEMS REQUIRING SPECIAL INSPECTION:

D. SOILS

TYPE OF

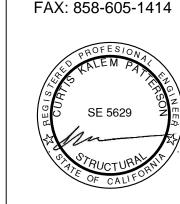
INSPECTION		DESCRIPTION AND LOCATION	STRENGTH	(PERMITEE TO PROVIDE)	DATE)
	Р	VERIFY MATERIALS BELOW FOOTINGS ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY.			
	Р	VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL.			
D.	Р	PERFORM CLASSIFICATION AND TESTING OF CONTROLLED FILL MATERIALS.		C.W. LA MONTE COMPANY, INC. 619-462-9861	
SOILS	С	VERIFY USE OF PROPER MATERIALS, DENSITIES AND LIFT THICKNESSES DURING PLACEMENT AND COMPACTION OF CONTROLLED FILL.		0.0 .02 000	
	Р	PRIOR TO PLACEMENT OF CONTROLLED FILL, OBSERVE SUBGRADE AND VERIFY THAT SITE HAS BEEN PREPARED PROPERLY.			
E. EPOXY	Р	SPECIAL INSPECTOR TO VERIFY THE DRILLING OF ANY HOLES, THE CLEANLINESS OF THE HOLE, THE MOISTURE IN THE HOLE, MIXING OF THE EPOXY, EPOXY BRAND AND PROPER MATERIAL ASSEMBLY.	SIMPSON SET-XP ESR-2508		
		CMU WALLS			
		CONTINUOUS (C)			
	С	GROUT PLACEMENT SHALL BE VERIFIED TO ENSURE COMPLIANCE WITH CODE AND CONSTRUCTION DOCUMENT PROVISIONS.			
	С	PREPARATION OF ANY REQUIRED GROUT SPECIMENS, MORTAR SPECIMENS AND/OR PRISMS SHALL BE OBSERVED.			
		PERIODIC (P)			
	Р	3. AS MASONRY CONSTRUCTION BEGINS, THE FOLLOWING SHALL BE VERIFIED TO ENSURE COMPLIANCE:			
	Р	a. PROPORTIONS OF SITE-PREPARED MORTAR.			
	Р	b. CONSTRUCTION OF MORTAR JOINTS.			
	Р	c. LOCATION OF REINFORCEMENT, CONNECTIONS, PRESTRESSING TENDONS AND ANCHORAGES.			
F. MASONRY	Р	4. THE INSPECTION PROGRAM SHALL VERIFY:			
CONSTRUCTION	Р	a. SIZE AND LOCATION OF STRUCTURAL ELEMENTS.			
	Р	b. TYPE, SIZE AND LOCATION OF ANCHORS, INCLUDING OTHER DETAILS OF ANCHORAGE OF MASONRY TO STRUCTURAL MEMBERS, FRAMES OR OTHER CONSTRUCTION.			
	Р	c. SPECIFIED SIZE, GRADE AND TYPE OF REINFORCEMENT.			
	٢	d. PROTECTION OF MASONRY DURING COLD WEATHER (TEMPERATURE BELOW 40°F) OR HOT WEATHER (TEMPERATURE ABOVE 90°F).			
	Р	5. PRIOR TO GROUTING, THE FOLLOWING SHALL BE VERIFIED TO ENSURE COMPLIANCE:			
		a. GROUT SPACE IS CLEAN.			
	Р	b. PLACEMENT OF REINFORCEMENT AND CONNECTORS AND PRESTRESSING TENDONS AND ANCHORAGES.			
	Р	c. PROPORTIONS OF SITE-PREPARED GROUT AND PRESTRESSING GROUT FOR BONDED TENDONS.			
	Р	d. CONSTRUCTION OF MORTAR JOINTS.			
	Р	6. COMPLIANCE WITH REQUIRED INSPECTION PROVISIONS OF THE CONSTRUCTION DOCUMENTS AND THE APPROVED SUBMITTALS SHALL BE VERIFIED. SUBMITTAL OF CERTIFICATION OF COMPLIANCE AND VERIFICATION OF F'M AND F'AAC PRIOR TO CONSTRUCTION.			

CONTINUOUS (C) PERIODIC (P)

TABLE 1704.4 — REQUIRED VERIFICATION AND	T		ONSTRUCTION	
VERIFICATION AND INSPECTION TASK	FREQUENCY OF		REFERENCED	IBC
	CONTIUOUS	PERIODIC	STANDARD	REFERENCE
1. INSPECTION OF REINFORCING STEEL, INCLUDING PRESTRESSING TENDONS AND PLACEMENT.		X	ACI 318: 3.5, 7.1-7.7	1913.4
2. INSPECTION OF REINFORCING STEEL WELDING IN ACCORDANCE WITH TABLE 1704.3 ITEM 5b.		X	AWS D1.4 ACI318: 3.5.2	
3. INSPECT BOLTS TO BE INSTALLED IN CONCRETE PRIOR TO AND DURING PLACEMENT OF CONCRETE WHERE ALLOWABLE LOADS HAVE BEEN INCREASED OR WHERE STRENGTH DESIGN IS USED.	Х		ACI 318: 8.1.3, 21.2.8	1911.5, 1912.1
4. INSPECTION OF ANCHORS INSTALLED IN HARDENED CONCRETE		Х	ACI 318: 3.8.6, 21.2.8	1912.1
5. VERIFYING USE OF REQUIRED DESIGN MIX.		X	ACI 318: CH 4, 5	1904.2.2, 1913.2, 1913.3
6. AT TIME FRESH CONCRETE IS SAMPLED TO FABRICATE SPECIMENS FOR STRENGTH TESTS, PERFORM SLUMP AND AIR CONTENT TESTS AND DETERMINE THE TEMPERATURE OF THE CONCRETE.	Х		ASTM C172 C31 ACI 318 5.6, 5.8	1913.10
7. INSPECTION OF CONCRETE AND SHOTCRETE PLACEMENT FOR PROPER APPLICATION TECHNIQUES.	X		ACI 318: 5.9, 5.10	1913.6 -7-8
8. INSPECTION FOR MAINTENANCE OF SPECIFIED CURING TEMPERATURE AND TECHNIQUES.		X	ACI 318: 5.11-5. 13	1913.9
9. INSPECTION OF PRESTRESSED CONCRETE. a. APPLICATION OF PRESTRESSING FORCES. b. GROUTING OF BONDED PRESTRESSING TENDONS IN THE SEISMIC FORCE—RESISTING SYSTEM.	X		ACI 318: 18.20, 18.18.4	
10. ERECTION OF PRECAST CONCRETE MEMBERS.		X	ACI 318: CH 16	
11. VERIFICATION OF IN-SITU CONCRETE STRENGTH, PRIOR TO STRESSING OF TENDONS IN POSTENSIONED CONCRETE AND PRIOR TO REMOVAL OF SHORES AND FORMS FROM BEAMS AND STRUCTURAL SLABS.		X	ACI 318: 6.2	
12. INSPECT FORMWORK FOR SHAPE, LOCATION, AND DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED.		X	ACI 318: 6.1.1	
13. INSPECT LENTON STANDARD MECHANICAL COUPLERS.		X	IAPMO ES 0129	

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INFORMATION DRAWINGS.
(NOT FOR CONSTRUCTION)
PRELIMINARY DRAWINGS.
(NOT FOR CONSTRUCTION)
CHECK SET
(NOT FOR CONSTRUCTION)

☐ PLAN CHECK DRAWINGS.

☐ CONSTRUCTION DOCUMENTS.
☐ OTHER
AS-BUILT

CONTENT

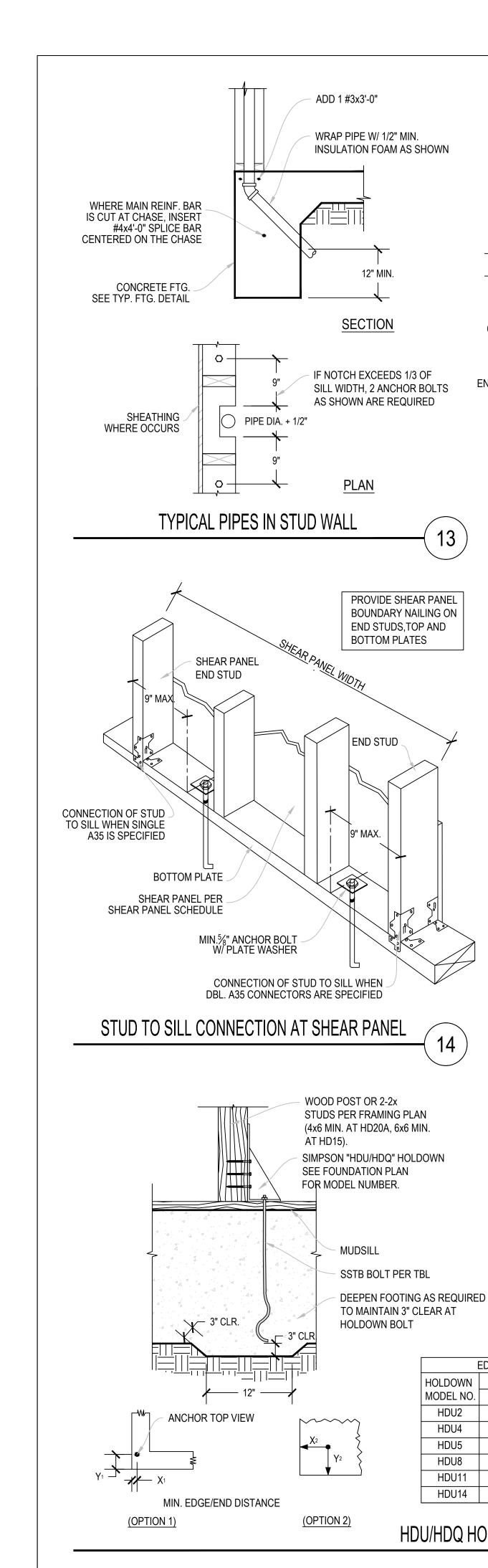
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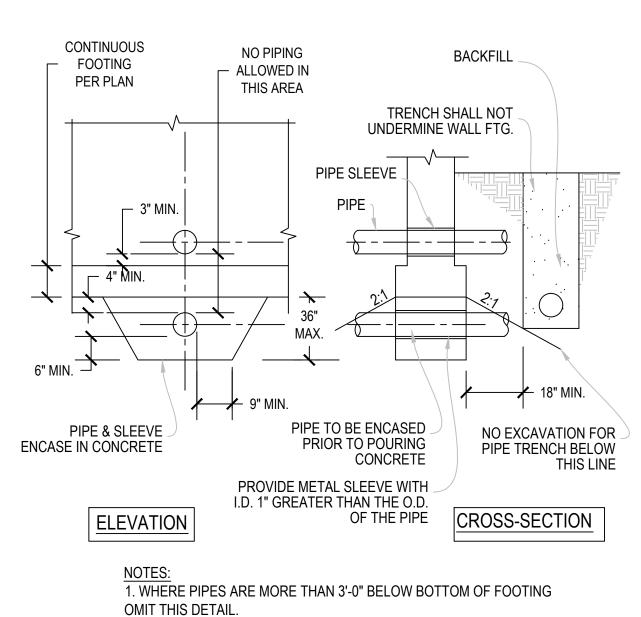
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S1.1





PIPE LOCATIONS AT FOOTING

TOP OF SLOPE

MINIMUM DISTANCE

SSTB34

N/A

1. USE SIMPSON SDS¹/₄x2-1/2

EMBEDMENTS U.N.O ON PLANS

. USE THIS SCHEDULE FOR ALL HOLDOWN BOLT

3. THE EMBEDMENT LENGTHS ARE BASED ON A

. WHEN USING COMPOSITE LUMBER POSTS,

SCREWS MUST BE APPLIED TO THE WIDE FACE

5. ALL DATA SHOWN PER SIMPSON STRONG-TIE CO.

STANDARD SSTB BOLTS PER SIMPSON.

ANCHOR NOTES

WOOD SCREWS.

OF THE COLUMN.

DEEPENED FOOTING AT SLOPE

HOLDOWN ANCHOR EMBED. ANCHOR BOLT TYPE

5/8" Ø | 12-5/8" | SSTB16

7/8" Ø 24-7/8" SSTB28

7/8" Ø 24-7/8" SSTB28

1" Ø 28" ASTM307

(OPTION 2)

7-1/2 7-1/2

7-1/2 7-1/2

N/A N/A

 X_2

24 | 11-5/8 | 24

24 N/A N/A

1" Ø 28"

EDGE DISTANCES (INCHES)

(OPTION 1)

1-3/4 5

1-3/4

2-3/4

HDU/HDQ HOLDOWN DETAILS

HDU14 2-3/4

1-3/4 5

1-3/4 | 14-1/16 | 7-1/2 | 7-1/2

24

HDU4

HDU5

Ø LENGTH MONO 2 POUR

SQUARE

SQUARE

HEAD BOLT

HEAD BOLT ASTM307

| 5/8" Ø | 12-5/8" | SSTB16 | SSTB20

TO DAYLIGHT= "H"/3 AND 7'-0"

SURFACE OF FINISH

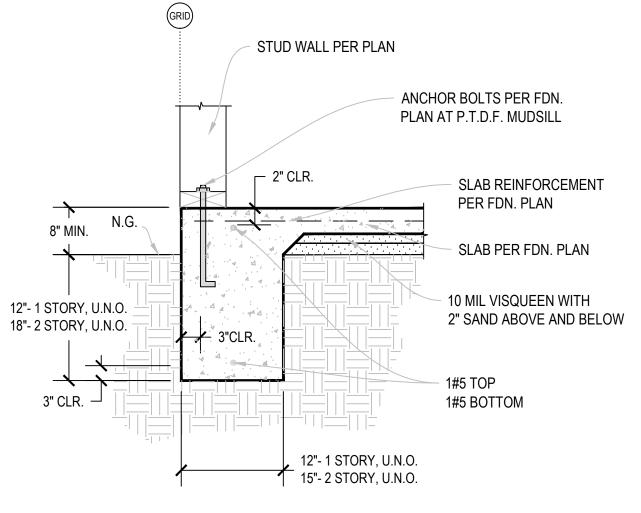
GRADE ADJOINING

FDN. PERIMETER

EXTEND FOOTING

DEPTH AS REQ'D.

