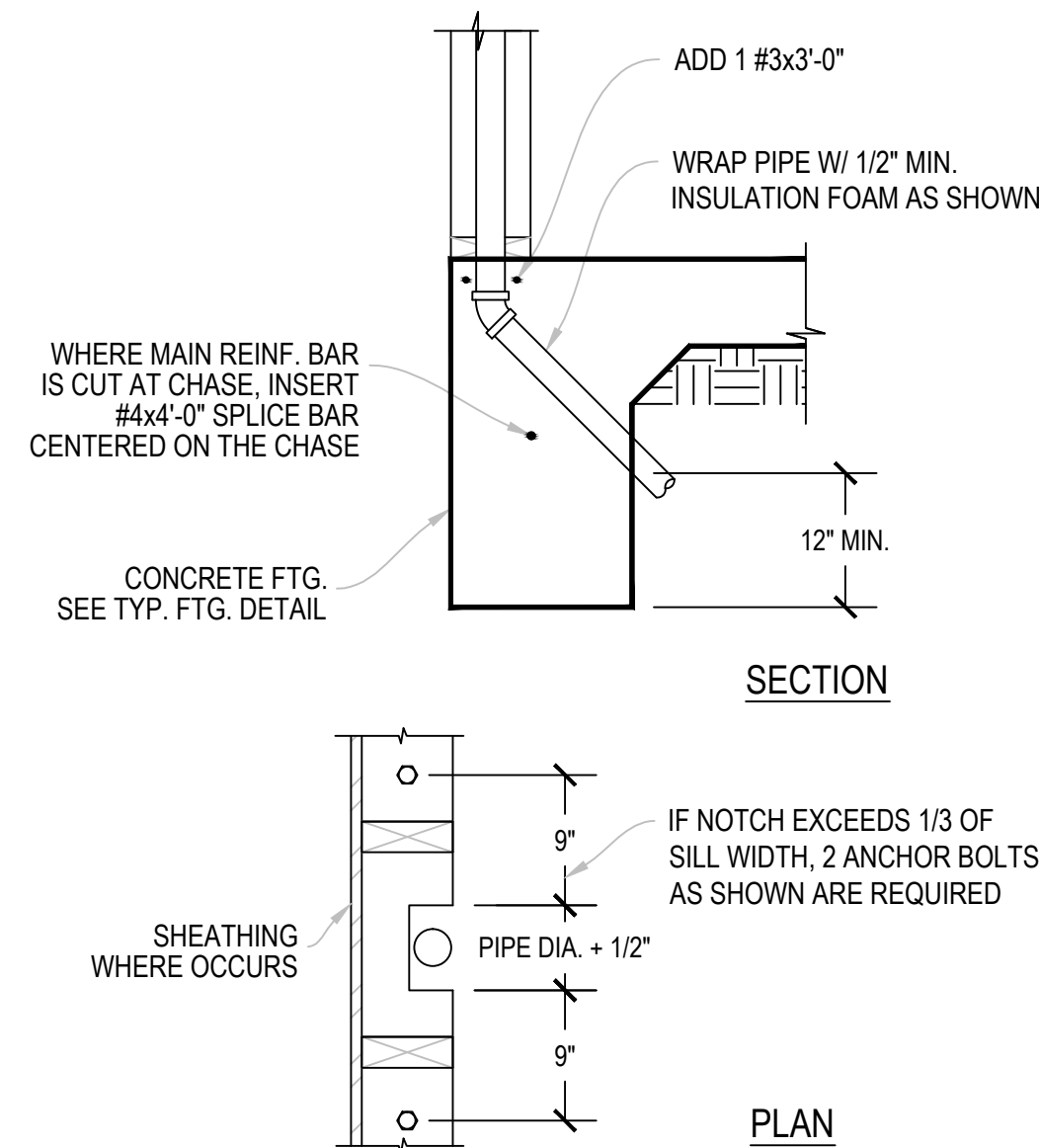
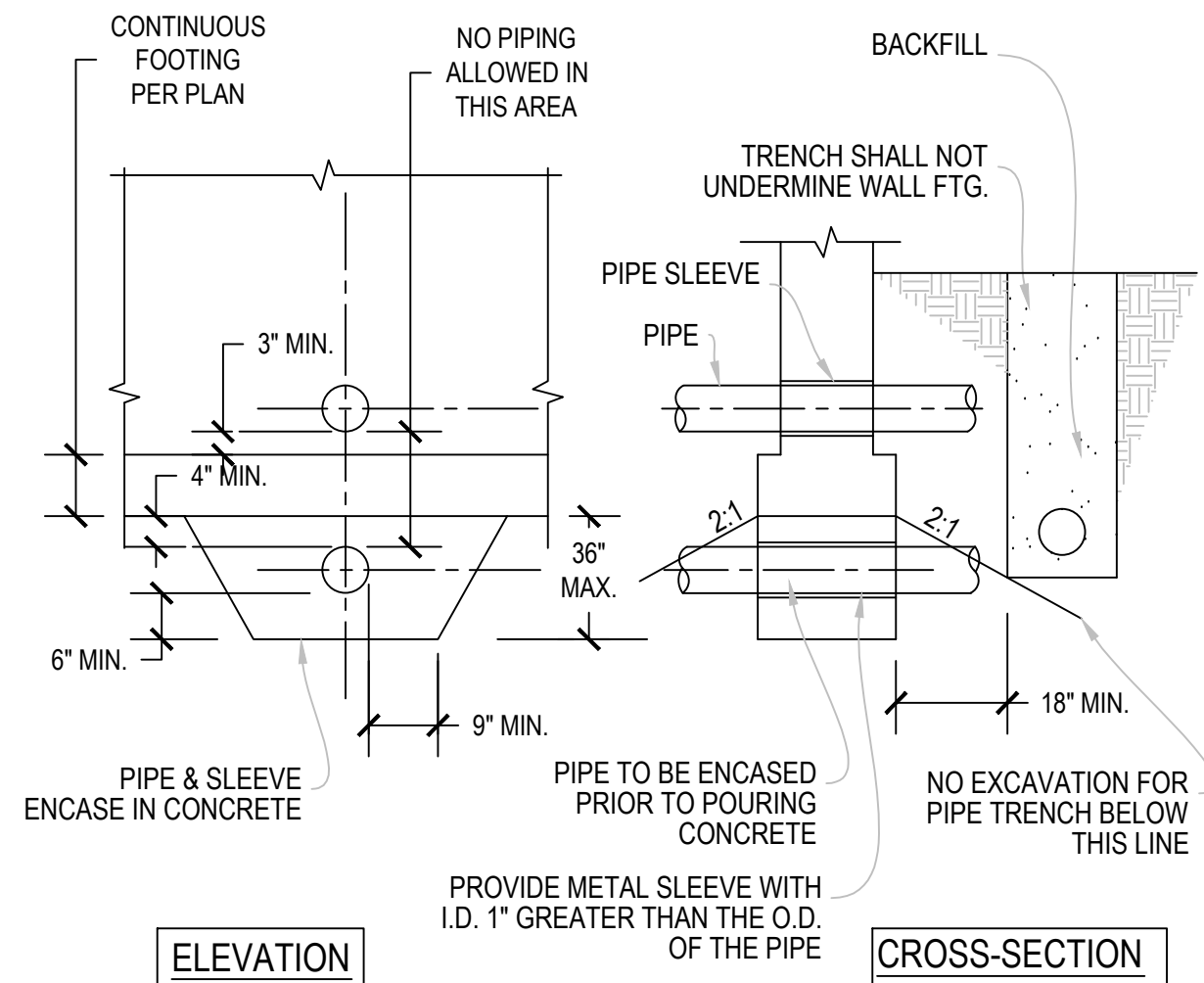