TOE OF SLOPE

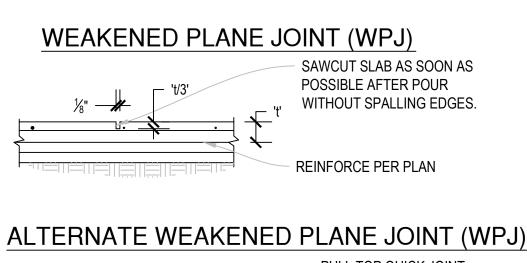


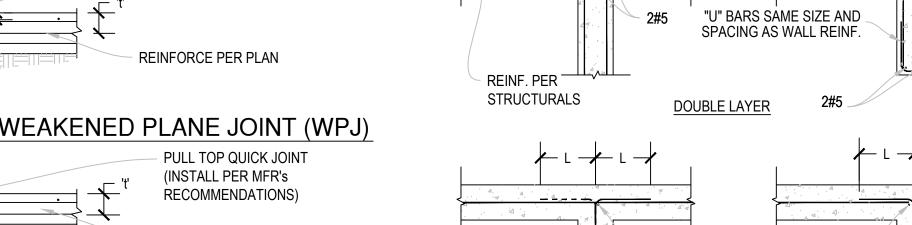
FOOTING, EXTERIOR WALL

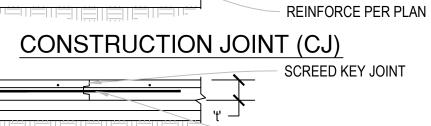
F4.0

FINISH FLOOR

4'-0" x 4'-0" x18"







SLAB JOINTS SHALL OCCUR IN A SQUARE OR RECTANGULAR PATTERN WITH 12'-0" MAX BETWEEN JOINTS EACH WAY UNLESS OTHERWISE NOTED ON PLANS.

L= 30" MINIMUM WHERE PA/PAHD/HPAHD HOLDOWNS OCCURS OTHERWISE L=40 DIAMETERS IN CONCRETE. L=48 DIAMETERS IN MASONRY

#5 TYPICAL

AS SHOWN

SINGLE LAYER

#5 TYPICAL

AS SHOWN

MIN BEND DIA. (PRINCIPAL REINF.)

D = 6db FOR #3 THRU #8

D = 8db FOR #9 THRU #11

D = 10db FOR #14 & #18

#3

CORNER/INTERSECTION REINF. @ WALLS & FTGS.

CONCRETE SLAB JOINTS

#3 x 3'-0" @ 32"

	FOUNDATION SCHE	DULE
PE	DIMENSIONS (1)	RE-BAR
SLAB	4" THICK	#3@18" O.C. (EW)
F1.0	15" x CONTINUOUS x 15"	1 #5 TOP 1 #5 BOTTOM
F2.0	2'-0" x 2'-0" x 18"	(3) #5 (EW)
F2.5	2'-6" x 2'-6" x18"	(4) #5 (EW)
F3.0	3'-0" x 3'-0" x 18"	(4) #5 (EW)
F3.5	3'-6" x 3'-6" x18"	(4) #5 (EW)

(1) DEPTH IS MEASURED INTO FIRM NATURAL GROUND OR PROPERTY COMPACTED FILL.

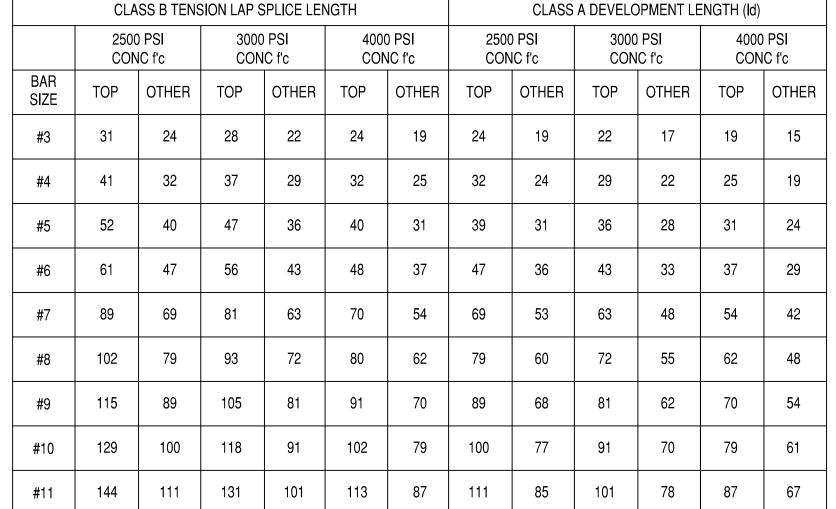
FOOTING SCHEDULE

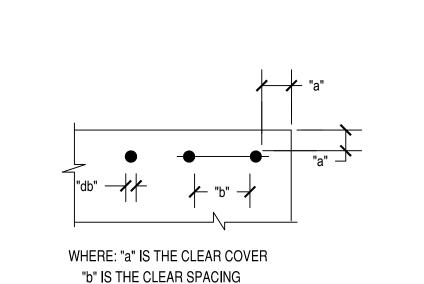
1'-0"

SIZE TO MATCH SLAB REINF.

(5) #5 (EW)

#3@12" O.C.





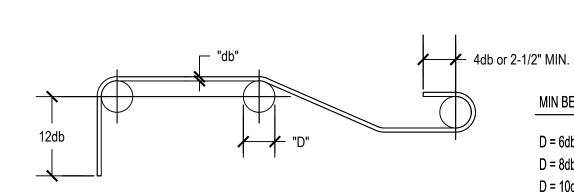
"db" IS THE BAR DIA.

LENGTHS SHOWN ARE IN INCHES.

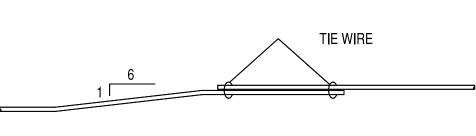
CONC. CAST BELOW THEM.

REQUIREMENTS.

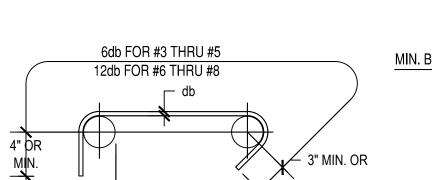
WITHIN A BUNDLE SHALL NOT OVERLAP.



STANDARD HOOK DETAILS FOR PRINCIPAL REINFORCEMENT



OFFSET & SPLICES



#5 2 1/2" #6 | 4 1/2" | STANDARD HOOK DETAILS FOR STIRRUPS & TIES

#4 CONT. **^**— 1'-0" — DEPRESSION 4" OR GREATER (12" MAX.) 1-1/2" CLR. SLAB "T" 7 REINF. SECTION SEE PLAN #4 CONT. ALL ADOLIND EVTEND SLAB "T" 7 REINF. SEE PLAN

AROUND EXTEND
DEPRESSION
4" MAX.
1'-0" 1'-0"
SECTION (D)
<u> </u>

TYP. SLAB ON GRADE DEPRESSION DETAIL

TENSION LAP SPLICE SCHEDULE

COL.S OR WHERE b < 2db FOR OTHER ELEMENTS.

ALL SPLICES SHALL BE CLASS B TENSION LAP SPLICES U.N.O.

INCREASE LENGTHS 30% FOR LIGHT WEIGHT CONC. & AT FOUR BAR

5. TOP BARS - HORIZONTAL BARS PLACED WITH MORE THAN 12" OF FRESH

6. INCREASE LENGTHS 50% WHERE a < db OR WHERE b < db FOR BEAMS &

BUNDLES (WHERE 2 BARS LAP WITH 2 OTHER BARS) INDIVIDUAL BARS

FOR #14 & #18 BARS, USE MECHANICAL SPLICE IN ACCORDANCE WITH CBC

LENGTHS SHOWN ARE FOR GRADE 60 UNCOATED BARS.

TYPICAL REINFORCEMENT DETAILS & TENSION LAP SPLICE SCHEDULE

TYPICAL MIN. BEND DIA. (STIRRUPS & TIES) **FOUNDATION** DETAILS BAR D #4 2" DRAWN BY: CR/CKP

CHECKED BY: CKP

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SAN DIEGO, CA 92109 www.pattersoneng.com

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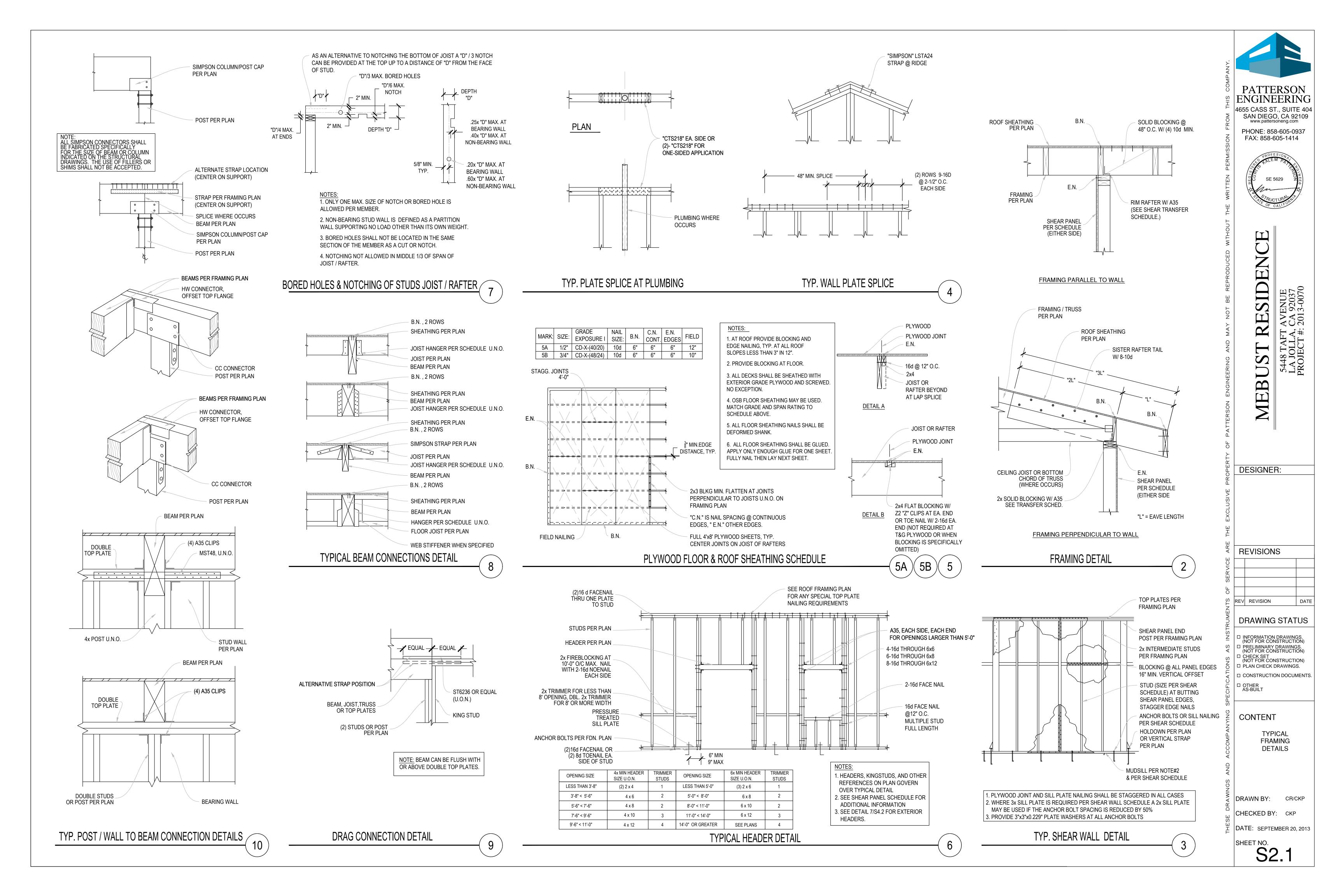
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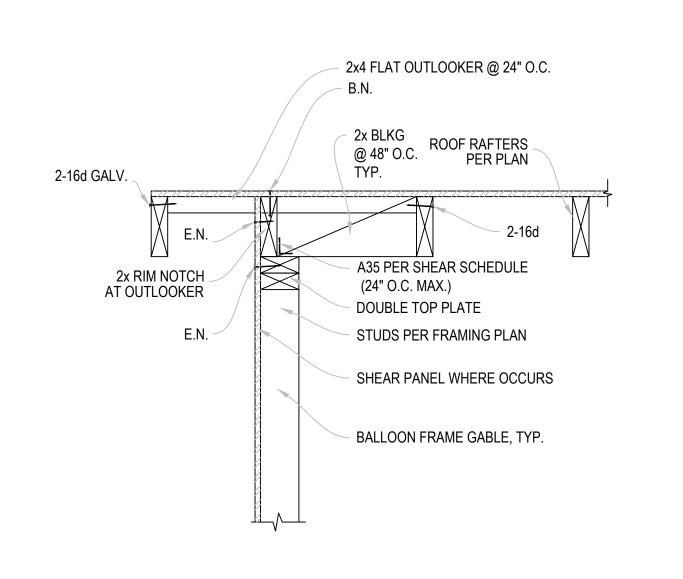
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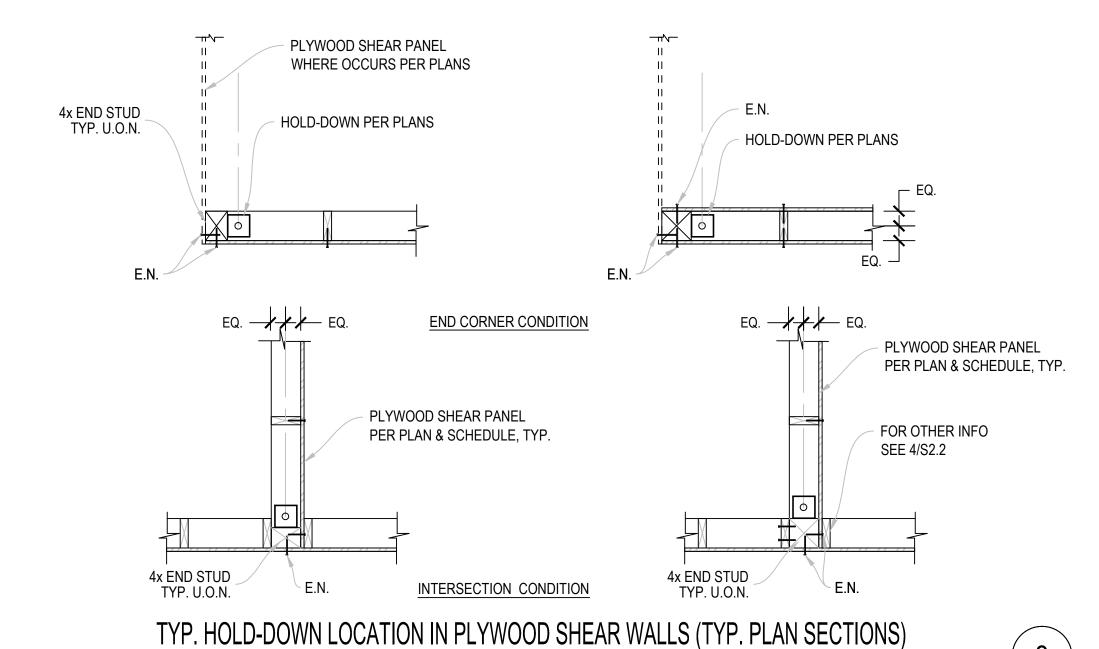
DATE