GOVERNING CODE: 2010 CALIFORNIA BUILDING CODE	FASTENING SCHEDULE (U.O.N.)	REINFORCING STEEL	STRUCTURAL WOOD	STRUCTURAL SHEATHING	FOUNDATION NOTES
	DIAPHRAGM SHEATHING NAILS OR OTHER APPROVED SHEATHING CONNECTORS SHALL BE DRIVEN SO THAT THEIR HEAD OR CROWN IS FLUSH WITH THE SURFACE OF THE SHEATHING.	01. BAR REINFORCEMENT SHALL BE ASTM A615, GRADE 60 FOR #4 AND LARGER AND GRADE 40 FOR #3 AND SMALLER.	01. ALL WOOD MEMBERS SHALL BE DOUGLAS FIR (DF) OR LARCH GRADE MARKED BY A RECOGNIZED GRADING AGENCY (WCLB & WWP)	1. ALL PLYWOOD SHALL BE CDX SHEATHING WITH EXTERIOR GLUE AND SHALL BEAR THE STAMP OF AN APPROVED TESTING AGENCY. LAY PLYWOOD WITH FACE GRAIN PERPENDICULAR TO SUPPORTS. USE A MINIMUM OF 4-PLY PLYWOOD. STAGGER JOINTS AND NAILS. ORIENTED STRAND BOARD (OSB), WITH EQUIVALENT PROPERTIES MAY REPLACE PLYWOOD.	01. FOR TYPICAL FOOTING DETAILS, SEE SHEET S2.0. 02. FOR HOLDOWN DETAILS, SHEET S2.0. 03. VERIFY DEPTH OF FOOTING AND SIZE OF STEEL REINFORCEMENT WITH CIVIL ENGINEER OR SOILS ENGINEER PRIOR TO START OF CONSTRUCTION. 04. CONCRETE FOR FOOTING AND SLABS SHALL HAVE A MIN. 28 DAYS COMPRESSIVE STRENGTH 2500 psi (UON.) 05. CONTINUOUS CONCRETE FOOTINGS SHALL BE IN ACCORDANCE WITH THE FOOTING SCHEDULE. 06. PROVIDE POST BASES AT POST WHERE SHOWN PER PLAN. 07. ANCHOR INTERIOR NON-SHEAR/NON-BEARING WALLS WITH HLTI "X-CP 72" ANCHOR W WASHER 0.145 O x 2.78" LONG PER ICC-ESR-2370 08. SEE "SILL BOLTING REQUIREMENTS" FOR ANCHOR BOLT INFORMATION. 09. LAP SPLICE CONTINUOUS REINFORCING STEEL PER DETAIL 3/S2.0 10. ALL HOLDOWNS COLUMNS BASES, ANCHORS BOLTS, POST ANCHORS, ETC. TO BE TIED IN PLACE AND VERIFIED BY ROUGH FRAMING CONTRACTOR PRIOR TO CALLING FOR FOUNDATION INSPECTION. 11. THE INSPECTOR WILL RECHECK FOR EXPANSIVE SOILS AND / OR GRADING REQUIREMENTS AT THE FIRST INSPECTION 12. FURTHER NOTES SHOULD BE REVIEWED ON SHEET S-2.
GENERAL STRUCTURAL NOTES	TABLE 2304.9.1 (CBC 2010)	02. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A185 AND SHALL BE LAPPED 12 INCHES MINIMUM.	02. WOOD GRADES: A) FOR HORIZONTAL MEMBERS (U.N.O.) : JOISTS & RAFTERS: GRADE #2 BEAMS: GRADE #1 LEDGERS AND NAILERS: GRADE #2 HEADERS: GRADE #1 TOP PLATES: MATCH VERTICAL MEMBERS DECKING: GRADE #2 B) FOR VERTICAL MEMBERS (U.N.O.) : 2 x 4 STUDS, 8'-0" LONG: STANDARD GRADE 2 x 4 STUDS, 8'-1" TO 14'-0": GRADE #1 2 x 8 & LARGER STUDS: GRADE #2 POSTS: GRADE #1	2. ROOF SHEATHING: 19/32" APA RATED SHEATHING SPAN RATING 40/20. UNBLOCKED WITH 10d COMMON NAILS AT 6" O.C. ALL BOUNDARIES AND SUPPORTED PANEL EDGES 12" O.C. FIELD UNLESS NOTED OTHERWISE.	
01. NOTES AND DETAILS ON THE STRUCTURAL DRAWINGS TAKE PRECEDENCE OVER THESE STANDARD STRUCTURAL NOTES. TYPICAL DETAILS SHALL BE USED WHENEVER APPLICABLE. REFER TO SPECIFICATIONS FOR INFORMATION NOT COVERED WITH THESE NOTES OR DRAWINGS.		03. MINIMUM LAP SPICES OF REINFORCING BARS SHALL BE AS FOLLOWS: A) CONCRETE: CLASS B AS DEFINED IN ACI 318-(CURRENT EDITION) UNLESS OTHERWISE NOTED ON PLANS.	03. PLYWOOD SHALL BE APA RATED SHEATHING AND SHALL CONFORM TO COMMERCIAL PRODUCTS STANDARD PS 1-00 WHERE REQUIRED. NAIL WITH COMMON NAILS OR AS APPROVED IN WRITING BY ENGINEER.	3. FLOOR / DECK SHEATHING: 3/4" APA RATED STURDI-FLOOR SPAN RATING 24". UNBLOCKED WITH 10d COMMON NAILS AT 6" O.C. ALL BOUNDARIES AND SUPPORTED PANEL EDGES. 12" O.C. FIELD UNLESS NOTED OTHERWISE.	
02. THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS, ELEVATIONS, AND SITE CONDITIONS BEFORE STARTING WORK, AND THE ENGINEER / ARCHITECT SHALL BE IMMEDIATELY NOTIFIED, IN WRITING, OF ANY DISCREPANCIES.		04. REINFORCING DETAILING, BENDING, AND PLACING SHALL BE IN ACCORDANCE WITH THE CONCRETE REINFORCING STEEL INSTITUTE "MANUAL OF STANDARD PRACTICE", LATEST EDITION.	04. CUTTING, NOTCHING, OR DRILLING OF STUDS SHALL BE PERMITTED ONLY AS IS DETAILED OR APPROVED BY THE ENGINEER AND/OR PER CBC SECTION 2308.9.10 OR 2308.9.11.	4. SHEAR WALL SHEATHING: STRUCTURAL I PLYWOOD OR O.S.B. PANEL BLOCKED WITH THE NAILING PER SHEAR WALL SCHEDULE, U.N.O.	
03. IN NO CASE SHALL DIMENSIONS BE SCALED FROM PLANS, SECTIONS, OR DETAILS ON THE STRUCTURAL DRAWINGS.		05. REINFORCING STEEL SHALL BE PROVIDED WITH THE FOLLOWING AMOUNTS OF COVER FOR CAST-IN-PLACE CONCRETE: A) CONCRETE DEPOSITED AGAINST EARTH: 3" B) CONCRETE SURFACE (FORMED) EXPOSED TO EARTH OR WEATHER: #6 THROUGH #18 BARS: 2" #5 BAR AND SMALLER: 1-1/2"	05. CUTTING, NOTCHING, OR DRILLING OF BEAMS OR JOISTS SHALL BE PERMITTED ONLY AS DETAILED OR APPROVED BY THE ENGINEER.	5. PLYWOOD PANELS TO BE SPACED AT 18" AT END AND EDGE JOINTS, FOR: ROOF, FLOOR, AND WALL CONSTRUCTION.	
04. ALL OMISSIONS AND CONFLICTS BETWEEN THE VARIOUS ELEMENTS OF THE DRAWINGS AND / OR SPECIFICATIONS SHALL BE BROUGHT TO THE ATTENTION OF, AND RESOLVED WITH, THE ENGINEER BEFORE PROCEEDING WITH ANY WORK SO INVOLVED.		06. ALL REINFORCING STEEL, ANCHOR BOLTS, DOWELS, AND INSERTS SHALL BE WELL SECURED IN POSITION WITH WIRE POSITIONERS BEFORE PLACING CONCRETE OR GROUT.	06. ALL NAILING SHALL CONFORM TO NAILING SCHEDULE, EXCEPT AS NOTED ON PLANS AND DETAILS.	6. ALL DECKS SHALL BE SHEATHED WITH EXTERIOR GRADE PLYWOOD AND SCREWED.	
05. WHERE A CONSTRUCTION DETAIL IS NOT SHOWN OR NOTED, THE DETAIL SHALL BE THE SAME AS FOR OTHER SIMILAR WORK. ANY QUESTIONS OF SUCH DETAILS SHOULD BE CONSULTED WITH THE ENGINEER OR ARCHITECT OF RECORD.		07. READY-MIX CONCRETE SHALL BE MIXED AND DELIVERED IN ACCORDANCE WITH ASTM C94.	07. ALL SILLS OR PLATES RESTING ON CONCRETE OR MASONRY, WHICH IS IN CONTACT WITH EARTH OR RESTING ON FOUNDATIONS, SHALL BE PRESSURE TREATED DOUGLAS FIR.		
06. THE CONTRACTOR SHALL DETERMINE THE LOCATION OF UTILITY SERVICES IN THE AREA TO BE EXCAVATED, BEFORE BEGINNING EXCAVATION.		08. ADMIXTURES MAY BE USED WITH PRIOR APPROVAL OF THE ENGINEER.	08. BOLTS IN WOOD SHALL NOT BE LESS THAN 7 DIAMETERS FROM THE END AND 4 DIAMETERS FROM THE EDGE OF THE MEMBER.		
07. NO PIPES, DUCTS, SLEEVES, CHASES, ETC. SHALL BE PLACED IN SLABS, BEAMS, OR WALLS, NOR SHALL ANY STRUCTURAL MEMBER BE CUT FOR PIPES, DUCTS, ETC. WITHOUT EXPRESSED WRITTEN PERMISSION FROM THE ENGINEER OF RECORD.		09. PROJECTING CORNERS OF SLABS, BEAMS, WALLS, COLUMNS, ETC., SHALL BE FORMED WITH A 3/4" CHAMFER.	10. ALL FRAMING ANCHORS, POST CAPS, BASES, HANGERS, STRAPS, ETC., SHALL BE AS MANUFACTURED BY "SIMPSON COMPANY" OR ENGINEER APPROVED EQUAL.		
08. ALL MATERIAL AND WORKSMANSHIP SHALL CONFORM TO THE REQUIREMENTS OF THE 2010 CBC.		10. REFER TO DRAWINGS OF OTHER DISCIPLINES FOR MOLDS, GROOVES, CLIPS, ORNAMENTS, OR GROUNDS REQUIRED TO BE CAST INTO CONCRETE.	11. PROVIDE DOUBLE JOISTS UNDER ALL PARALLEL PARTITIONS.		
09. THE CONTRACTOR SHALL ASSUME SOLE AND COMPLETE RESPONSIBILITY FOR JOB SITE CONDITIONS DURING THE COURSE OF CONSTRUCTION OF THIS PROJECT, INCLUDING SAFETY OF ALL PERSONS AND PROPERTY. THIS REQUIREMENT SHALL APPLY CONTINUOUSLY AND NOT BE LIMITED TO NORMAL WORKING HOURS. THE CONTRACTOR SHALL DEFEND, INDEMNIFY, AND HOLD THE ENGINEER FREE AND HARMLESS OF ALL CLAIMS, DEMANDS AND / OR ALL LIABILITY, REAL OR ALLEGED, IN CONNECTION WITH THE PERFORMANCE OF WORK ON THIS PROJECT, EXCEPT FOR LIABILITY ARISING FROM THE NEGLIGENCE OF THE ENGINEER OF RECORD.		11. CONTRACTOR SHALL SUBMIT MIX DESIGNS FOR REVIEW BEFORE FABRICATION AND INSTALLATION.	12. WHERE POSTS OCCUR ON 2nd FLOOR, ADD THE SAME POST ON FIRST FLOOR AND BETWEEN FLOOR SHEATHING AND 1st FLOOR TOP PLATE.		
10. VERIFICATION OF A CALIFORNIA REGISTERED CIVIL ENGINEER IS NEEDED AND RETAINED TO DESIGN ALL TEMPORARY BRACING, SHORING, AND SUPPORT REQUIREMENTS DURING CONSTRUCTION.		12. PLANS MUST BE APPROVED BY BUILDING DEPARTMENT PRIOR TO CONSTRUCTION.	13. TOP PLATES OF ALL WOOD STUD WALLS TO BE 2x2 MINIMUM (SAME WIDTH AS STUDS), LAP 48" (MINIMUM), WITH NOT LESS THAN 6-16d NAILS AT EACH LAP AND NOT MORE THAN 12" BETWEEN NAILS.		
11. THE CONTRACTOR / OWNER / DESIGNER MUST SUBMIT MATERIAL ALTERNATIVES TO THE ENGINEER OF RECORD PRIOR TO USE AND POSSIBLE PURCHASE TO AVOID UNNECESSARY COST TO CONTRACTOR / OWNER / DESIGNER. SUCH ENGINEERING REVIEW IS CONSIDERED SEPARATE ENGINEERING FEES UNDER ON-SITE ENGINEERING COSTS.		13. ALL SITE SOIL WORK SHALL BE DONE UNDER THE DIRECT OBSERVATION OF THE SOILS ENGINEER.	14. MOISTURE CONTENT OF WOOD AT TIME OF PLACING SHALL NOT EXCEED 19 PERCENT.		
12. PLANS MUST BE APPROVED BY BUILDING DEPARTMENT PRIOR TO CONSTRUCTION.		14. SOILS ENGINEER SHALL VERIFY IN WRITING TO THE ENGINEER THAT ANY COMPACTION SITE SOIL WORK COMPLIES WITH ALL OF THE RECOMMENDATIONS AND CONCLUSIONS OF THE SOILS REPORT.	15. INSTALL ALL SPECIFIED FASTENERS BEFORE LOADING THE CONNECTION.		
FOUNDATION		15. THE FINISH EXCAVATION FOR FOUNDATIONS SHALL BE NEAT & TRUE TO LINE.	16. FOUNDATION EXCAVATIONS SHALL BE KEPT FREE OF LOOSE MATERIAL AND STANDING WATER AND SHALL BE CHECKED AND APPROVED BY THE SOILS ENGINEER BEFORE THE PLACEMENT OF ANY CONCRETE.		
01. THE SOILS REPORT RECOMMENDATIONS SHALL BE COMPLIED WITH BY THE CONTRACTOR.		16. FOUNDATION EXCAVATIONS SHALL BE KEPT FREE OF LOOSE MATERIAL AND STANDING WATER AND SHALL BE CHECKED AND APPROVED BY THE SOILS ENGINEER BEFORE THE PLACEMENT OF ANY CONCRETE.	17. THE TESTING LAB SHALL SUBMIT COMPACTION REPORTS FOR ALL FILL TO THE ENGINEER BEFORE REQUESTING FOUNDATION INSPECTION. ALL LOOSE SOIL AND FILL DIRT, INCLUDING BACKFILL BEHIND RETAINING WALLS, SHALL BE COMPACTED TO AT LEAST 90% OF MAXIMUM DENSITY, OR GREATER AS REQUIRED BY THE SOILS REPORT.		
02. SOILS INFORMATION: PREPARED BY: C.W. LA MONTE COMPANY, INC. DATE: AUGUST 2, 2013 PROJECT NO. 13-0275 VERTICAL BEARING PRESSURE: 2000 PSF ACTIVE PRESSURE: 35 PSF LATERAL BEARING PRESSURE: 350 PSF		18. SOILS ENGINEER SHALL VERIFY IN WRITING TO THE ENGINEER THAT ANY COMPACTION SITE SOIL WORK COMPLIES WITH ALL OF THE RECOMMENDATIONS AND CONCLUSIONS OF THE SOILS REPORT.	18. SOILS ENGINEER SHALL VERIFY IN WRITING TO THE ENGINEER THAT ANY COMPACTION SITE SOIL WORK COMPLIES WITH ALL OF THE RECOMMENDATIONS AND CONCLUSIONS OF THE SOILS REPORT.		
NOTE: WHEN NO SOILS REPORT IS COMPLETED, FOOTINGS ARE DESIGNED ON A ASSUMED ALLOWABLE SOIL PRESSURE. IT IS STRONGLY RECOMMENDED THAT SOILS TESTING BY A LICENSED GEOTECHNICAL ENGINEER BE PERFORMED TO VERIFY SOIL BEARING CAPACITY AND STABILITY.		19. THE FINISH EXCAVATION FOR FOUNDATIONS SHALL BE NEAT & TRUE TO LINE.	19. THE TESTING LAB SHALL SUBMIT COMPACTION REPORTS FOR ALL FILL TO THE ENGINEER BEFORE REQUESTING FOUNDATION INSPECTION. ALL LOOSE SOIL AND FILL DRY, INCLUDING BACKFILL BEHIND RETAINING WALLS, SHALL BE COMPACTED TO AT LEAST 90% OF MAXIMUM DENSITY, OR GREATER AS REQUIRED BY THE SOILS REPORT.		
03. ALL SITE SOIL WORK SHALL BE DONE UNDER THE DIRECT OBSERVATION OF THE SOILS ENGINEER.		20. FOUNDATION EXCAVATIONS SHALL BE KEPT FREE OF LOOSE MATERIAL AND STANDING WATER AND SHALL BE CHECKED AND APPROVED BY THE SOILS ENGINEER BEFORE THE PLACEMENT OF ANY CONCRETE.	20. FOUNDATION EXCAVATIONS SHALL BE KEPT FREE OF LOOSE MATERIAL AND STANDING WATER AND SHALL BE CHECKED AND APPROVED BY THE SOILS ENGINEER BEFORE THE PLACEMENT OF ANY CONCRETE.		
04. SOILS ENGINEER SHALL VERIFY IN WRITING TO THE ENGINEER THAT ANY COMPACTION SITE SOIL WORK COMPLIES WITH ALL OF THE RECOMMENDATIONS AND CONCLUSIONS OF THE SOILS REPORT.		21. THE TESTING LAB SHALL SUBMIT COMPACTION REPORTS FOR ALL FILL TO THE ENGINEER BEFORE REQUESTING FOUNDATION INSPECTION. ALL LOOSE SOIL AND FILL DRY, INCLUDING BACKFILL BEHIND RETAINING WALLS, SHALL BE COMPACTED TO AT LEAST 90% OF MAXIMUM DENSITY, OR GREATER AS REQUIRED BY THE SOILS REPORT.	21. THE TESTING LAB SHALL SUBMIT COMPACTION REPORTS FOR ALL FILL TO THE ENGINEER BEFORE REQUESTING FOUNDATION INSPECTION. ALL LOOSE SOIL AND FILL DRY, INCLUDING BACKFILL BEHIND RETAINING WALLS, SHALL BE COMPACTED TO AT LEAST 90% OF MAXIMUM DENSITY, OR GREATER AS REQUIRED BY THE SOILS REPORT.		
05. THE FINISH EXCAVATION FOR FOUNDATIONS SHALL BE NEAT & TRUE TO LINE.		22. SOILS ENGINEER SHALL VERIFY IN WRITING TO THE ENGINEER THAT ANY COMPACTION SITE SOIL WORK COMPLIES WITH ALL OF THE RECOMMENDATIONS AND CONCLUSIONS OF THE SOILS REPORT.	22. SOILS ENGINEER SHALL VERIFY IN WRITING TO THE ENGINEER THAT ANY COMPACTION SITE SOIL WORK COMPLIES WITH ALL OF THE RECOMMENDATIONS AND CONCLUSIONS OF THE SOILS REPORT.		
06. FOUNDATION EXCAVATIONS SHALL BE KEPT FREE OF LOOSE MATERIAL AND STANDING WATER AND SHALL BE CHECKED AND APPROVED BY THE SOILS ENGINEER BEFORE THE PLACEMENT OF ANY CONCRETE.		23. THE FINISH EXCAVATION FOR FOUNDATIONS SHALL BE NEAT & TRUE TO LINE.	23. THE TESTING LAB SHALL SUBMIT COMPACTION REPORTS FOR ALL FILL TO THE ENGINEER BEFORE REQUESTING FOUNDATION INSPECTION. ALL LOOSE SOIL AND FILL DRY, INCLUDING BACKFILL BEHIND RETAINING WALLS, SHALL BE COMPACTED TO AT LEAST 90% OF MAXIMUM DENSITY, OR GREATER AS REQUIRED BY THE SOILS REPORT.		
07. THE TESTING LAB SHALL SUBMIT COMPACTION REPORTS FOR ALL FILL TO THE ENGINEER BEFORE REQUESTING FOUNDATION INSPECTION. ALL LOOSE SOIL AND FILL DIRT, INCLUDING BACKFILL BEHIND RETAINING WALLS, SHALL BE COMPACTED TO AT LEAST 90% OF MAXIMUM DENSITY, OR GREATER AS REQUIRED BY THE SOILS REPORT.		24. SOILS ENGINEER SHALL VERIFY IN WRITING TO THE ENGINEER THAT ANY COMPACTION SITE SOIL WORK COMPLIES WITH ALL OF THE RECOMMENDATIONS AND CONCLUSIONS OF THE SOILS REPORT.	24. SOILS ENGINEER SHALL VERIFY IN WRITING TO THE ENGINEER THAT ANY COMPACTION SITE SOIL WORK COMPLIES WITH ALL OF THE RECOMMENDATIONS AND CONCLUSIONS OF THE SOILS REPORT.		
08. ALL FOOTINGS MUST BEAR IN COMPETENT NATURAL GROUND.		25. THE TESTING LAB SHALL SUBMIT COMPACTION REPORTS FOR ALL FILL TO THE ENGINEER BEFORE REQUESTING FOUNDATION INSPECTION. ALL LOOSE SOIL AND FILL DRY, INCLUDING BACKFILL BEHIND RETAINING WALLS, SHALL BE COMPACTED TO AT LEAST 90% OF MAXIMUM DENSITY, OR GREATER AS REQUIRED BY THE SOILS REPORT.	25. THE TESTING LAB SHALL SUBMIT COMPACTION REPORTS FOR ALL FILL TO THE ENGINEER BEFORE REQUESTING FOUNDATION INSPECTION. ALL LOOSE SOIL AND FILL DRY, INCLUDING BACKFILL BEHIND RETAINING WALLS, SHALL BE COMPACTED TO AT LEAST 90% OF MAXIMUM DENSITY, OR GREATER AS REQUIRED BY THE SOILS REPORT.		
CONCRETE MASONRY		26. SOILS ENGINEER SHALL VERIFY IN WRITING TO THE ENGINEER THAT ANY COMPACTION SITE SOIL WORK COMPLIES WITH ALL OF THE RECOMMENDATIONS AND CONCLUSIONS OF THE SOILS REPORT.	26. SOILS ENGINEER SHALL VERIFY IN WRITING TO THE ENGINEER THAT ANY COMPACTION SITE SOIL WORK COMPLIES WITH ALL OF THE RECOMMENDATIONS AND CONCLUSIONS OF THE SOILS REPORT.		
01. CONCRETE MASONRY WALLS SHALL HAVE A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF f'm =1500 PSI.		27. THE TESTING LAB SHALL SUBMIT COMPACTION REPORTS FOR ALL FILL TO THE ENGINEER BEFORE REQUESTING FOUNDATION INSPECTION. ALL LOOSE SOIL AND FILL DRY, INCLUDING BACKFILL BEHIND RETAINING WALLS, SHALL BE COMPACTED TO AT LEAST 90% OF MAXIMUM DENSITY, OR GREATER AS REQUIRED BY THE SOILS REPORT.	27. THE TESTING LAB SHALL SUBMIT COMPACTION REPORTS FOR ALL FILL TO THE ENGINEER BEFORE REQUESTING FOUNDATION INSPECTION. ALL LOOSE SOIL AND FILL DRY, INCLUDING BACKFILL BEHIND RETAINING WALLS, SHALL BE COMPACTED TO AT LEAST 90% OF MAXIMUM DENSITY, OR GREATER AS REQUIRED BY THE SOILS REPORT.		
02. CONCRETE MASONRY UNITS SHALL BE MEDIUM WEIGHT UNITS CONFORMING TO ASTM C 90, GRADE N-L, WITH MAX LINEAR SHRINKAGE OF 0.06 % (1900 PSI MINIMUM).		28. SOILS ENGINEER SHALL VERIFY IN WRITING TO THE ENGINEER THAT ANY COMPACTION SITE SOIL WORK COMPLIES WITH ALL OF THE RECOMMENDATIONS AND CONCLUSIONS OF THE SOILS REPORT.	28. SOILS ENGINEER SHALL VERIFY IN WRITING TO THE ENGINEER THAT ANY COMPACTION SITE SOIL WORK COMPLIES WITH ALL OF THE RECOMMENDATIONS AND CONCLUSIONS OF THE SOILS REPORT.		
03. MORTAR SHALL BE TYPE "S", CONFORMING TO CBC TABLE 2103.9.1) (MINIMUM 1900 PSI AT 28 DAYS).		29. THE TESTING LAB SHALL SUBMIT COMPACTION REPORTS FOR ALL FILL TO THE ENGINEER BEFORE REQUESTING FOUNDATION INSPECTION. ALL LOOSE SOIL AND FILL DRY, INCLUDING BACKFILL BEHIND RETAINING WALLS, SHALL BE COMPACTED TO AT LEAST 90% OF MAXIMUM DENSITY, OR GREATER AS REQUIRED BY THE SOILS REPORT.	29. THE TESTING LAB SHALL SUBMIT COMPACTION REPORTS FOR ALL FILL TO THE ENGINEER BEFORE REQUESTING FOUNDATION INSPECTION. ALL LOOSE SOIL AND FILL DRY, INCLUDING BACKFILL BEHIND RETAINING WALLS, SHALL BE COMPACTED TO AT LEAST 90% OF MAXIMUM DENSITY, OR GREATER AS REQUIRED BY THE SOILS REPORT.		
04. GROUT SHALL BE COMPOSED OF THE FOLLOWING RATIO BY VOLUME: 1-PART PORTLAND CEMENT, 3-PARTS SAND, 2-PARTS PEA GRAVEL AND SUFFICIENT WATER FOR POURING WITHOUT SEGREGATION OF GROUT CONSTITUENTS (MIN. COMPRESSIVE STRENGTH OF 2000 PSI AT 28 DAYS).		30. SOILS ENGINEER SHALL VERIFY IN WRITING TO THE ENGINEER THAT ANY COMPACTION SITE SOIL WORK COMPLIES WITH ALL OF THE RECOMMENDATIONS AND CONCLUSIONS OF THE SOILS REPORT.	30. SOILS ENGINEER SHALL VERIFY IN WRITING TO THE ENGINEER THAT ANY COMPACTION SITE SOIL WORK COMPLIES WITH ALL OF THE RECOMMENDATIONS AND CONCLUSIONS OF THE SOILS REPORT.		
05. ALL CELLS CONTAINING REINFORCING STEEL OR EMBEDDED ITEMS & ALL CELLS IN RETAINING WALLS AND WALLS BELOW GRADE SHALL BE SOLID GROUTED.		31. THE TESTING LAB SHALL SUBMIT COMPACTION REPORTS FOR ALL FILL TO THE ENGINEER BEFORE REQUESTING FOUNDATION INSPECTION. ALL LOOSE SOIL AND FILL DRY, INCLUDING BACKFILL BEHIND RETAINING WALLS, SHALL BE COMPACTED TO AT LEAST 90% OF MAXIMUM DENSITY, OR GREATER AS REQUIRED BY THE SOILS REPORT.	31. THE TESTING LAB SHALL SUBMIT COMPACTION REPORTS FOR ALL FILL TO THE ENGINEER BEFORE REQUESTING FOUNDATION INSPECTION. ALL LOOSE SOIL AND FILL DRY, INCLUDING BACKFILL BEHIND RETAINING WALLS, SHALL BE COMPACTED TO AT LEAST 90% OF MAXIMUM DENSITY, OR GREATER AS REQUIRED BY THE SOILS REPORT.		
06. ALL HORIZONTAL REINFORCEMENT SHALL BE PLACED IN BOND BEAM OR LINTEL BEAM UNITS.		32. SOILS ENGINEER SHALL VERIFY IN WRITING TO THE ENGINEER THAT ANY COMPACTION SITE SOIL WORK COMPLIES WITH ALL OF THE RECOMMENDATIONS AND CONCLUSIONS OF THE SOILS REPORT.	32. SOILS ENGINEER SHALL VERIFY IN WRITING TO THE ENGINEER THAT ANY COMPACTION SITE SOIL WORK COMPLIES WITH ALL OF THE RECOMMENDATIONS AND CONCLUSIONS OF THE SOILS REPORT.		
07. WHEN GROUTING IS STOPPED FOR ONE HOUR OR LONGER, HORIZ. CONSTRUCTION JOINTS SHALL BE FORMED BY STOPPING THE GROUT POUR 1-1/2" BELOW TOP OF THE UPPERMOST UNIT.		33. THE TESTING LAB SHALL SUBMIT COMPACTION REPORTS FOR ALL FILL TO THE ENGINEER BEFORE REQUESTING FOUNDATION INSPECTION. ALL LOOSE SOIL AND FILL DRY, INCLUDING BACKFILL BEHIND RETAINING WALLS, SHALL BE COMPACTED TO AT LEAST 90% OF MAXIMUM DENSITY, OR GREATER AS REQUIRED BY THE SOILS REPORT.	33. THE TESTING LAB SHALL SUBMIT COMPACTION REPORTS FOR ALL FILL TO THE ENGINEER BEFORE REQUESTING FOUNDATION INSPECTION. ALL LOOSE SOIL AND FILL DRY, INCLUDING BACKFILL BEHIND RETAINING WALLS, SHALL BE COMPACTED TO AT LEAST 90% OF MAXIMUM DENSITY, OR GREATER AS REQUIRED BY THE SOILS REPORT.		
08. ALL BOND BEAM BLOCK SHALL BE "DEEP CUT" UNITS.		34. SOILS ENGINEER SHALL VERIFY IN WRITING TO THE ENGINEER THAT ANY COMPACTION SITE SOIL WORK COMPLIES WITH ALL OF THE RECOMMENDATIONS AND CONCLUSIONS OF THE SOILS REPORT.	34. SOILS ENGINEER SHALL VERIFY IN WRITING TO THE ENGINEER THAT ANY COMPACTION SITE SOIL WORK COMPLIES WITH ALL OF THE RECOMMENDATIONS AND CONCLUSIONS OF THE SOILS REPORT.		
09. PROVIDE INSPECTION AND CLEANOUT HOLES AT BASE OF GROUT LIFT FOR VERTICAL CELLS HAVING GROUT LIFTS IN EXCESS OF 5'-0" OF HEIGHT.		35. THE TESTING LAB SHALL SUBMIT COMPACTION REPORTS FOR ALL FILL TO THE ENGINEER BEFORE REQUESTING FOUNDATION INSPECTION. ALL LOOSE SOIL AND FILL DRY, INCLUDING BACKFILL BEHIND RETAINING WALLS, SHALL BE COMPACTED TO AT LEAST 90% OF MAXIMUM DENSITY, OR GREATER AS REQUIRED BY THE SOILS REPORT.	35. THE TESTING LAB SHALL SUBMIT COMPACTION REPORTS FOR ALL FILL TO THE ENGINEER BEFORE REQUESTING FOUNDATION INSPECTION. ALL LOOSE SOIL AND FILL DRY, INCLUDING BACKFILL BEHIND RETAINING WALLS, SHALL BE COMPACTED TO AT LEAST 90% OF MAXIMUM DENSITY, OR GREATER AS REQUIRED BY THE SOILS REPORT.		
10. ALL GROUT SHALL BE CONSOLIDATED WITH A MECHANICAL VIBRATOR.		36. SOILS ENGINEER SHALL VERIFY IN WRITING TO THE ENGINEER THAT ANY COMPACTION SITE SOIL WORK COMPLIES WITH ALL OF THE RECOMMENDATIONS AND CONCLUSIONS OF THE SOILS REPORT.	36. SOILS ENGINEER SHALL VERIFY IN WRITING TO THE ENGINEER THAT ANY COMPACTION SITE SOIL WORK COMPLIES WITH ALL OF THE RECOMMENDATIONS AND CONCLUSIONS OF THE SOILS REPORT.		
11. ANCHOR BOLTS MUST BE SET WITH TEMPLATES AND HELD IN PLACE PRIOR TO GROUTING. PROVIDE AT LEAST ONE INCH OF GROUT BETWEEN ANCHOR BOLT AND MASONRY.		37. THE TESTING LAB SHALL SUBMIT COMPACTION REPORTS FOR ALL FILL TO THE ENGINEER BEFORE REQUESTING FOUNDATION INSPECTION. ALL LOOSE SOIL AND FILL DRY, INCLUDING BACKFILL BEHIND RETAINING WALLS, SHALL BE COMPACTED TO AT LEAST 90% OF MAXIMUM DENSITY, OR GREATER AS REQUIRED BY THE SOILS REPORT.	37. THE TESTING LAB SHALL SUBMIT COMPACTION REPORTS FOR ALL FILL TO THE ENGINEER BEFORE REQUESTING FOUNDATION INSPECTION. ALL LOOSE SOIL AND FILL DRY, INCLUDING BACKFILL BEHIND RETAINING WALLS, SHALL BE COMPACTED TO AT LEAST 90% OF MAXIMUM DENSITY, OR GREATER AS REQUIRED BY THE SOILS REPORT.		
		38. SOILS ENGINEER SHALL VERIFY IN WRITING TO THE ENGINEER THAT ANY COMPACTION SITE SOIL WORK COMPLIES WITH ALL OF THE RECOMMENDATIONS AND CONCLUSIONS OF THE SOILS REPORT.	38. SOILS ENGINEER SHALL VERIFY IN WRITING TO THE ENGINEER THAT ANY COMPACTION SITE SOIL WORK COMPLIES WITH ALL OF THE RECOMMENDATIONS AND CONCLUSIONS OF THE SOILS REPORT.		
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		40. SOILS ENGINEER SHALL VERIFY IN WRITING TO THE ENGINEER THAT ANY COMPACTION SITE SOIL WORK COMPLIES WITH ALL OF THE RECOMMENDATIONS AND CONCLUSIONS OF THE SOILS REPORT.	40. SOILS		