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5448 TAF LA JOLLA PROJECT







SHEAR TRANSFER, GABLE END

9

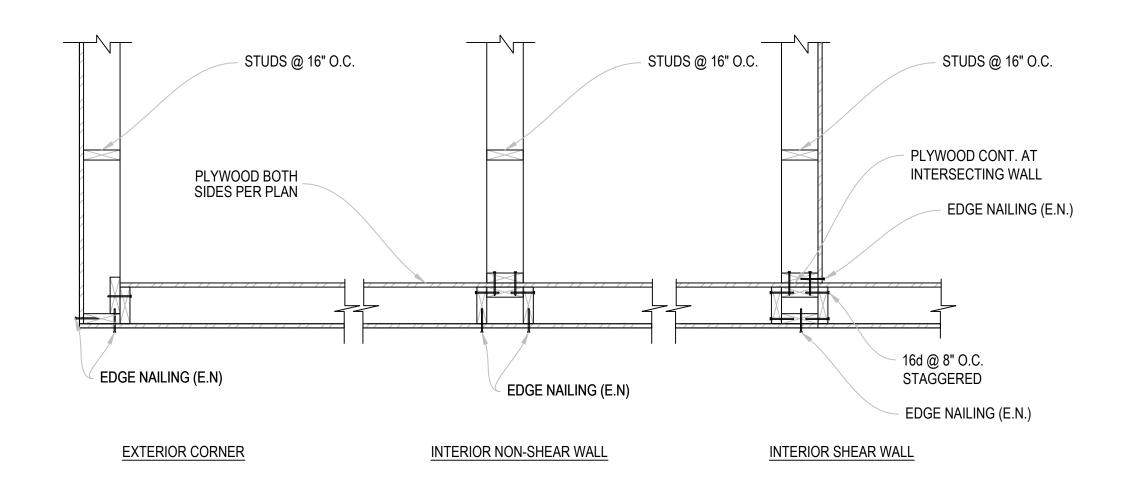
3

ROOF SHEATHING MUST
EXTEND UNDER CALIFORNIA
TO RAFTER/TOP PLATE

SHEATHING B.N.

2x BOARD NOTCH AS REQUIRED
W/ 3-16d PER RAFTER FOLLOWING
LINE OF CRICKET FRAMING
ROOF PLYWOOD

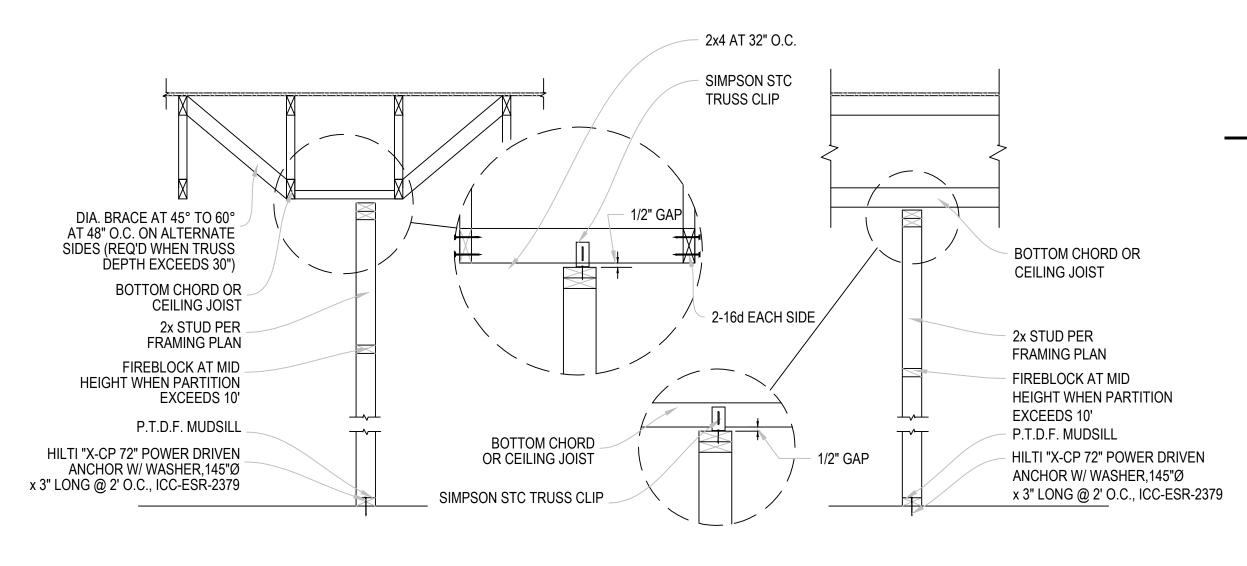
2x CRICKET FRAMING
PERPENDICULAR ROOF FRAMING



CAL-FILL SHEAR TRANSFER

-(10)

STUD WALL INTERSECTIONS-PLAN



<u>PARALLEL</u> <u>PERPENDICULAR</u>

NON-BEARING PARTITION CONNECTION

MEMBER SIZE	FACE MOUNT HANGER	CAPACITY (LBS.)	TOP FLANGE HANGER	CAPACITY (LBS.)
2x4	LUS24	670	HU24TF	2090
2x6	LUS26	865	HU26TF	2275
2x8	LUS22	865	HU28TF	2335
2x10	LUS28	1100	HU210TF	2335
2x12	LUS210	1340	HU212TF	2335
4x6	HUS46	1065	HUS46TF	2700
4x8	HUS48	1595	BA48	3800
4x10	HUS410	2125	BA410	3800
4x12	HUS412	2660	BA412	3800
6x8	HUC68	2085	HU68TF	3500
6x10	HUC610	2680	HU610TF	4150
6x12	HUC612	3275	HU612TF	4550
8x8	HUC88	2085	HW88	5285
8x10	HUC810	2680	HW810	5285
8x12	HUC812	3275	HW812	5285
3-1/2x9-1/2 V.L.	HUS410	2125	LBV3.56/9.5	2590
3-1/2x11-7/8 V.L.	HUS412	2660	BA3.56/11.88	3800
5-1/4x9-1/2 V.L	HUC610	2680	HWU5.5/9.5	6000
5-1/4x11-7/8 V.L.	HUC612	3275	HWU5.5/11.88	6000
7x9-1/2 V.L.	HUC410-2	2680	HWU7.12/9.5	6000
7x11-7/8 V.L.	HUC410-2	3275	HWU7.12/11.88	6000
3-1/8 x 10-1/2 GLB	HUC210-2	2680	WPU	4165
3-1/8 x 12 GLB	HUC212-2	3275	WPU	4165
3-1/8 x 16-1/2 GLB	HUC216-2	3870	WPU	4165
5-1/8 x 10-1/2 GLB	HUC5.125/12	3275	WPU	4165
5-1/8 x 12 GLB	HUC5.125/12	3275	WPU	4165
5-1/8 x 13-1/2 GLB	HUC5.125/13.5	3870	WPU	4165
5-1/8 x 16 GLB	HUC5.125/16	3870	WPU	4165
9-1/2 BCI 5000	IUS2.06/9.5	950	ITS 2.06/9.5	1520
9-1/2 BCI 6000	IUS2.37/9.5	950	ITS 2.37/9.5	1520
9-1/2 BCI 6500	IUS2.56/9.5	950	ITS 2.56/9.5	1520
9-1/2 BCI 60	IUS2.37/9.5	950	ITS 2.37/9.5	1520
9-1/2 BCI 90	IUS3.56/9.5	950	ITS 3.56/9.5	1520
11-7/8 BCI 5000	IUS2.06/11.88	1185	ITS 2.06/11.88	1520
11-7/8 BCI 6000	IUS2.37/11.88	1185	ITS 2.37/11.88	1520
11-7/8 BCI 6500	IUS2.56/11.88	1185	ITS 2.56/11.88	1520
11-7/8 BCI 60	IUS2.37/11.88	1185	ITS 2.37/11.88	1520
11-7/8 BCI 90	IUS3.56/11.88	1420	ITS 3.56/11.88	1520

TYPICAL FRAMING HANGER SCHEDULE

 CEILING JOIST SCHEDULE U.N.O.

 SPACING
 SCHEDULE @ 16"

 SCHEDULE @ 12"
 SIZE
 MAX

 2 x 4
 12" O.C. - 12'-5"
 2 x 4
 16" O.C. - 11'-3"

 2 x 6
 12" O.C. - 19'-6"
 2 x 6
 16" O.C. - 17'-8"

 2 x 8
 12" O.C. - 25'-8"
 2 x 8
 16" O.C. - 23'-0"

 2 x 10
 16" O.C. - 26'-0"
 2 x 10 16" O.C. - 26'-0"

 FOOTNOTES:

 1. REFER TO 2010 I.B.C

 TABLE 2308.10.2(1)
 2. USE DF No 2,L.L. 10 PSF,

TYPICAL CEILING JOIST SCHEDULE

2 x 8 24" O.C.. - 18'-9"

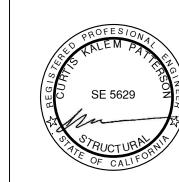
2 x 10 24" O.C. - 22'-11"

D.L. 5 PSF

FOR GARAGE USE

@16" O.C. ONLY.

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ENGINEERING
4655 CASS ST., SUITE 404
SAN DIEGO, CA 92109
www.pattersoneng.com
PHONE: 858-605-0937
FAX: 858-605-1414



RESIDENCE
TAVENUE
A. CA 92037

MEBUST RESID
5448 TAFT AVENUE
LA JOLLA, CA 92037
PROJECT #: 2013-0070

DESIGNER:

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DATE

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CONTENT

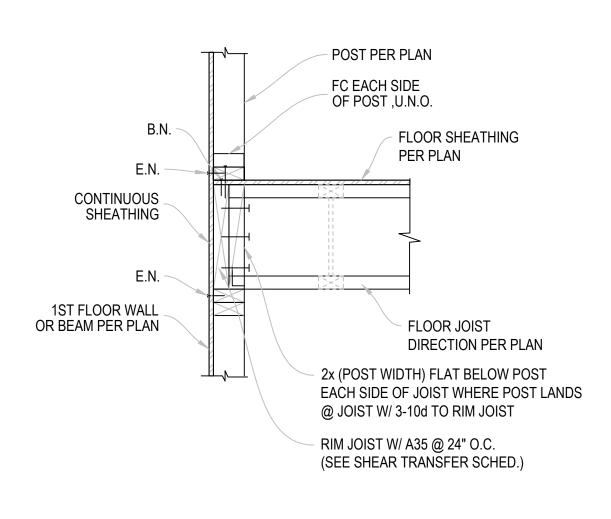
TYPICAL FRAMING DETAILS

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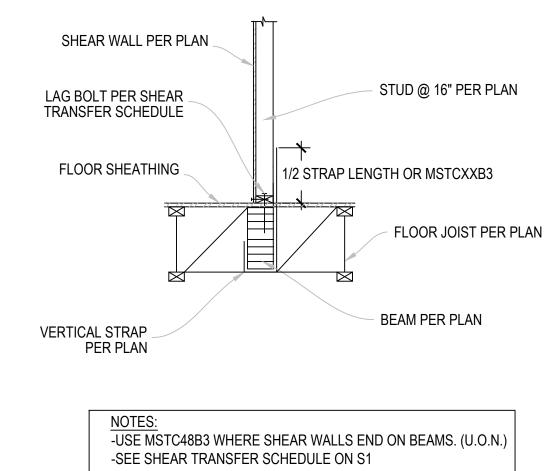
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DATE: SEPTEMBER 20, 2013