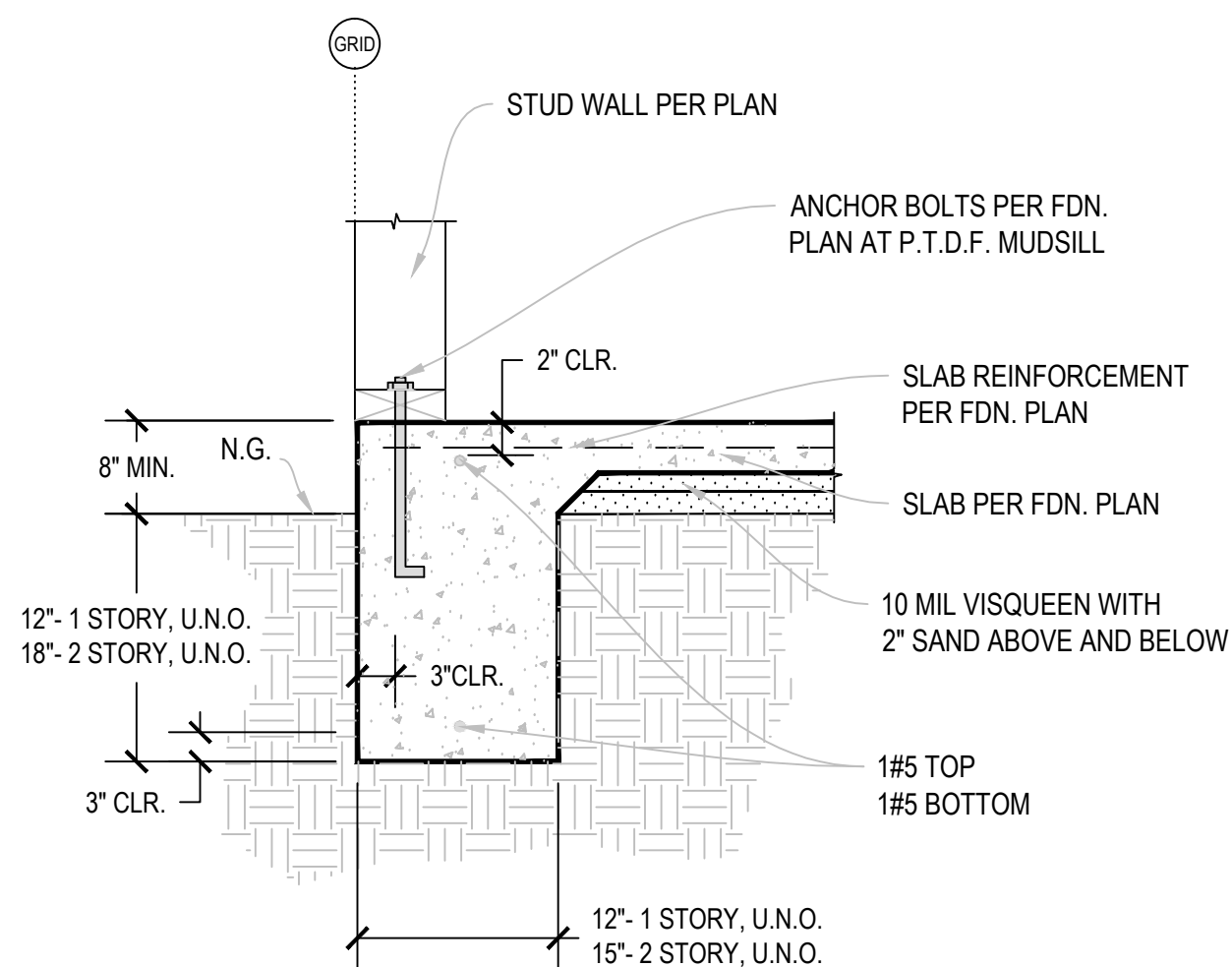
TYPICAL PIPES IN STUD WALL

13



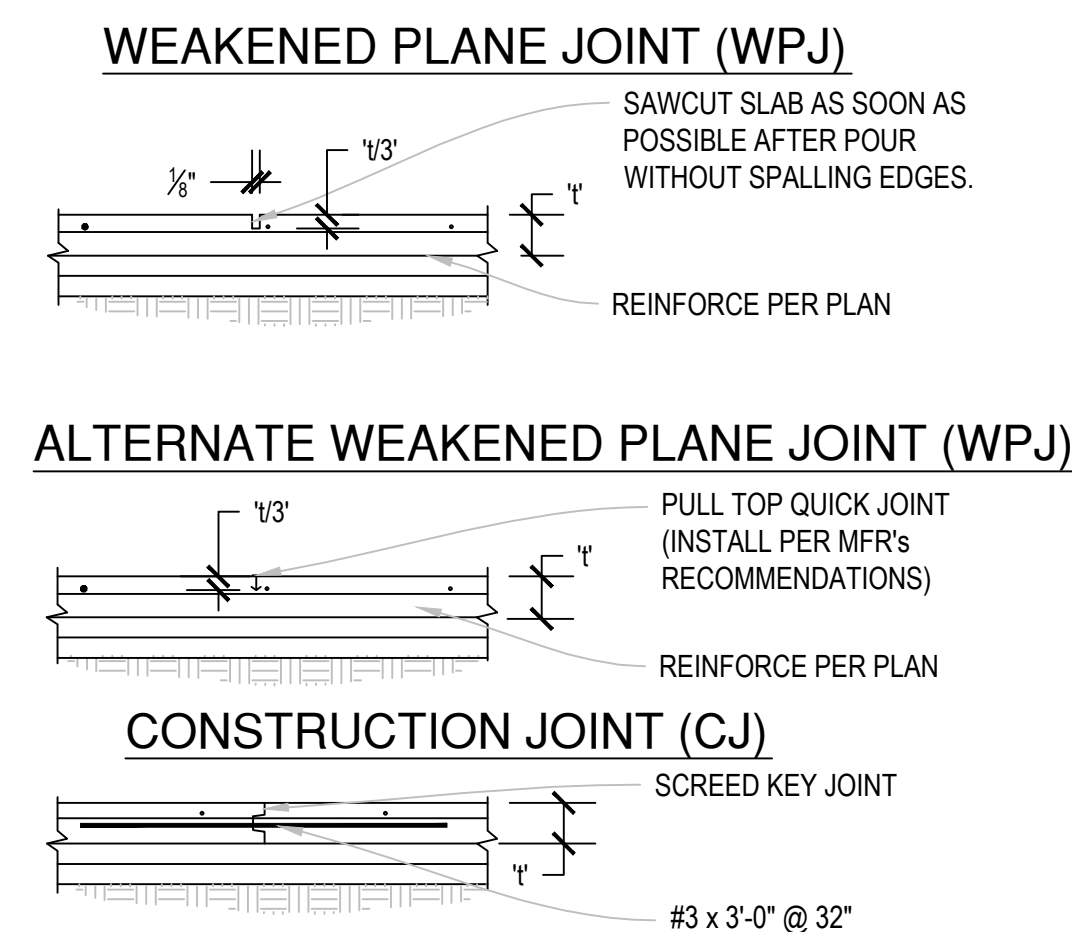
PIPE LOCATIONS AT FOOTING

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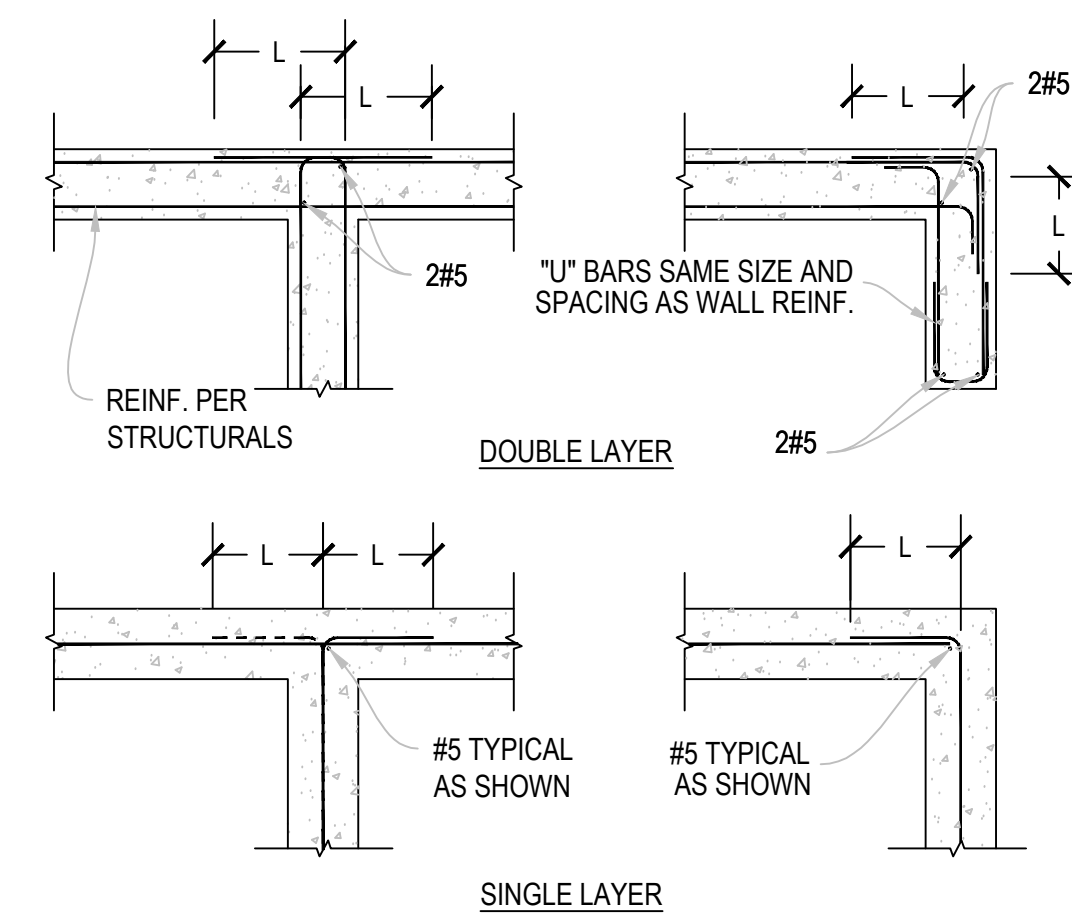
FOOTING, EXTERIOR WALL

7



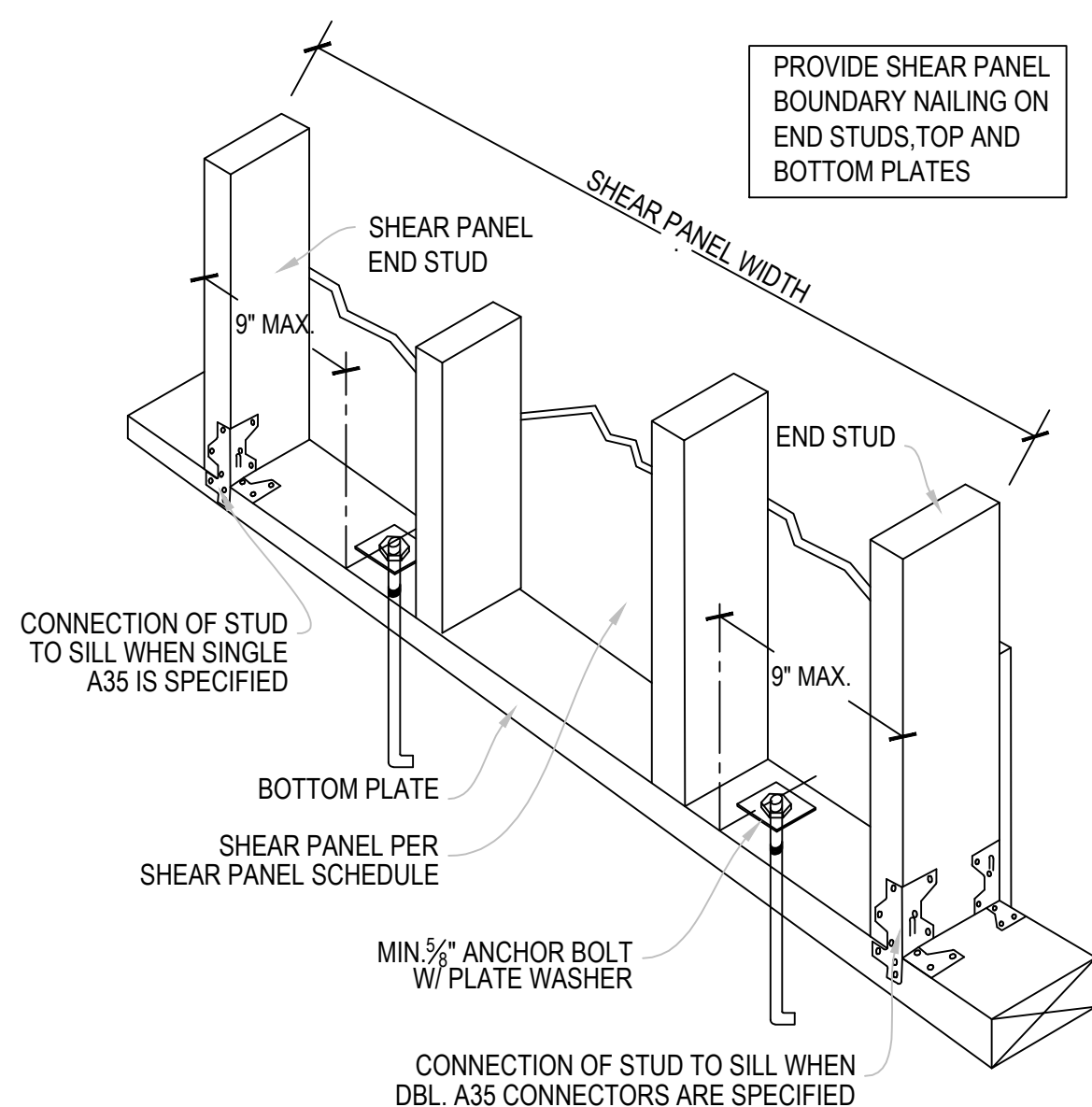
CONCRETE SLAB JOINTS

4



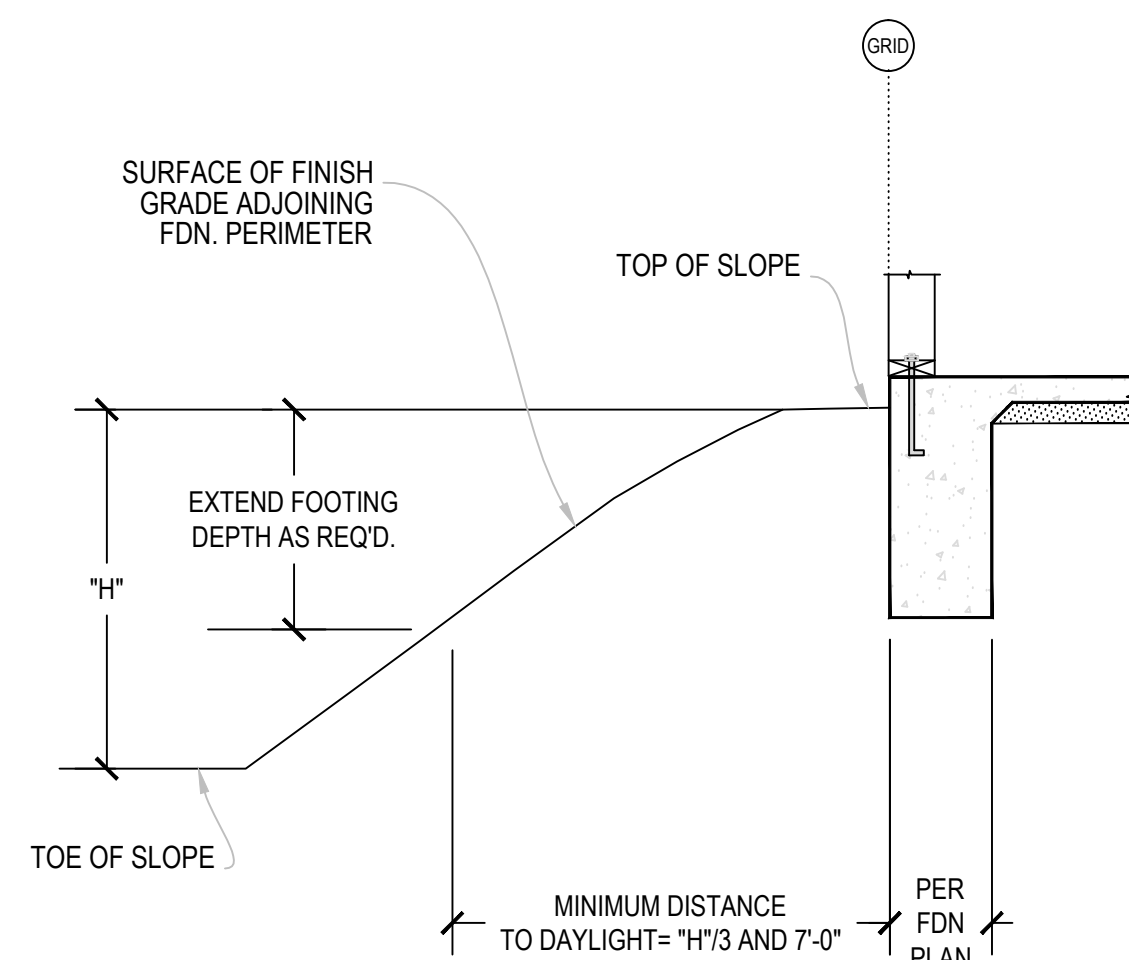
CORNER/INTERSECTION REINF. @ WALLS & FTGS.

1



STUD TO SILL CONNECTION AT SHEAR PANEL

14



DEEPEEN FOOTING AT SLOPE

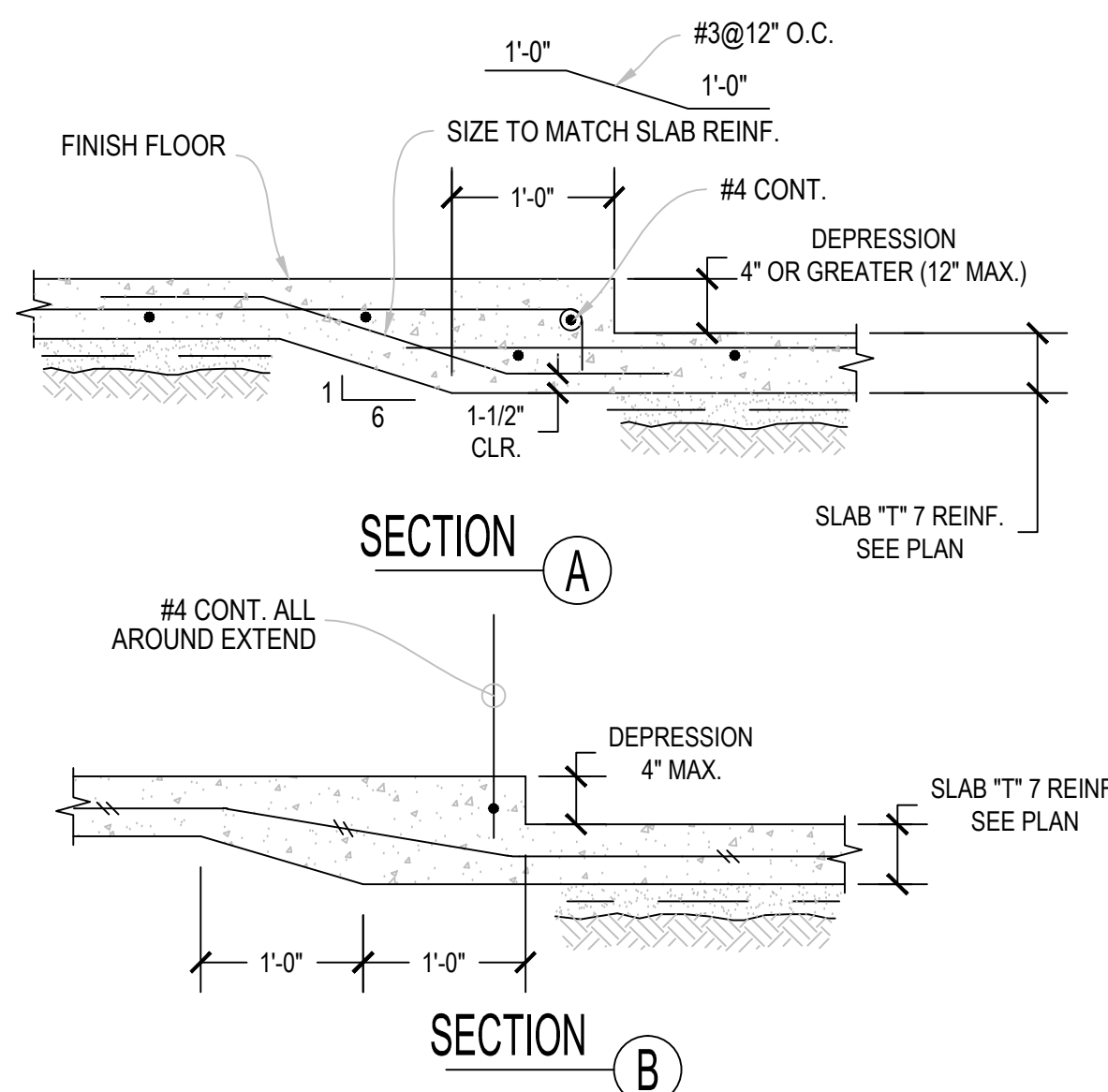
11

FOUNDATION SCHEDULE		
TYPE	DIMENSIONS (1)	RE-BAR
SLAB	4" THICK	#3@18" O.C. (EW)
F1.0	15" x CONTINUOUS x 15"	1 #5 TOP 1 #5 BOTTOM
F2.0	2'-0" x 2'-0" x 18"	(3) #5 (EW)
F2.5	2'-6" x 2'-6" x 18"	(4) #5 (EW)
F3.0	3'-0" x 3'-0" x 18"	(4) #5 (EW)
F3.5	3'-6" x 3'-6" x 18"	(4) #5 (EW)
F4.0	4'-0" x 4'-0" x 18"	(5) #5 (EW)

(1) DEPTH IS MEASURED INTO FIRM NATURAL GROUND OR PROPERTY COMPACTED FILL.