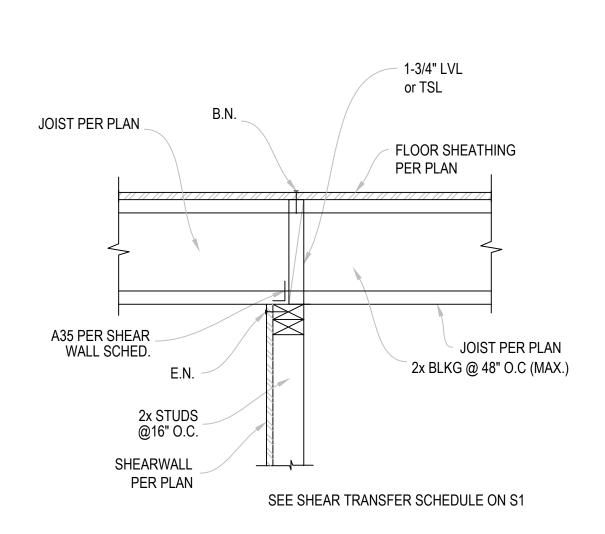
SHEET NO. S2.2

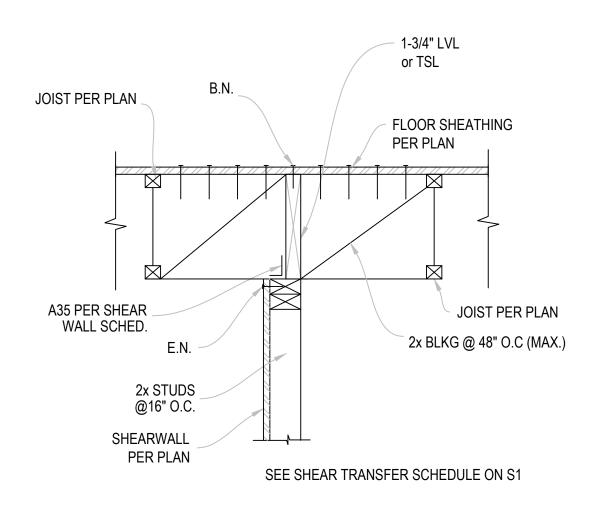




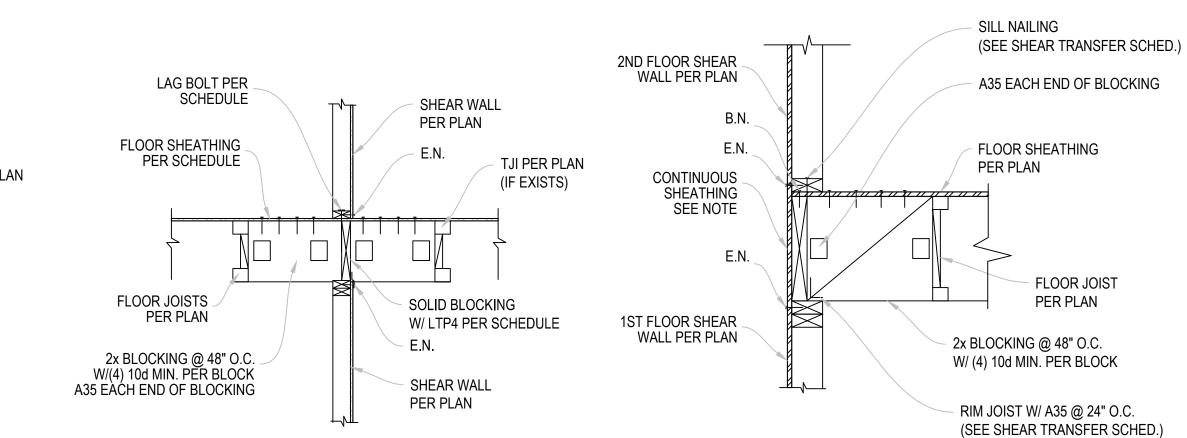




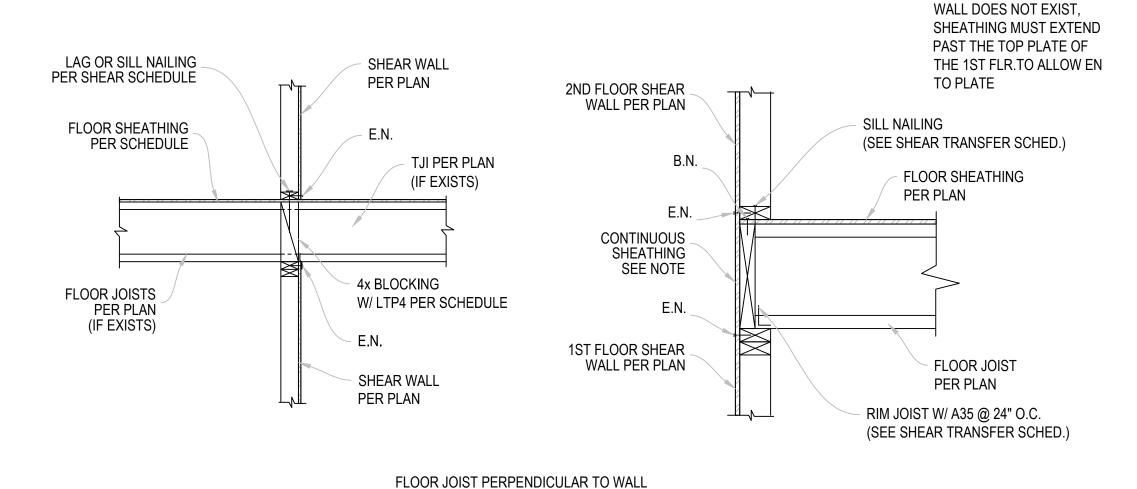




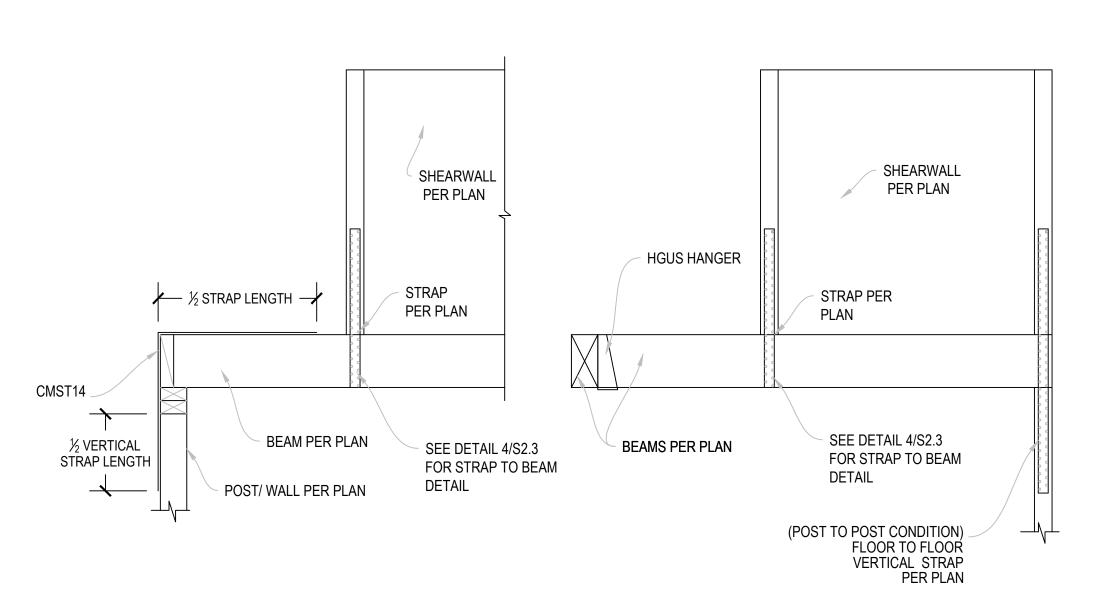




FLOOR JOIST PARALLEL TO WALL



2-STORY SHEAR TRANSFER



FLOOR STRAPPING

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SIDENC

AVENUE CA 92037 : 2013-0070 RE 5448 TAFT / LA JOLLA, (PROJECT #: 2 B ME

NOTE: IF 1ST FLR. SHEAR

DESIGNER:

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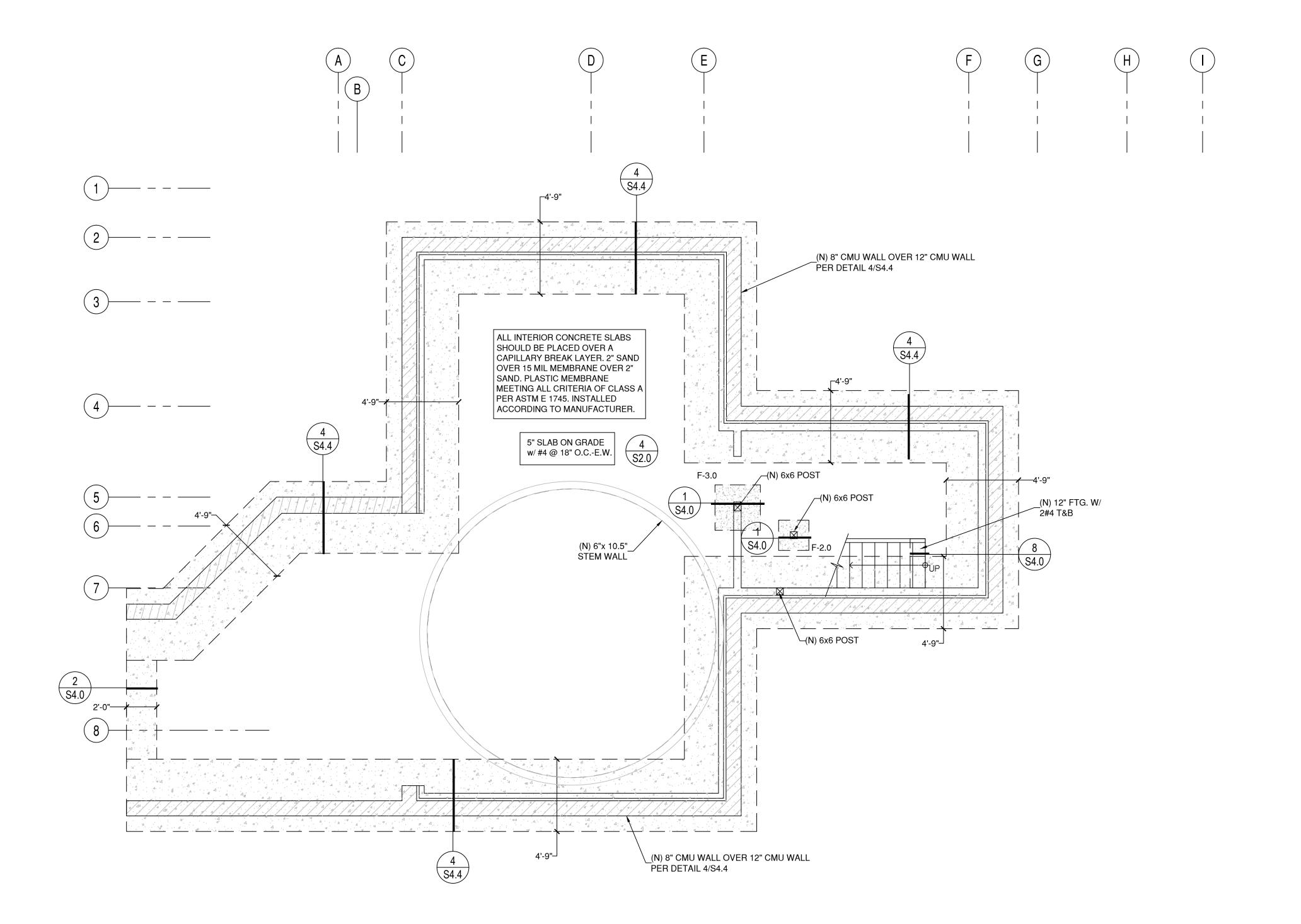
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CONTENT

TYPICAL FRAMING DETAILS

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CHECKED BY: CKP DATE: SEPTEMBER 20, 2013



LEGEND				
POST UP POST DOWN				
(N) BEARING WALLS BELOW WALLS ABOVE (E) BEARING WALLS BELOW NON BEARING WALLS BELOW				
FOOTING PER PLAN	<u> </u>			
SHEAR WALL PER SHEAR PANEL SCHEDULE & 3/S2.1	<u> </u>			
PERFORATED SHEAR WALL	X'-X" 4x8			
NEW HEADER PER DETAIL 6/S2.1	4x8			
THROUGH FLOOR HOLDOWN SEE DETAILS 3/S2.3 & 4/S2.3.	L CSXX MSTXX			
DRAG STRAP PER DETAIL 9/S2.1.	(N) STRAP			
FRAMING PER PLAN				
CALIFORNIA FILL				
BLOCKED FLOOR DIAPHRAGM 4" BOUNDARY NAILING PER 5/S2.1				
TYPICAL	TYP.			
SIMILAR	SIM.			
HOLDOWN PER DETAIL 12/S2.0 & 12/S4.0	[°] HDU			
PAD FOOTING PER PLAN SEE SCHEDULE FOR ADDITIONAL INFORMATION	F-X.X			

12/S4.0.

THE PROPER MATERIAL FOR THE ASSEMBLY.

3. SEE SHEET S-1.1 FOR ADDITIONAL REQUIREMENTS FOR FOUNDATION SPECIAL INSPECTIONS.

4. SEE SHEET S-1.0 FOR ADDITIONAL FOUNDATION NOTES. 5. ARCHITECTS DIMENSIONS TAKE PRECEDENCE. CONTACT THE ENGINEER WITH DISCREPANCIES.

6. SEE SOILS REPORT FOR ADDITIONAL SUBGRADE, WATERPROOFING, AND DRAINAGE REQUIREMENTS. 7. SHEAR WALLS AT THIS LEVEL ARE ABOVE.

NOTES:

1. HOLDOWNS MUST BE TIED INTO PLACE PRIOR TO CALLING FOR INSPECTION. FOR HOLDOWN DETAILS SEE 12/S2.0 &

2. THE SPECIAL INSPECTOR IS TO VERIFY THE DRILLING OF ANY HOLES, THE CLEANLINESS OF THE HOLE, THE MOISTURE IN THE HOLE, MIXING THE EPOXY, THE BRAND OF EPOXY AND

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□ PLAN CHECK DRAWINGS.

□ CONSTRUCTION DOCUMENTS.

CONTENT

GARAGE FOUNDATION PLAN

DRAWN BY: CR/CKP

CHECKED BY: CKP

DATE: SEPTEMBER 20, 2013

SHEET NO.



DIMENSIONS. ALL CONSTRUCTION DIMENSIONS MUST BE VERIFIED WITH THE ARCH. PLANS.

RB-1	4x10
RB-2	4x12
CB-1	4x12
FB-1	4x12
FB-2	6x12
FB-3	5-1/8x12 GLB
FB-4	3-1/2x13.5 GLB
FB-5	5-1/8x13.5 GLB
FB-6	7x14 PSL
FB-7	7x16 PSL
FB-S1	W10x19
FB-S2	W10x30
FB-S3	W10x39
FB-S4	W16x57

LEGEND	
POST UP POST DOWN	
(N) BEARING WALLS BELOW WALLS ABOVE (E) BEARING WALLS BELOW NON BEARING WALLS BELOW	
FOOTING PER PLAN	
SHEAR WALL PER SHEAR PANEL SCHEDULE & 3/S2.1	<u>\(\text{X'-X''} \) \(Times of the content </u>
PERFORATED SHEAR WALL	<u> </u>
NEW HEADER PER DETAIL 6/S2.1	4x8
THROUGH FLOOR HOLDOWN SEE DETAILS 3/S2.3 & 4/S2.3.	CSXX MSTXX
DRAG STRAP PER DETAIL 9/S2.1.	(N) STRAF
FRAMING PER PLAN	
CALIFORNIA FILL	
BLOCKED FLOOR DIAPHRAGM 4" BOUNDARY NAILING PER 5/S2.1	
TYPICAL	TYP.
SIMILAR	SIM.
HOLDOWN PER DETAIL 12/S2.0 & 12/S4.0	[°] _HDU
PAD FOOTING PER PLAN SEE SCHEDULE FOR ADDITIONAL INFORMATION	F-X.X

NOTES:

1. HOLDOWNS MUST BE TIED INTO PLACE PRIOR TO CALLING FOR INSPECTION. FOR HOLDOWN DETAILS SEE 12/S2.0 &

2. THE SPECIAL INSPECTOR IS TO VERIFY THE DRILLING OF ANY HOLES, THE CLEANLINESS OF THE HOLE, THE MOISTURE IN THE HOLE, MIXING THE EPOXY, THE BRAND OF EPOXY AND THE PROPER MATERIAL FOR THE ASSEMBLY.

3. SEE SHEET S-1.1 FOR ADDITIONAL REQUIREMENTS FOR FOUNDATION SPECIAL INSPECTIONS.

4. SEE SHEET S-1.0 FOR ADDITIONAL FOUNDATION NOTES.

5. ARCHITECTS DIMENSIONS TAKE PRECEDENCE. CONTACT THE ENGINEER WITH DISCREPANCIES. 6. SEE SOILS REPORT FOR ADDITIONAL SUBGRADE, WATERPROOFING, AND DRAINAGE REQUIREMENTS.