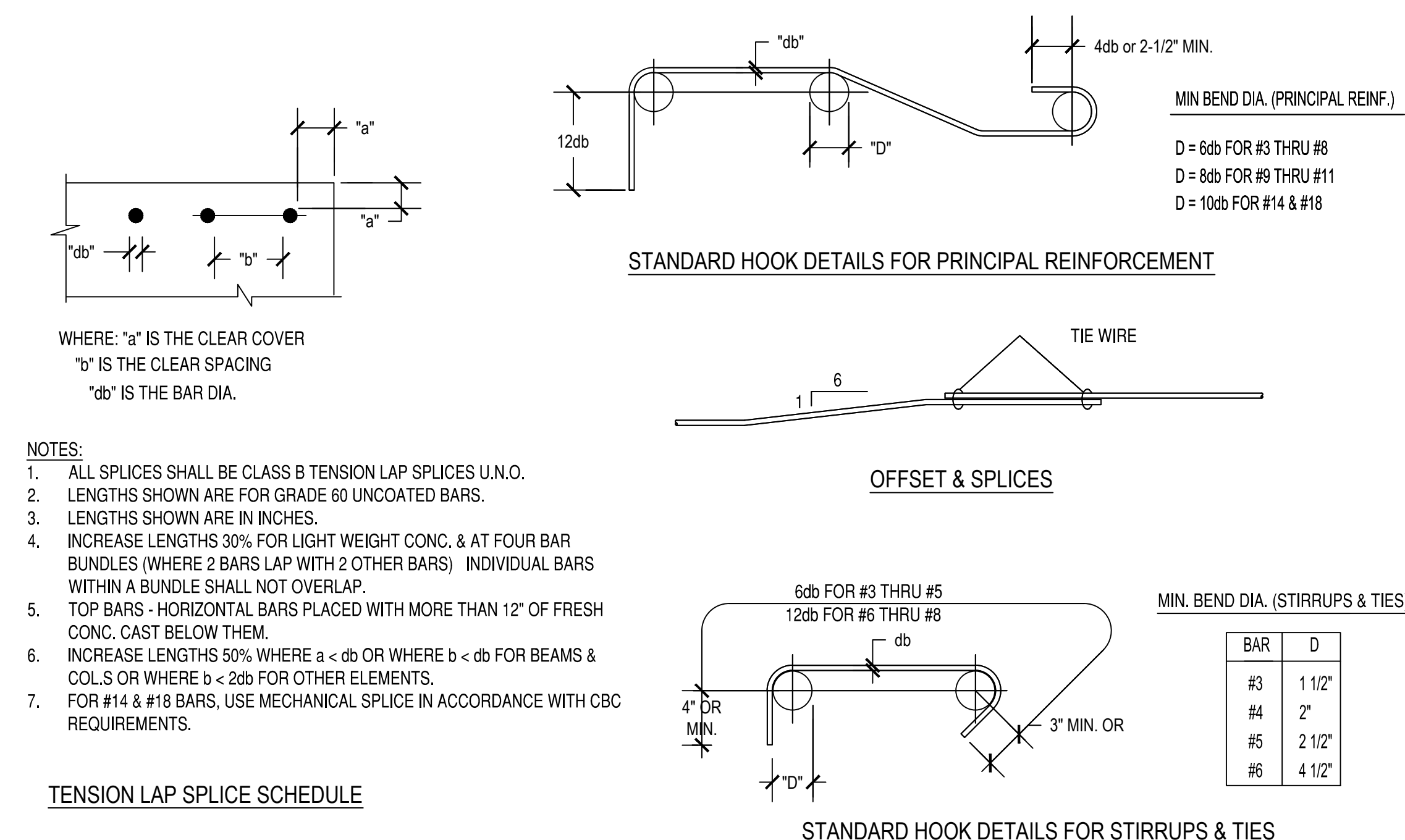
FOOTING SCHEDULE

8



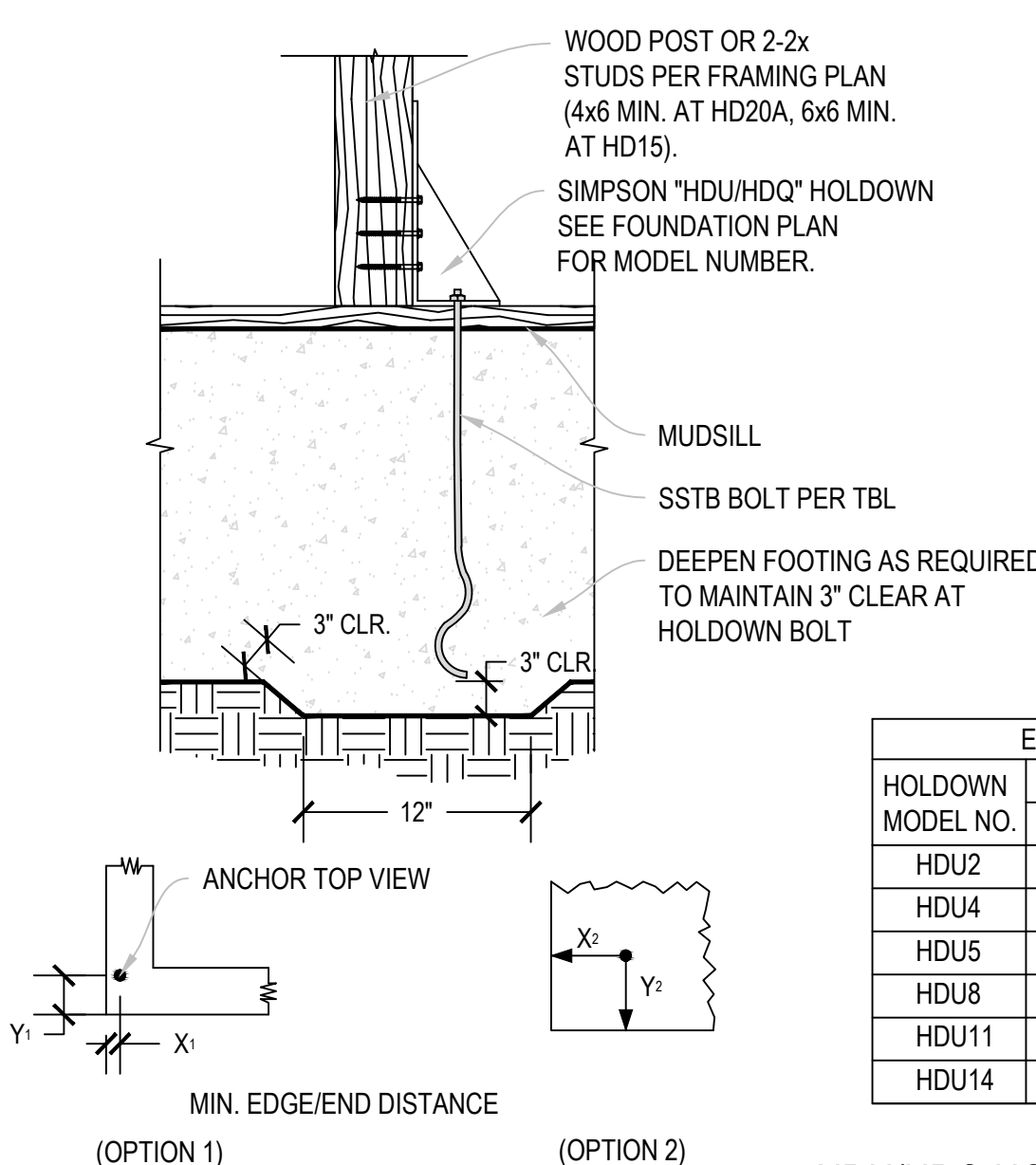
TYP. SLAB ON GRADE DEPRESSION DETAIL

9



TYPICAL REINFORCEMENT DETAILS & TENSION LAP SPLICE SCHEDULE

3



HDU/HDQ HOLDOWN DETAILS

12

HOLDOWN MODEL NO.	ANCHOR Ø	EMBED. LENGTH	ANCHOR BOLT TYPE
HDU2	5/8" Ø	12-5/8"	SSTB16
HDU4	5/8" Ø	12-5/8"	SSTB16
HDU5	7/8" Ø	24-7/8"	SSTB28
HDU8	7/8" Ø	24-7/8"	SSTB28
HDU11	1" Ø	28"	ASTM307 SQUARE HEAD BOLT
HDU14	1" Ø	28"	ASTM307 SQUARE HEAD BOLT

HOLDOWN MODEL NO.	EDGE DISTANCES (INCHES)			
	(OPTION 1)	(OPTION 2)	(OPTION 1)	(OPTION 2)
HDU2	1-3/4	5	7-1/2	7-1/2
HDU4	1-3/4	5	7-1/2	7-1/2
HDU5	1-3/4	14-1/16	7-1/2	7-1/2
HDU8	1-3/4	24	11-5/8	24
HDU11	2-3/4	24	N/A	N/A
HDU14	2-3/4	24	N/A	N/A

ANCHOR NOTES
1. USE SIMPSON SDS/4x2-1/2 WOOD SCREWS.
2. USE THIS SCHEDULE FOR ALL HOLDOWN BOLT EMBEDMENTS U.N.O ON PLANS
3. THE EMBEDMENT LENGTHS ARE BASED ON A STANDARD SSTB BOLTS PER SIMPSON.
4. WHEN USING COMPOSITE LUMBER POSTS, SCREWS MUST BE APPLIED TO THE WIDE FACE OF THE COLUMN.
5. ALL DATA SHOWN PER SIMPSON STRONG-TIE CO.

BAR SIZE	CLASS B TENSION LAP SPLICE LENGTH						CLASS A DEVELOPMENT LENGTH (ld)					
	2500 PSI CONC f _c	3000 PSI CONC f _c	4000 PSI CONC f _c	2500 PSI CONC f _c	3000 PSI CONC f _c	4000 PSI CONC f _c	2500 PSI CONC f _c	3000 PSI CONC f _c	4000 PSI CONC f _c	2500 PSI CONC f _c	3000 PSI CONC f _c	4000 PSI CONC f _c
#3	31	24	28	22	24	19	24	19	22	17	19	15
#4	41	32	37	29	32	25	32	24	29	22	25	19
#5	52	40	47	36	40	31	39	31	36	28	31	24
#6	61	47	56	43	48	37	47	36	43	33	37	29
#7	89	69	81	63	70	54	69	53	63	48	54	42
#8	102	79	93	72	80	62	79	60	72	55	62	48
#9	115	89	105	81	91	70	89	68	81	62	70	54
#10	129	100	118	91	102	79	100	77	91	70	79	61
#11	144	111	131	101	113	87	111	85	101	78	87	67

DESIGNER:

REVISIONS

REV REVISION DATE

DRAWING STATUS

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☐ PRELIMINARY DRAWINGS (NOT FOR CONSTRUCTION)
☐ CHECK SET (NOT FOR CONSTRUCTION)
☐ PLAN CHECK DRAWINGS.
☐ CONSTRUCTION DOCUMENTS.
☐ OTHER AS-BUILT

CONTENT

TYPICAL FOUNDATION DETAILS

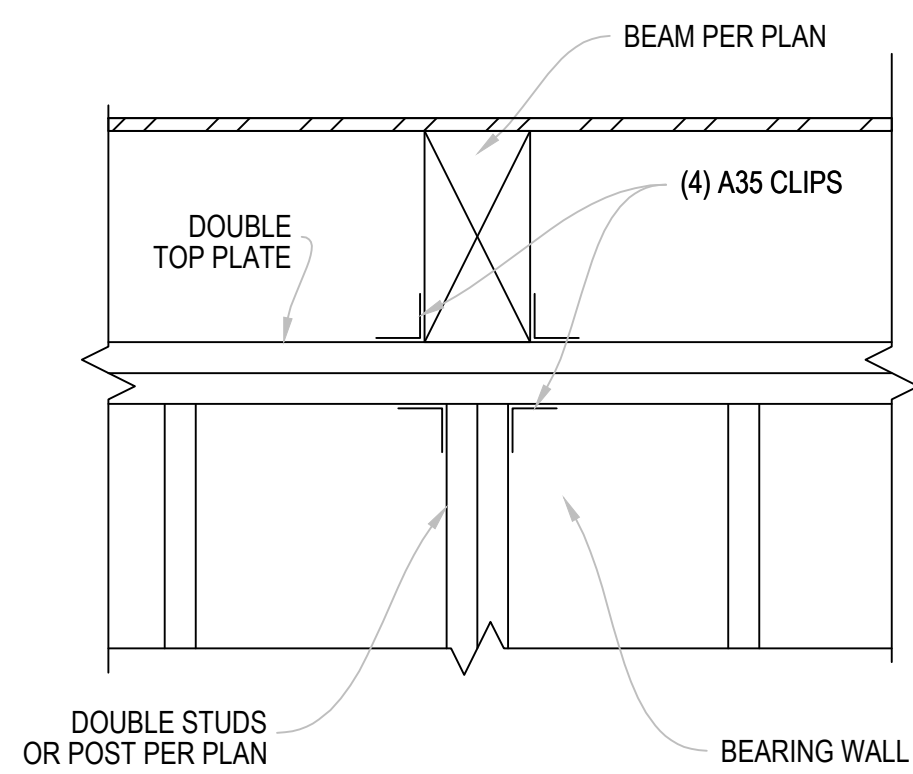
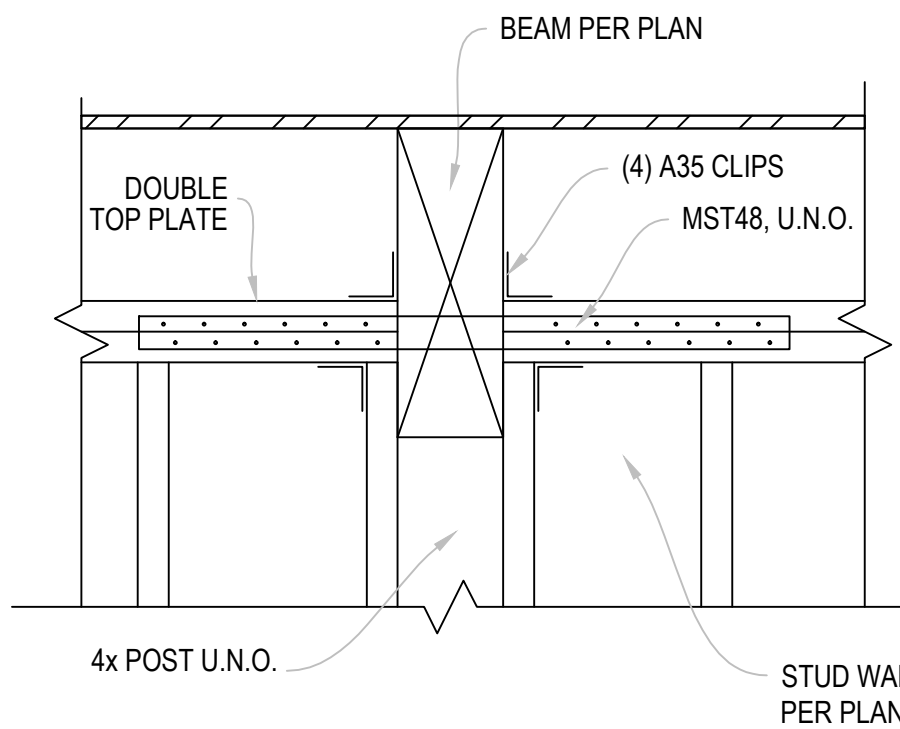
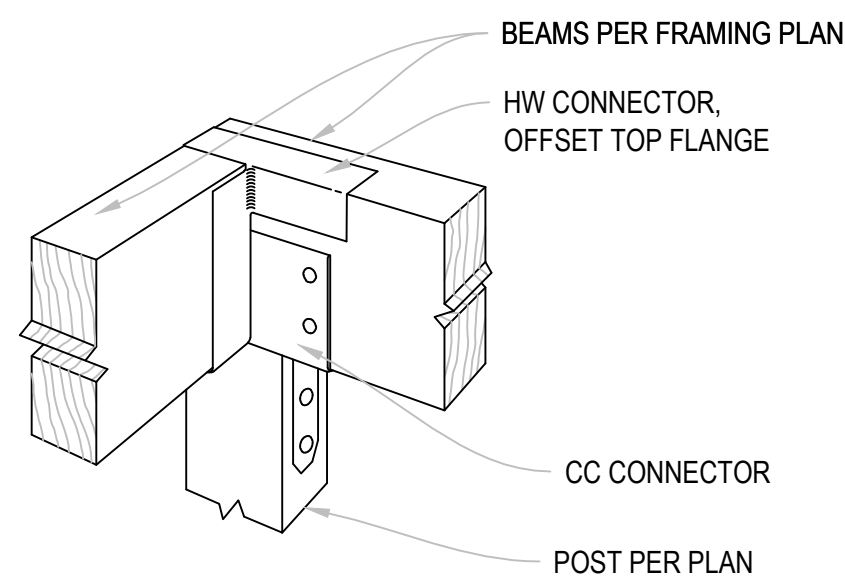
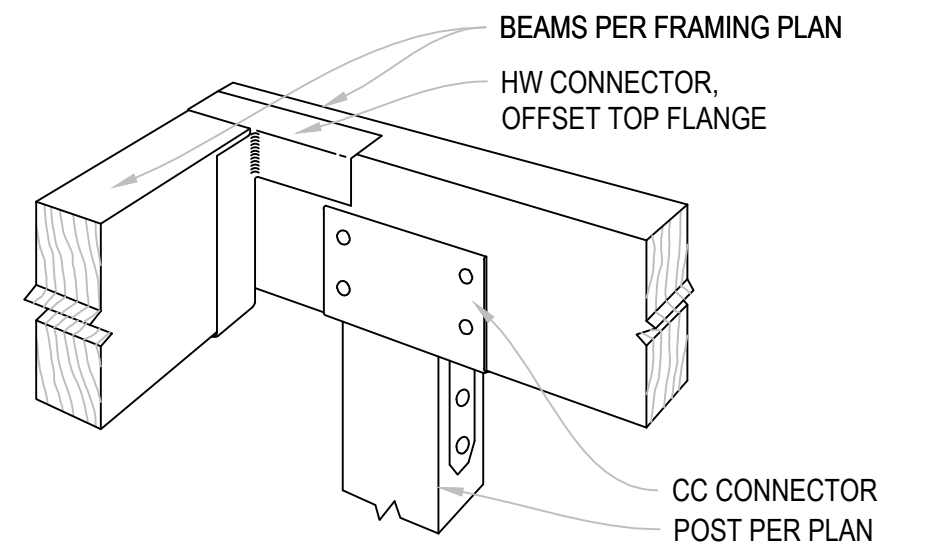
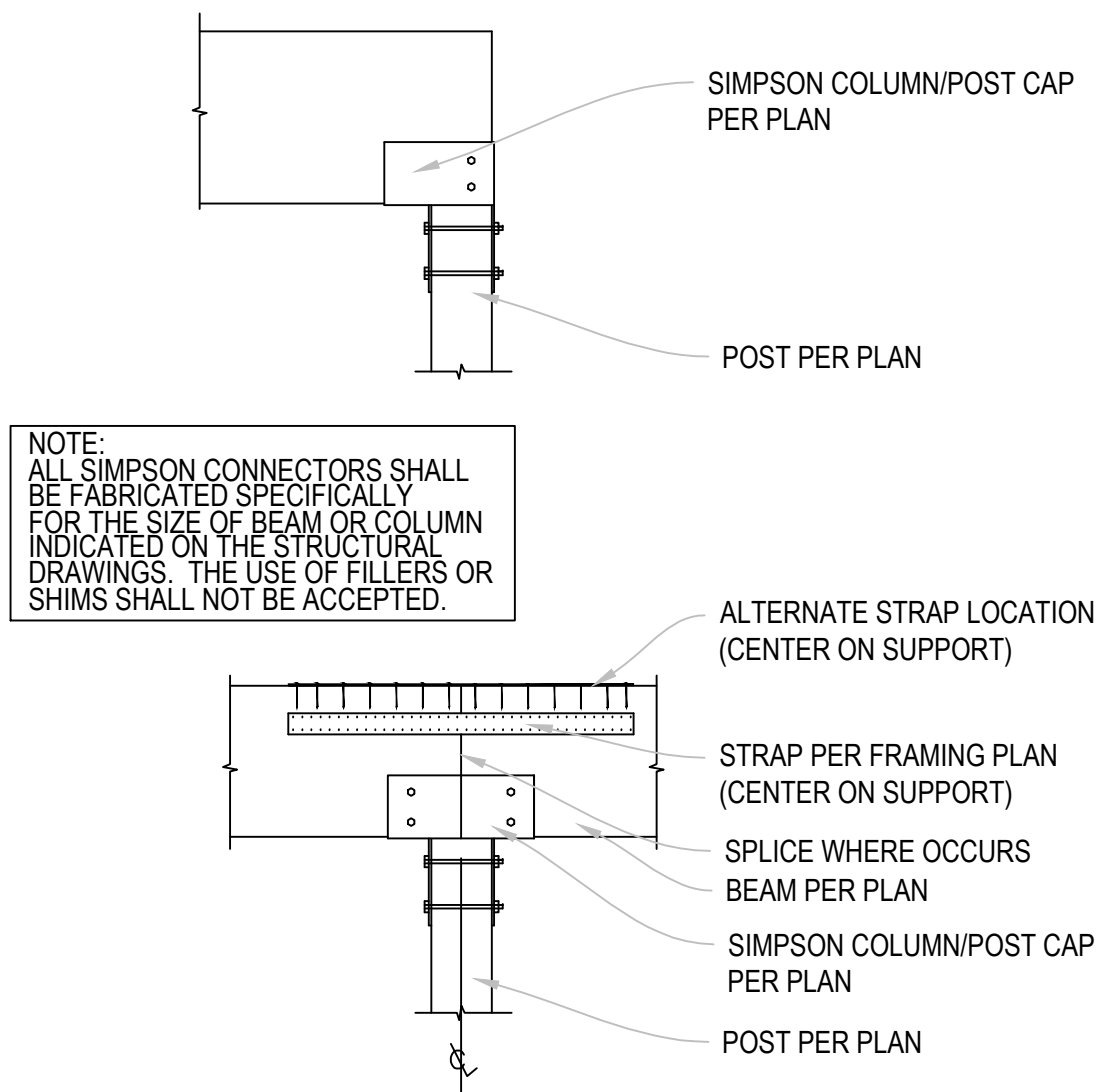
DRAWN BY: CR/CKP

CHECKED BY: CKP

DATE: SEPTEMBER 20, 2013

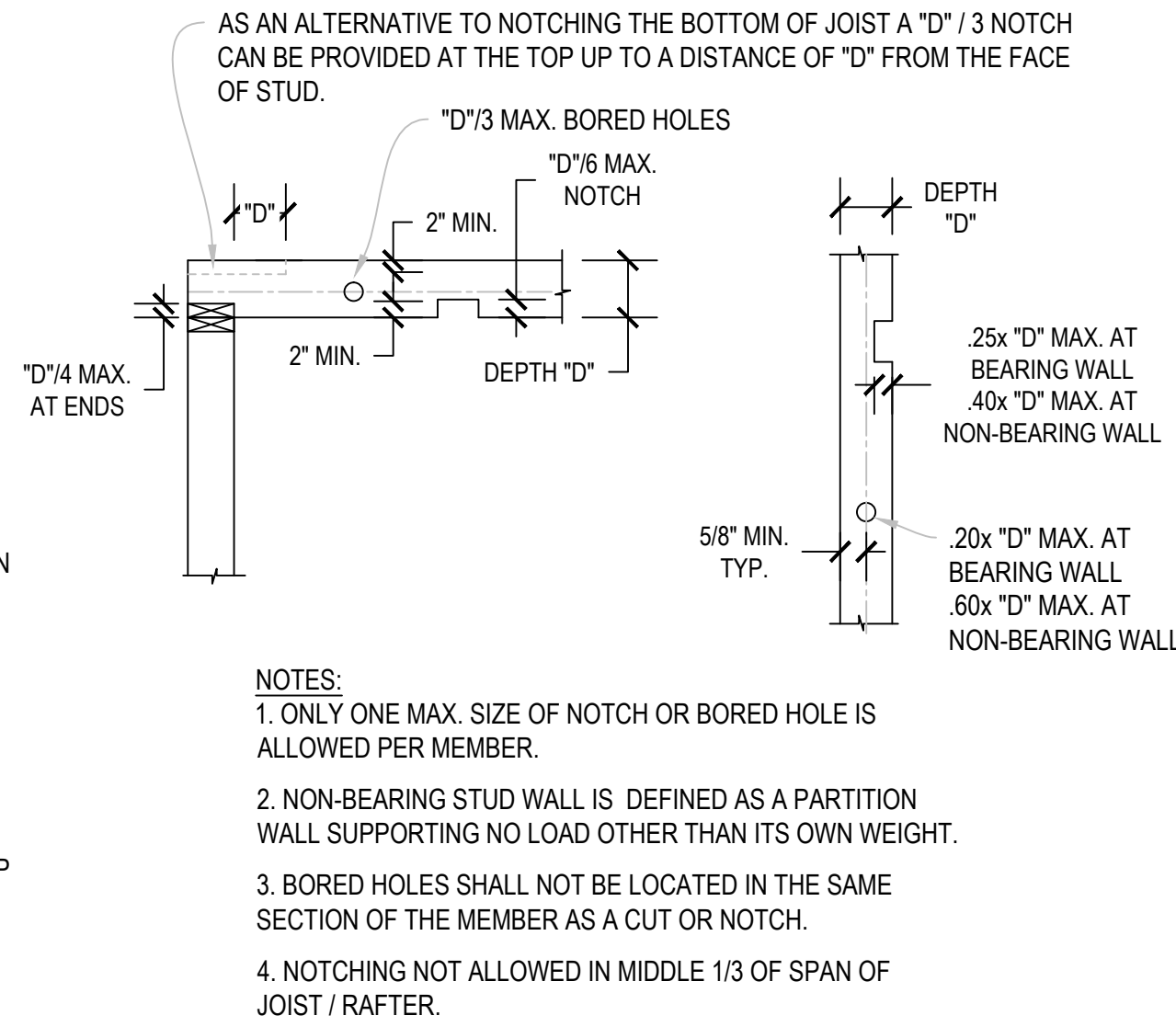
SHEET NO.