7. SHEAR WALLS AT THIS LEVEL ARE ABOVE.

REVISIONS

REV REVISION

DESIGNER:

ENGINEERING

4655 CASS ST., SUITE 404 SAN DIEGO, CA 92109 www.pattersoneng.com

PHONE: 858-605-0937 FAX: 858-605-1414

DRAWING STATUS

☐ INFORMATION DRAWINGS. (NOT FOR CONSTRUCTION) ☐ PRELIMINARY DRAWINGS. (NOT FOR CONSTRUCTION) CHECK SET (NOT FOR CONSTRUCTION) ☐ PLAN CHECK DRAWINGS.

□ CONSTRUCTION DOCUMENTS.

CONTENT

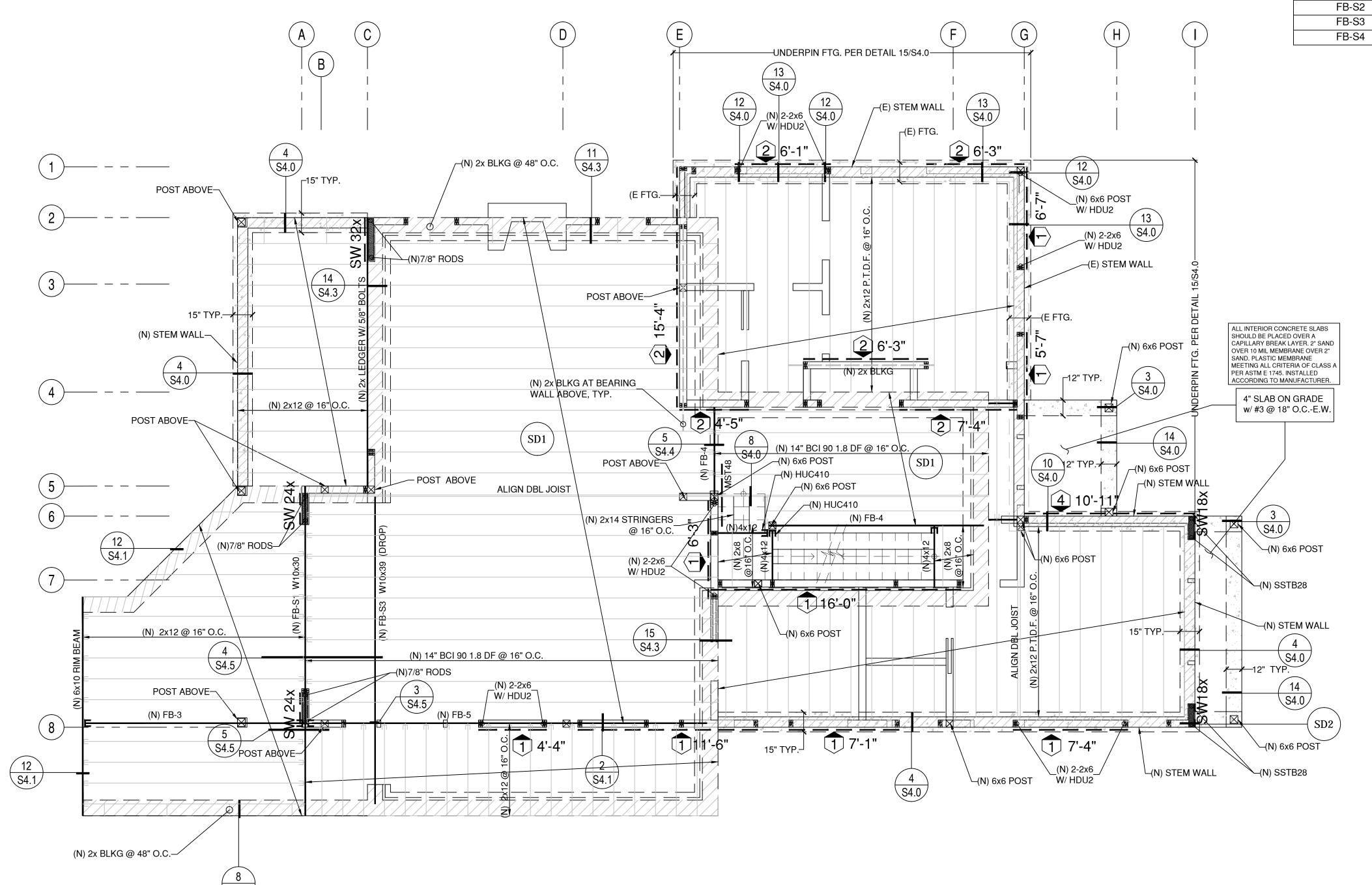
FIRST FLOOR FOUNDATION PLAN

DRAWN BY: CR/CKP

CHECKED BY: CKP

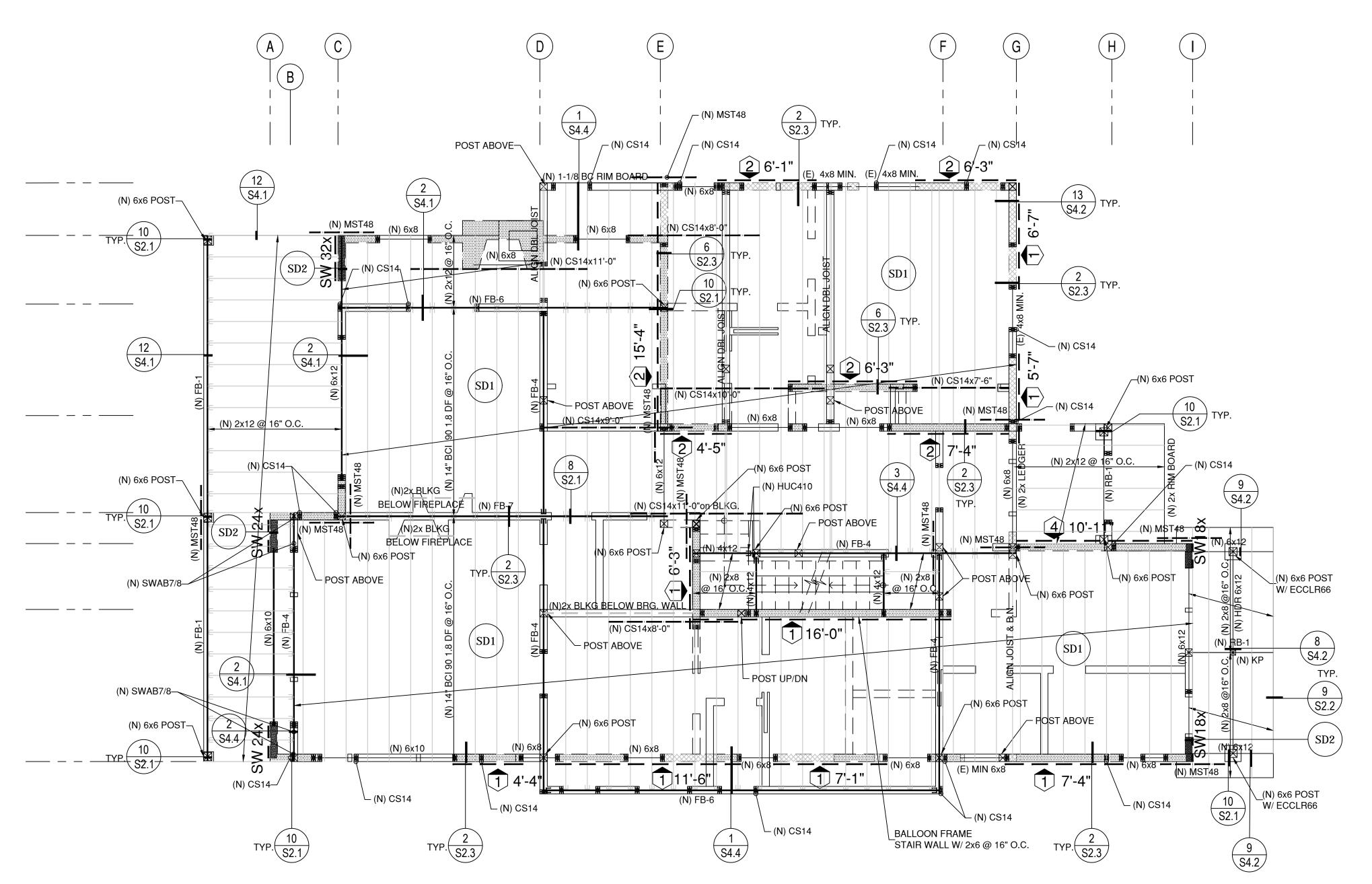
DATE: SEPTEMBER 20, 2013

SHEET NO.



1/4" = 1'-0" DO NOT SCALE PLANS FOR CONSTRUCTION DIMENSIONS. ALL CONSTRUCTION DIMENSIONS MUST BE VERIFIED WITH THE ARCH. PLANS.

BEAM SCHEDULE			
RB-1	4x10		
RB-2	4x12		
CB-1	4x12		
FB-1	4x12		
FB-2	6x12		
FB-3	5-1/8x12 GLB		
FB-4	3-1/2x13.5 GLB		
FB-5	5-1/8x13.5 GLB		
FB-6	7x14 PSL		
FB-7	7x16 PSL		
FB-S1	W10x19		
FB-S2	W10x30		
FB-S3	W10x39		
FB-S4	W16x57		
	•		



S3.2

FIRST FLOOR FRAMING PLAN

1/4" = 1'-0"

DO NOT SCALE PLANS FOR CONSTRUCTION DIMENSIONS. ALL CONSTRUCTION DIMENSIONS MUST BE VERIFIED WITH THE ARCH. PLANS.

LEGEND	
POST UP POST DOWN	
(N) BEARING WALLS BELOW WALLS ABOVE (E) BEARING WALLS BELOW	
NON BEARING WALLS BELOW	
FOOTING PER PLAN	
SHEAR WALL PER SHEAR PANEL SCHEDULE & 3/S2.1	<u>X'-X"</u>
PERFORATED SHEAR WALL PER 1/S4.1	<u>X'-X"</u>
NEW HEADER PER DETAIL 6/S2.1	4x8
THROUGH FLOOR HOLDOWN SEE DETAILS 3/S2.3 & 4/S2.3.	CSX MSTXX
DRAG STRAP PER DETAIL 9/S2.1.	(N) STRA
FRAMING PER PLAN	
CALIFORNIA FILL PER DETAIL 10/S2.2	
BLOCKED FLOOR DIAPHRAGM 4" BOUNDARY NAILING PER 5/S2.1	
TYPICAL	TYP.
SIMILAR	SIM.
HOLDOWN PER DETAIL 12/S2.0 & 12/S4.0	[°] HDl
PAD FOOTING PER PLAN SEE SCHEDULE FOR ADDITIONAL INFORMATION	F-X.X

GENERAL NOTES.

1.- ALL LOAD BEARING CEILING BEAMS & FLOOR BEAMS SHALL HAVE A MINIMUM OF 4x4 DF#1 POSTS WITH SIMPSON FC4 @ TOP & BOTTOM PLATES TO POST CONNECTIONS. SIZE OF POST IS DETERMINED BY THE WIDTH OF THE BEAM BEING SUPPORTED. (UNLESS OTHERWISE NOTED IN PLAN OR DETAIL) ALL ISOLATED POST SHALL HAVE SIMPSON CC OR ECC & CB CONNECTORS.

2.- DIAPHRAGM SHEATHING NAILS OR OTHER APPROVED SHEATHING CONNECTORS SHALL BE DRIVEN SO THAT THEIR HEAD OR CROWN IS FLUSH WITH THE SURFACE OF THE SHEATHING.

- 3.- BOTH SIDES OF EACH SHEAR WALL SHALL HAVE A MINIMUM OF A VERTICAL MST48 INSTALLED AT THE FLOOR LEVEL, U.N.O.
- 4.- LAP SPLICE PLATES A MIN OF 48" W/ 2 ROWS 9-16d @ 2-1/2" O.C. OR USE MST48 W/16d, SEE DETAIL 4/S2.1.
- 5.- FASTEN COILED STRAPS A MINIMUM OF 3'-0" TO ADJOINING SHEAR WALLS
- 6.- FOR BLOCKED DIAPHRAGMS WITH NAILING LESS THAN 4": FRAMING AT ADJOINING PANEL EDGES SHALL BE 3" NOMINAL OR WIDER AND NAILS SHALL BE STAGGERED.
- 7.- REFER TO DETAIL 2/S2.2 FOR CEILING JOIST SCHEDULE.
- 8.- REFER TO DETAIL 1/S2.2 FOR FRAMING HANGER SCHEDULE.

COMPANY.		
0	PATTER	RSON
THIS	ENGINE	ERIN
	4655 CASS ST.,	
FROM	SAN DIEGO, www.patterson	CA 92109 eng.com
	PHONE: 858-	
Z O	FAX: 858-60	5-1414



AND CALIFORNIA

5448 TAFT AVENUE LA JOLLA, CA 92037

DESIGNER:

REVISIONS

DRAWING STATUS

REV REVISION

 □ INFORMATION DRAWINGS. (NOT FOR CONSTRUCTION)
 □ PRELIMINARY DRAWINGS. (NOT FOR CONSTRUCTION)
 □ CHECK SET (NOT FOR CONSTRUCTION)
 □ PLAN CHECK DRAWINGS.

☐ CONSTRUCTION DOCUMENTS.

□ OTHER AS-BUILT

CONTENT

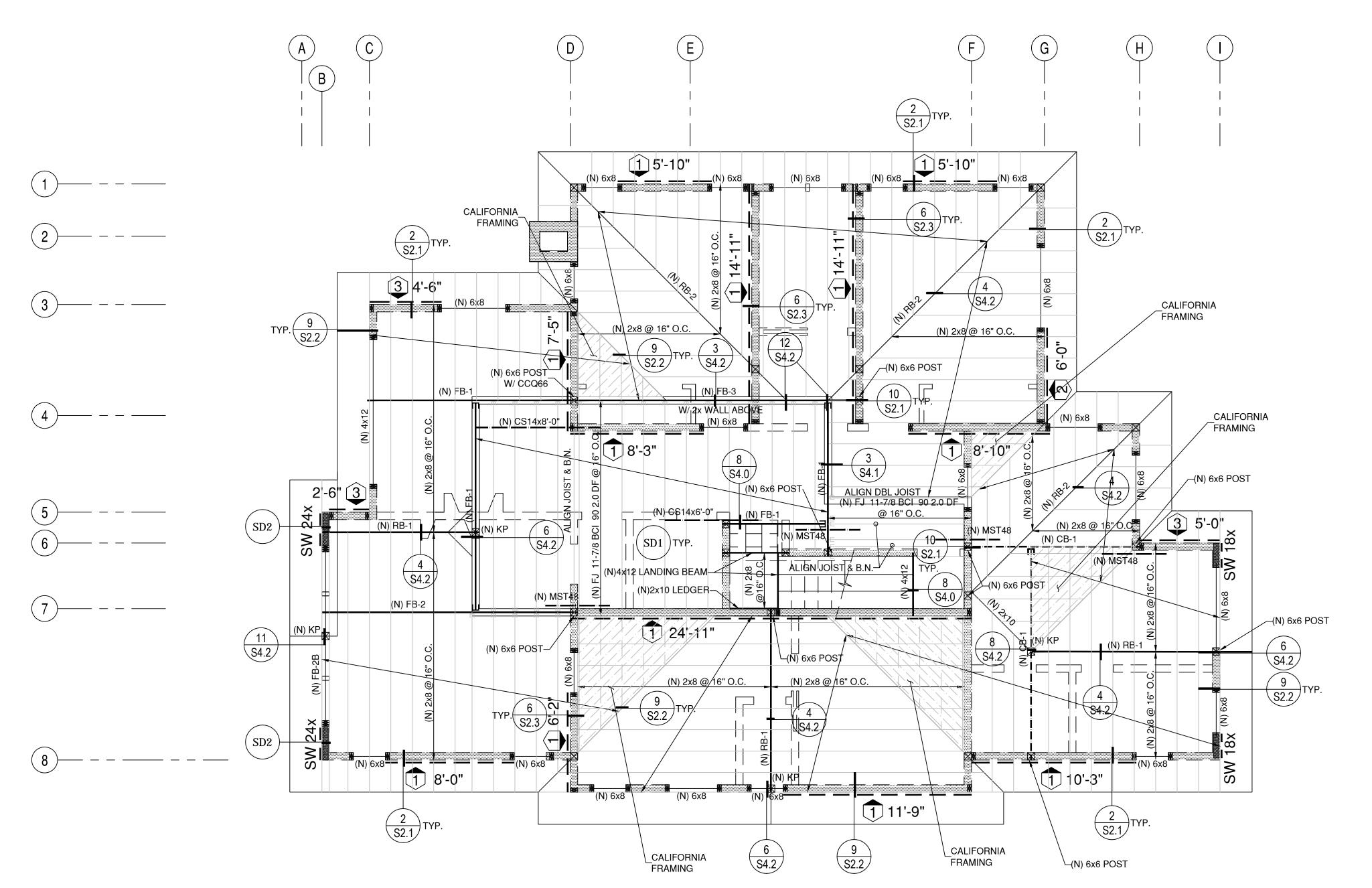
FIRST FLOOR FRAMING PLAN

DRAWN BY: CR/CKP
CHECKED BY: CKP

DATE: SEPTEMBER 20, 2013
SHEET NO.

S3.2

BEAM SC	HEDULE
RB-1	4x10
RB-2	4x12
CB-1	4x12
FB-1	4x12
FB-2	6x12
FB-3	5-1/8x12 GLB
FB-4	3-1/2x13.5 GLB
FB-5	5-1/8x13.5 GLB
FB-6	7x14 PSL
FB-7	7x16 PSL
FB-S1	W10x19
FB-S2	W10x30
ED 00	14/4/05/00



LEGEND	
POST UP POST DOWN	
(N) BEARING WALLS BELOW WALLS ABOVE (E) BEARING WALLS BELOW NON BEARING WALLS BELOW	
FOOTING PER PLAN	
SHEAR WALL PER SHEAR PANEL SCHEDULE & 3/S2.1	<u>X'-X"</u>
PERFORATED SHEAR WALL PER 1/S4.1	<u> </u>
NEW HEADER PER DETAIL 6/S2.1	4x8
THROUGH FLOOR HOLDOWN SEE DETAILS 3/S2.3 & 4/S2.3.	CSXX MSTXX
DRAG STRAP PER DETAIL 9/S2.1.	(N) STRAP
FRAMING PER PLAN	
CALIFORNIA FILL PER DETAIL 10/S2.2	
BLOCKED FLOOR DIAPHRAGM 4" BOUNDARY NAILING PER 5/S2.1	
TYPICAL	TYP.
SIMILAR	SIM.
HOLDOWN PER DETAIL 12/S2.0 & 12/S4.0	°_ HDU
PAD FOOTING PER PLAN SEE SCHEDULE FOR ADDITIONAL INFORMATION	F-X.X ()

GENERAL NOTES.

1.- ALL LOAD BEARING CEILING BEAMS & FLOOR BEAMS SHALL HAVE A MINIMUM OF 4x4 DF#1 POSTS WITH SIMPSON FC4 @ TOP & BOTTOM PLATES TO POST CONNECTIONS. SIZE OF POST IS DETERMINED BY THE WIDTH OF THE BEAM BEING SUPPORTED. (UNLESS OTHERWISE NOTED IN PLAN OR DETAIL) ALL ISOLATED POST SHALL HAVE SIMPSON CC

3.- BOTH SIDES OF EACH SHEAR WALL SHALL HAVE A MINIMUM OF A VERTICAL MST48 INSTALLED AT THE FLOOR

4.- LAP SPLICE PLATES A MIN OF 48" W/ 2 ROWS 9-16d @

5.- FASTEN COILED STRAPS A MINIMUM OF 3'-0" TO ADJOINING SHEAR WALLS

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OR ECC & CB CONNECTORS.

2.- DIAPHRAGM SHEATHING NAILS OR OTHER APPROVED SHEATHING CONNECTORS SHALL BE DRIVEN SO THAT THEIR HEAD OR CROWN IS FLUSH WITH THE SURFACE OF THE SHEATHING.

LEVEL, U.N.O.

2-1/2" O.C. OR USE MST48 W/16d, SEE DETAIL 4/S2.1.

SCHEDULE.

SAN DIEGO, CA 92109 www.pattersoneng.com PHONE: 858-605-0937 FAX: 858-605-1414

PATTERSON ENGINEERING

4655 CASS ST., SUITE 404

DESIGNER:

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☐ CONSTRUCTION DOCUMENTS.

CONTENT

SECOND FLOOR ROOF / DECK FRAMING PLAN

DRAWN BY: CR/CKP

CHECKED BY: CKP

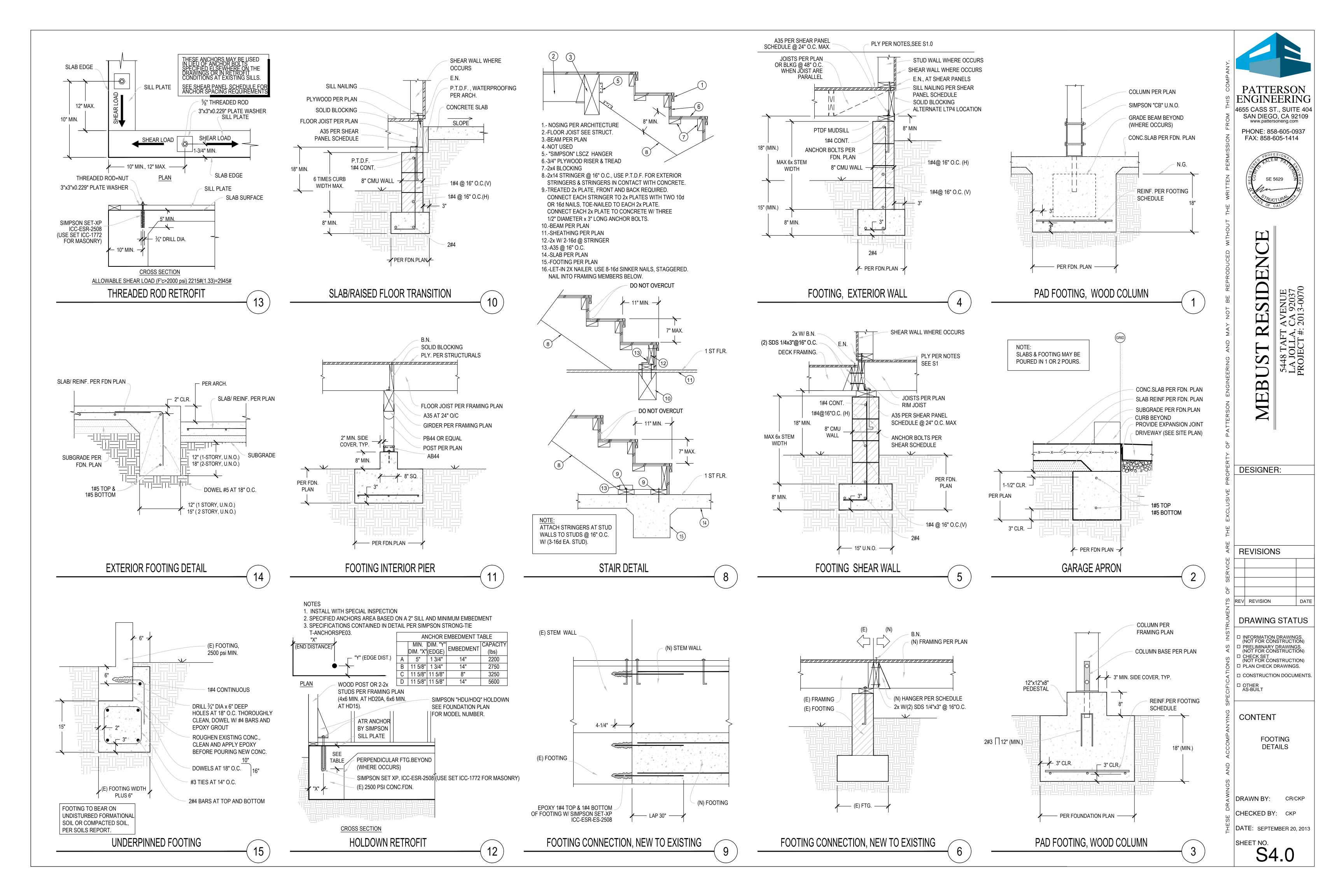
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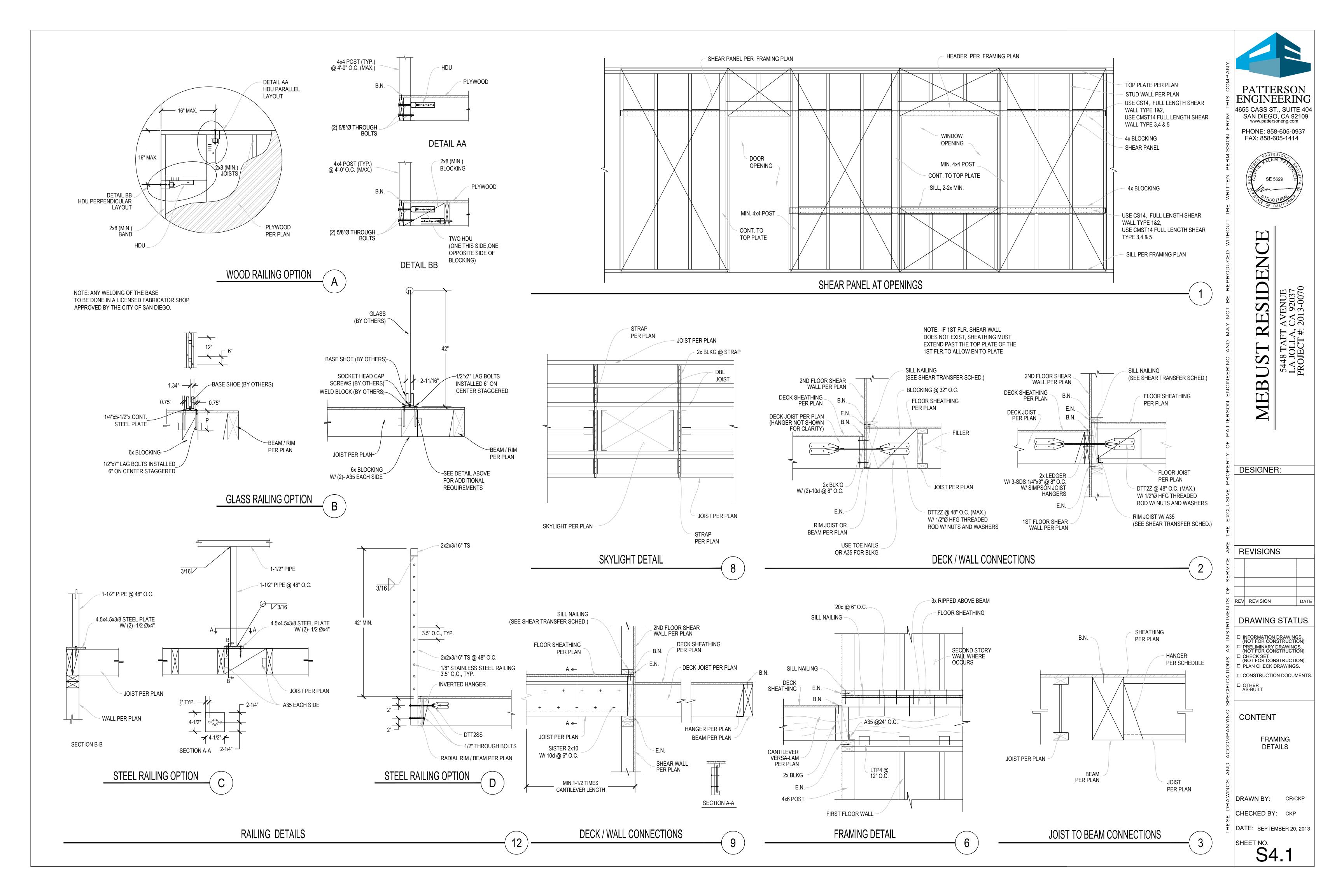
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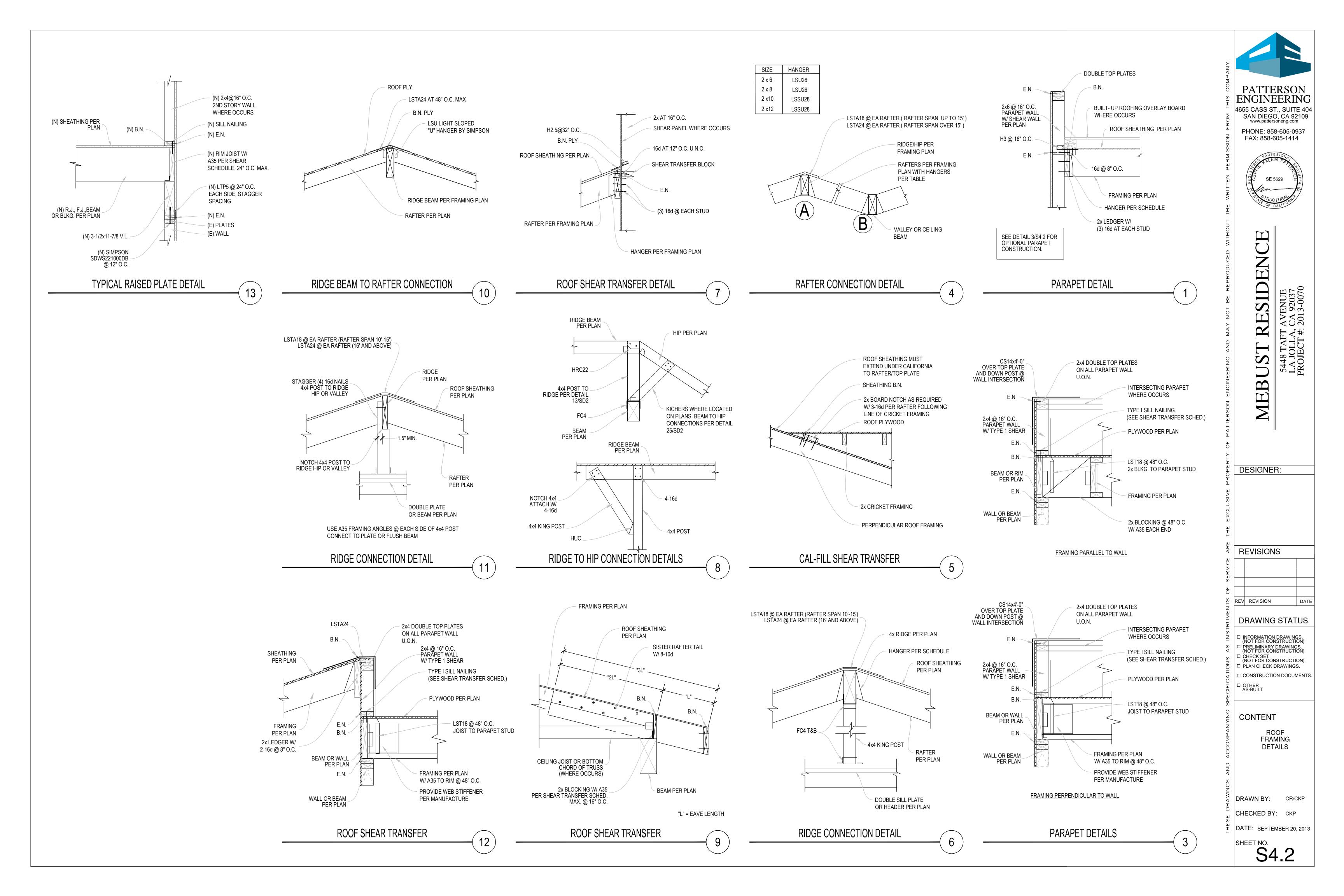
S3.3

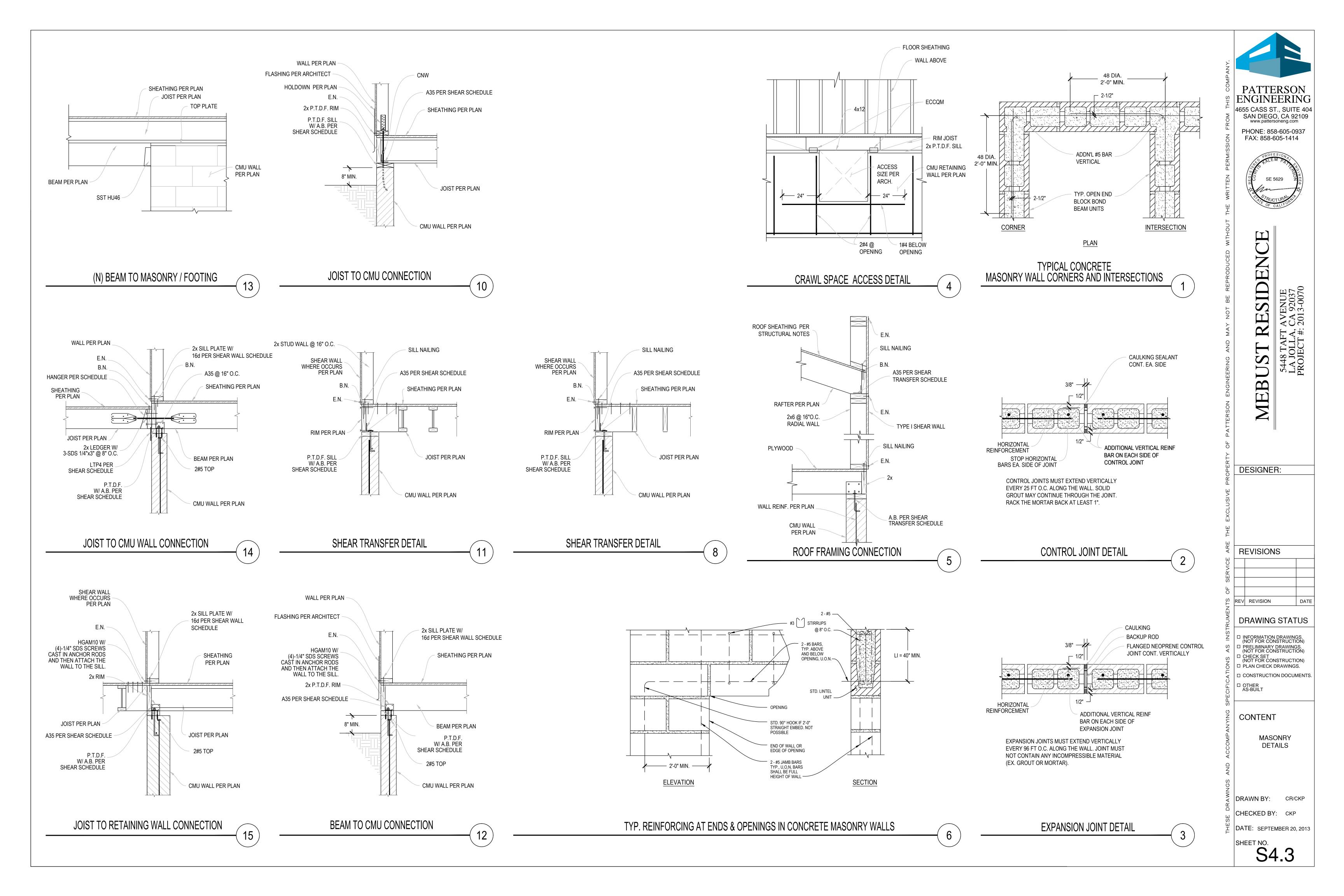
SECOND FLOOR ROOF DECK FRAMING PLAN

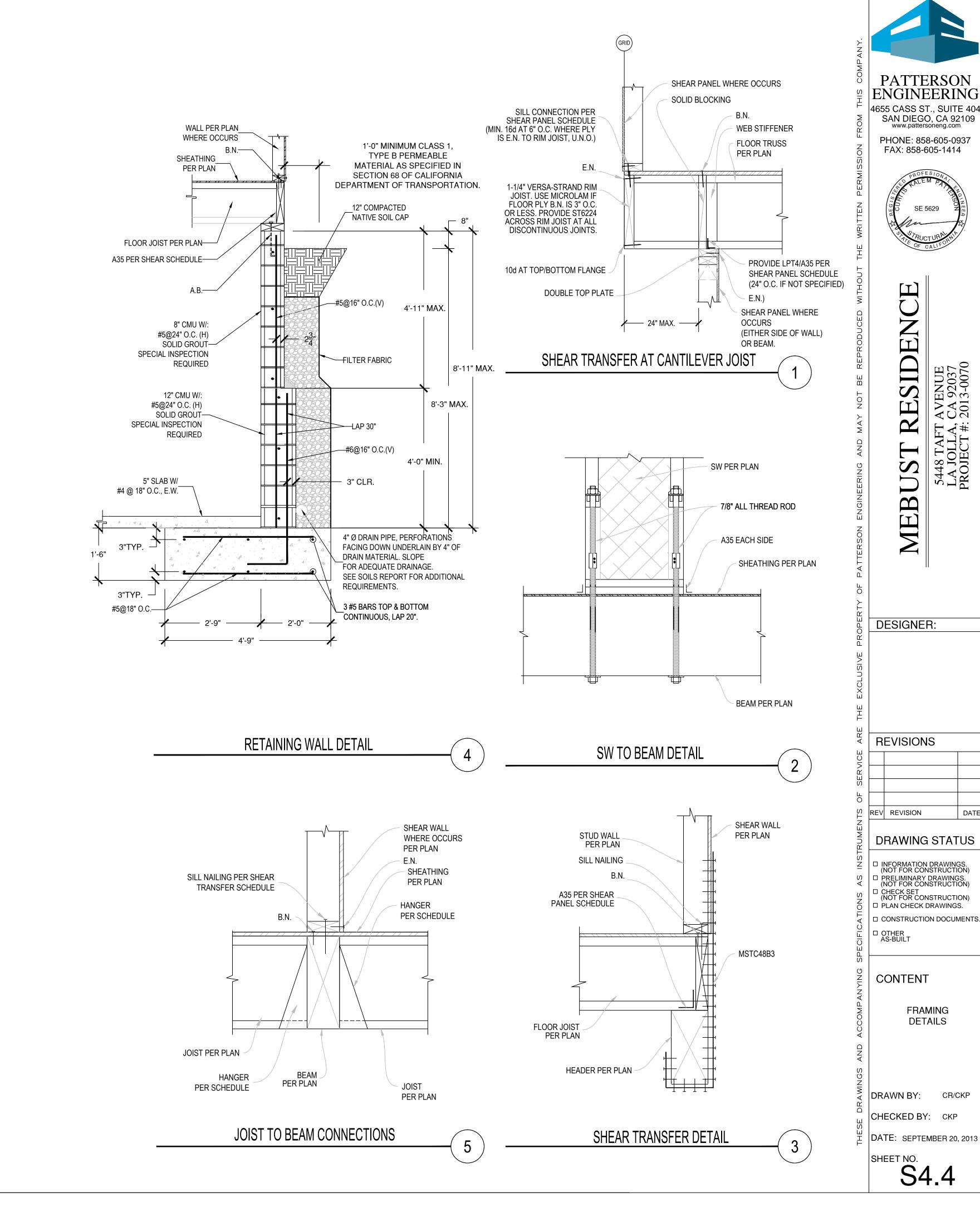
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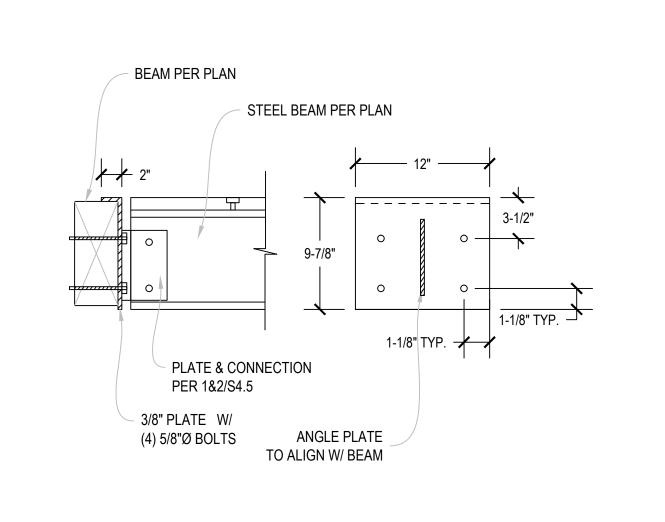




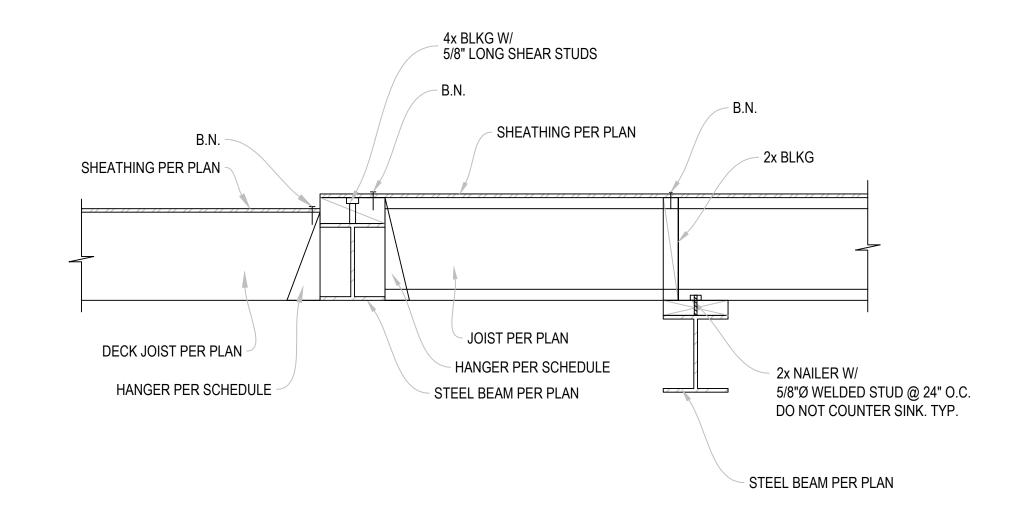


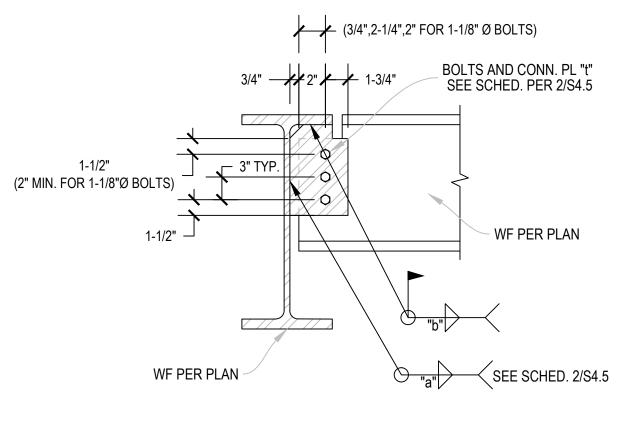
PATTERSON ENGINEERING 4655 CASS ST., SUITE 404 SAN DIEGO, CA 92109 www.pattersoneng.com

DATE



BEAM TO BEAM CONNECTION





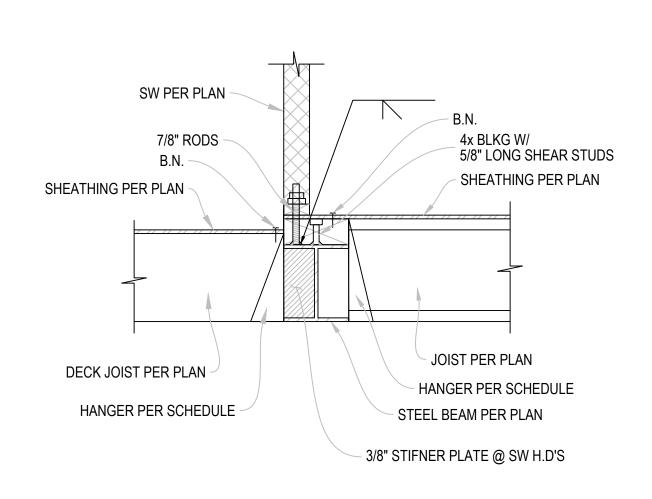
FRAMING TO W BEAM CONNECTION

BEAM TO BEAM CONNECTION

STEEL BEAM PER PLAN

2x NAILER

(4) - 3/4"Ø M.B.



BEAM / GIRDER SIZE	NUMBER & SIZE OF A325X BOLTS U.N.O.		CONNECTION PLATE A-36 U.N.O.		
			PLATE	WELD	WELD
	ROW 1	ROW 2	THICKNESS "t"	"a"	"b"
W6x, C6	1-7/8"Ø	1-7/8"Ø	3/8"	1/4"	1/4"
C8x, C10x, W8x, W10x	2-7/8"Ø		3/8"	1/4"	1/4"
W12x, W14x, C12x	3-7/8"Ø		1/2"	5/16"	5/16"
W16x, W18x	4-7/8"Ø		1/2"	5/16"	5/16"
W21x	5-7/8"Ø		1/2"	5/16"	5/16"
W24x	6-7/8"Ø		1/2"	5/16"	5/16"

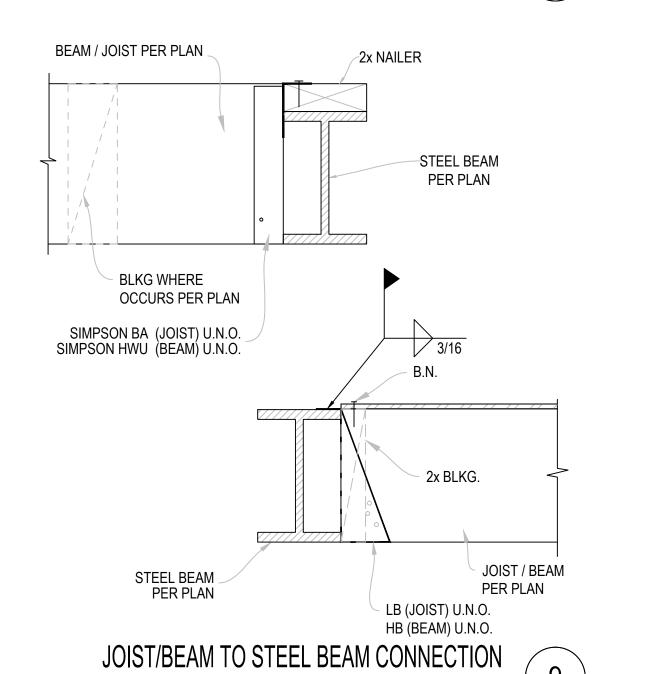
1. ALL CONNECTIONS OF PRIMARY FRAMING BEAMS TO COLUMNS ARE TO DEVELOP 50% OF BEAM SHEAR CAPACITY (MIN.) U.N.O.

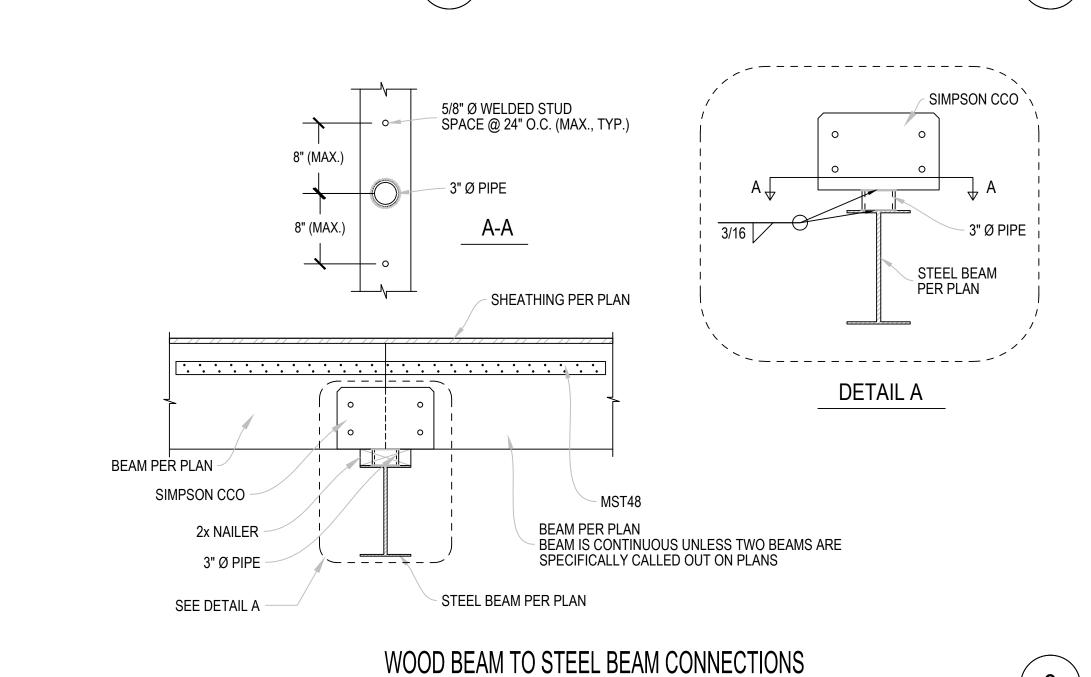
2. ALL FILLET WELD SIZES SHOWN ARE MINIMUM WELD SIZE, WHERE WELD SIZE SHOWN ARE SMALLER THAN AWS MINIMUM WELD SIZE, AWS MINIMUM WELD SIZE SHALL BE USED.



SW TO BEAM CONNECTION



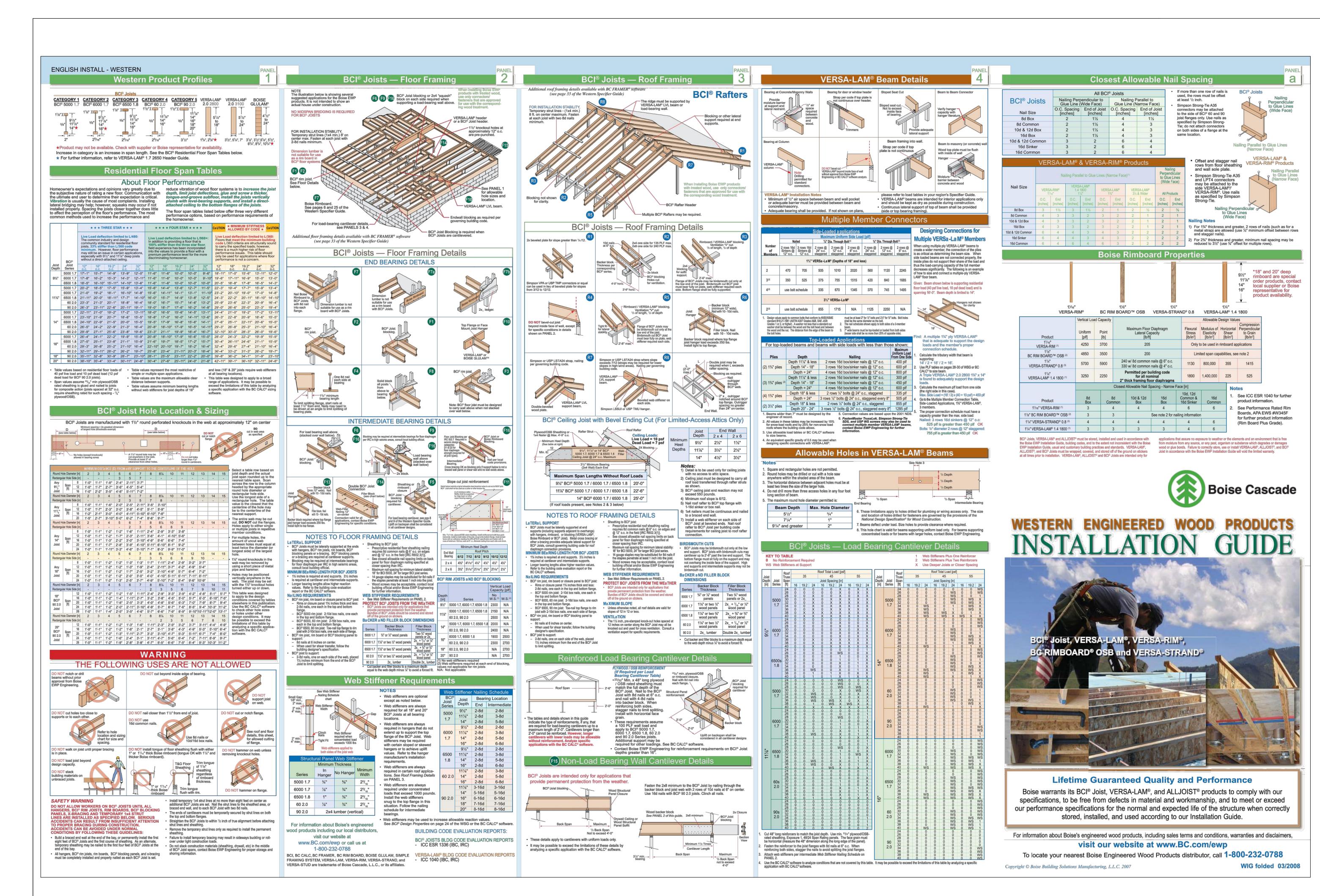




PATTERSON ENGINEERING 4655 CASS ST., SUITE 404 SAN DIEGO, CA 92109 www.pattersoneng.com PHONE: 858-605-0937 FAX: 858-605-1414 RESIDENCE 5448 TAFT AVENUE LA JOLLA, CA 92037 PROJECT #: 2013-0070 MEB DESIGNER: REVISIONS REV REVISION DATE DRAWING STATUS ☐ INFORMATION DRAWINGS. (NOT FOR CONSTRUCTION)
☐ PRELIMINARY DRAWINGS. (NOT FOR CONSTRUCTION) □ CHECK SET
(NOT FOR CONSTRUCTION)
□ PLAN CHECK DRAWINGS. ☐ CONSTRUCTION DOCUMENTS. CONTENT STEEL **DETAILS** DRAWN BY: CR/CKP CHECKED BY: CKP DATE: SEPTEMBER 20, 2013

SHEET NO.

S4.5



ENGINEERING 4655 CASS ST., SUITE 404

SAN DIEGO, CA 92109

PHONE: 858-605-0937 FAX: 858-605-1414

[L]

DESIGNER:

REVISIONS

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□ CONSTRUCTION DOCUMENTS

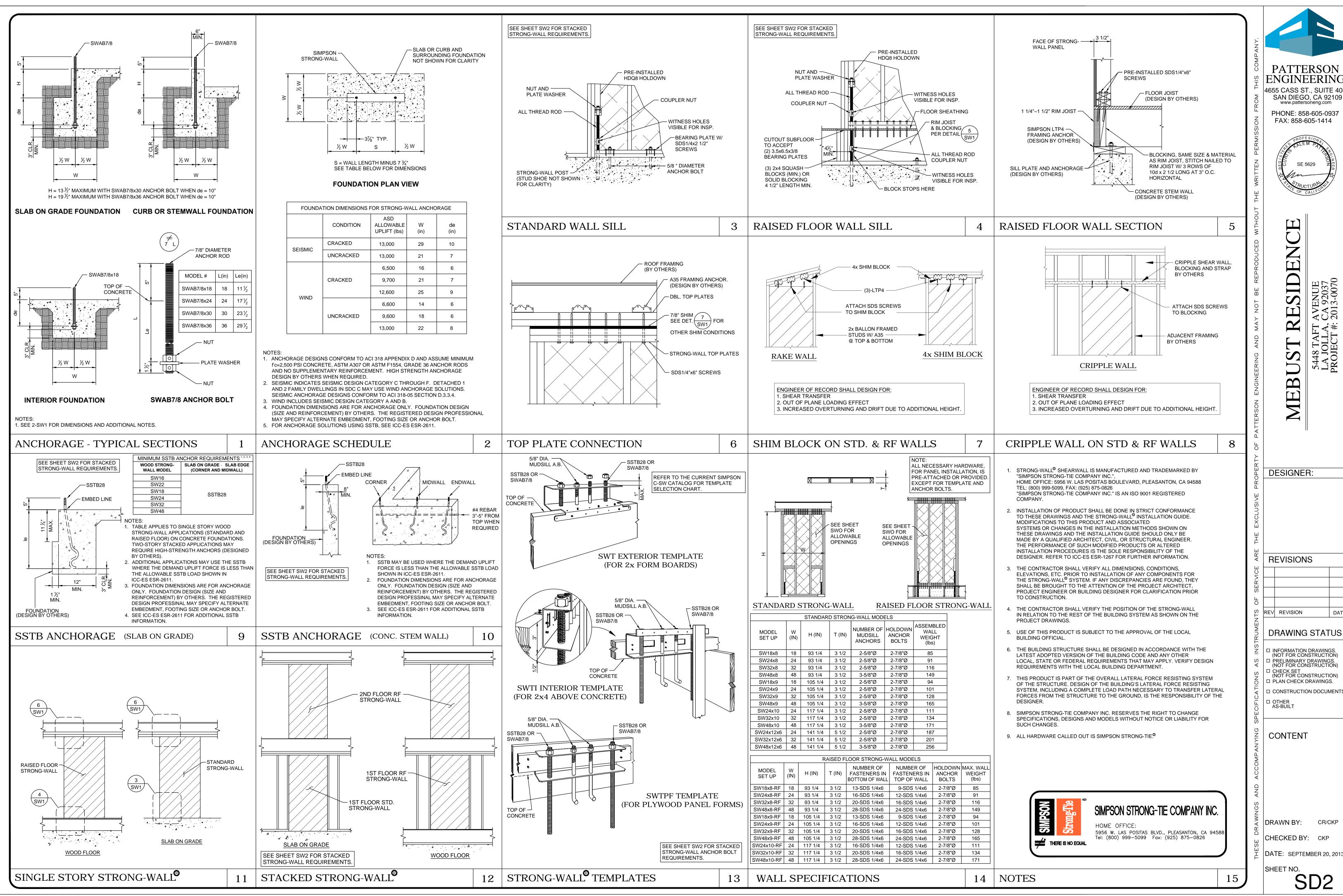
CONTENT

BCI DETAILS

DRAWN BY: CR/CKP

CHECKED BY: CKP

DATE: SEPTEMBER 20, 2013



ENGINEERING 4655 CASS ST., SUITE 404



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DESIGNER:

REVISIONS

DATE

DRAWING STATUS

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CONTENT

DRAWN BY: CR/CKP

CHECKED BY: CKP DATE: SEPTEMBER 20, 2013