S2.0



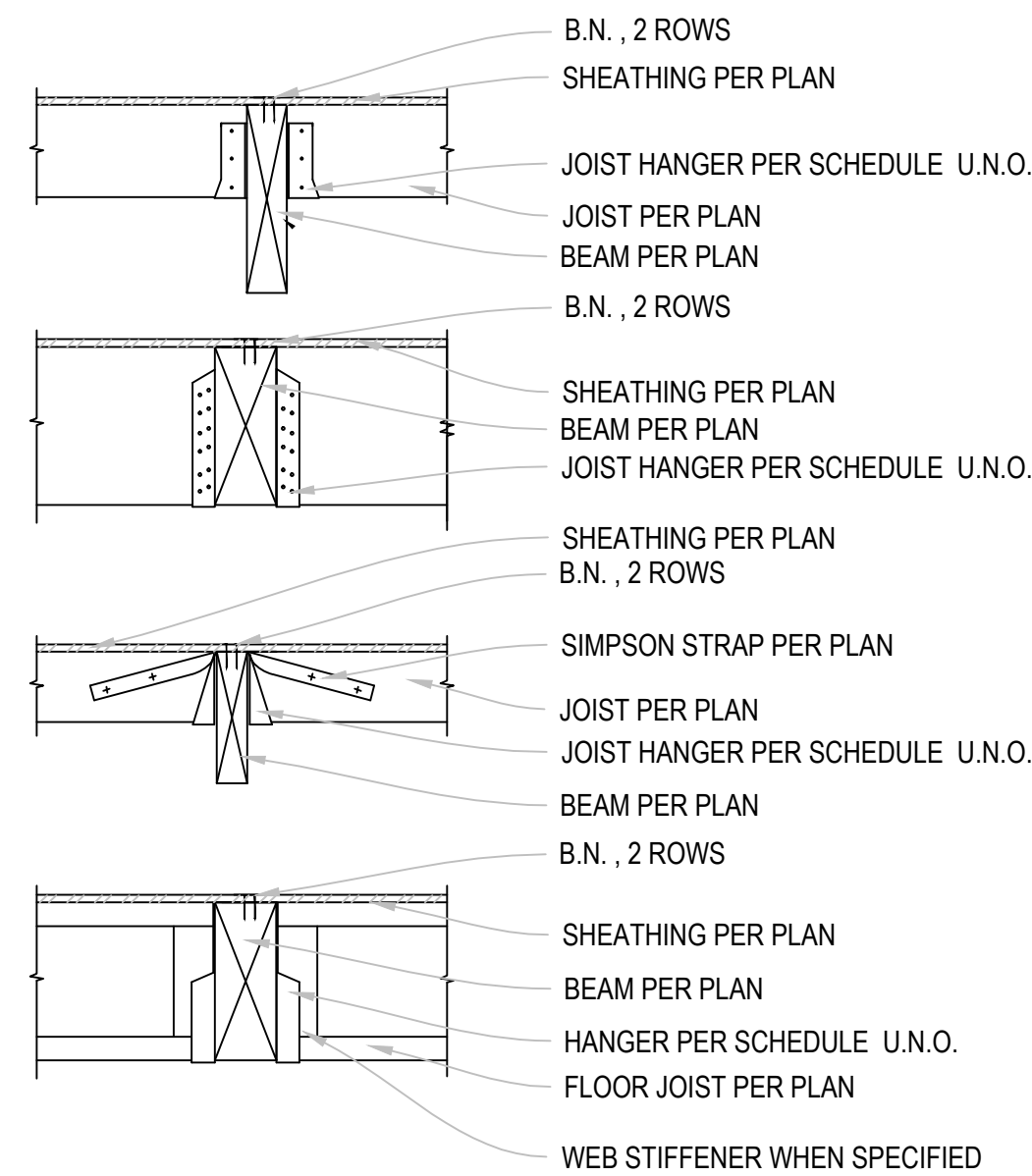
TYP. POST / WALL TO BEAM CONNECTION DETAILS

10



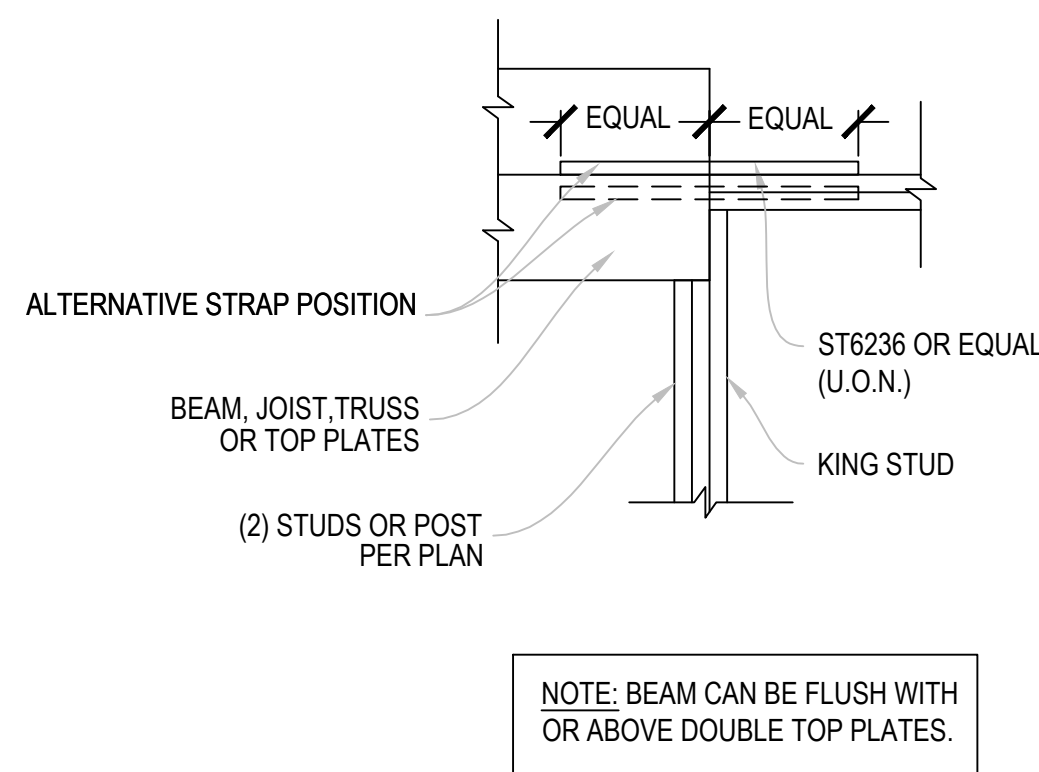
BORED HOLES & NOTCHING OF STUDS JOIST / RAFTER

7



TYPICAL BEAM CONNECTIONS DETAIL

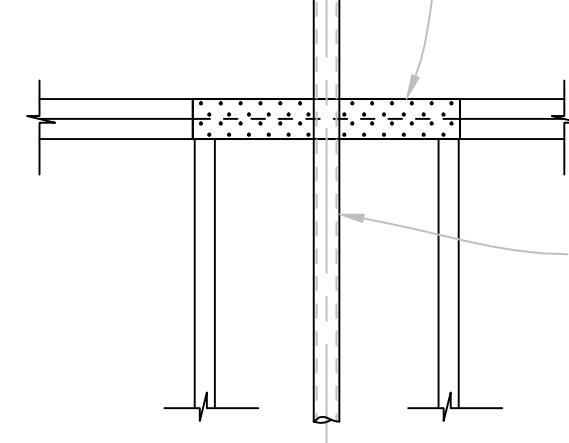
8



DRAG CONNECTION DETAIL

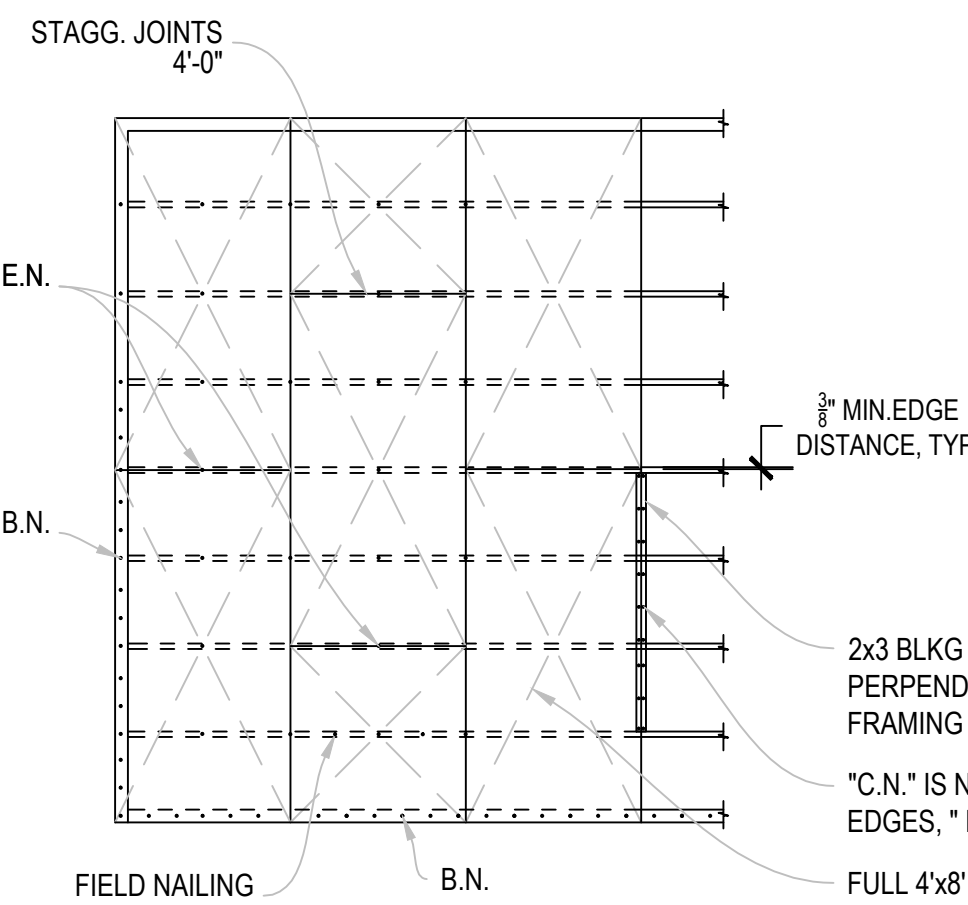
9

PLAN



TYP. PLATE SPLICE AT PLUMBING

MARK	SIZE	GRADE EXPOSURE	NAIL SIZE	B.N.	C.N.	E.N.	FIELD
5A	1/2"	CD-X(40/20)	10d	6"	6"	6"	12"
5B	3/4"	CD-X(48/24)	10d	6"	6"	6"	10"

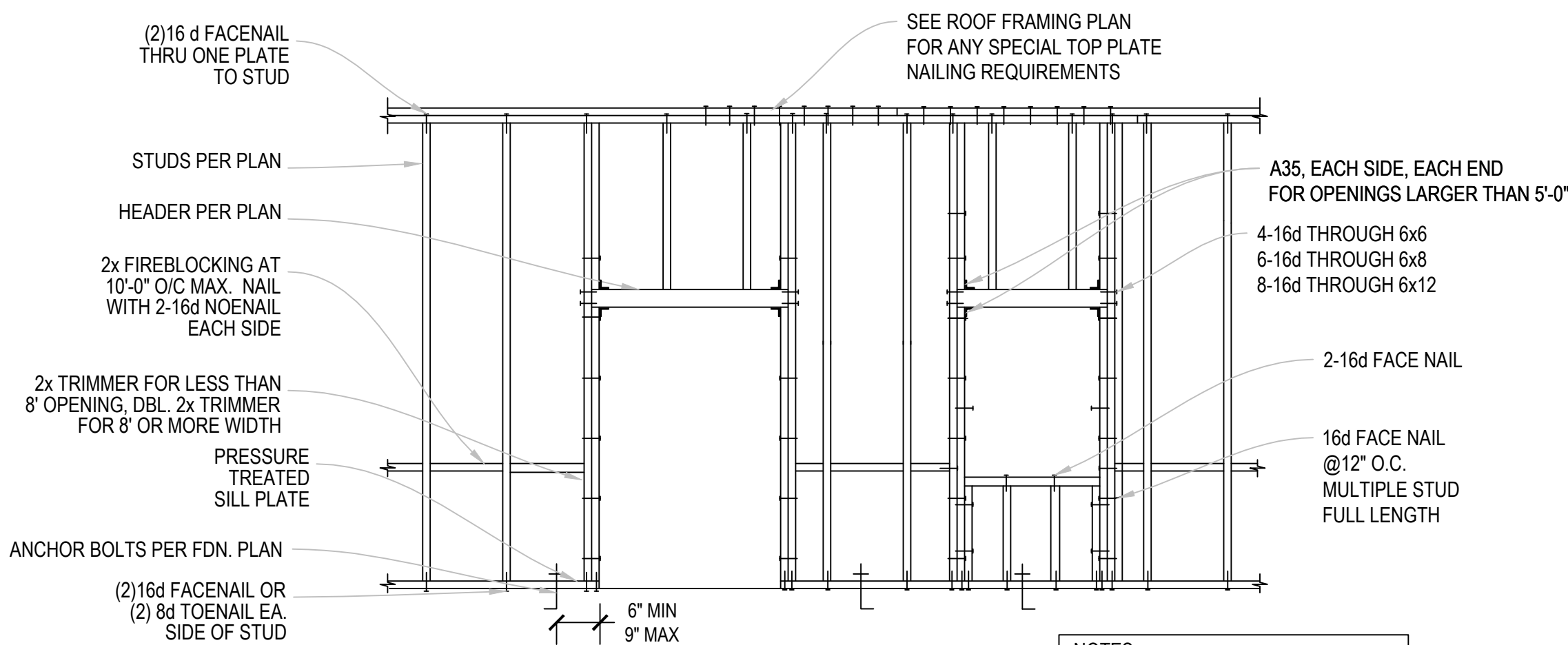


PLYWOOD FLOOR & ROOF SHEATHING SCHEDULE

5A

5B

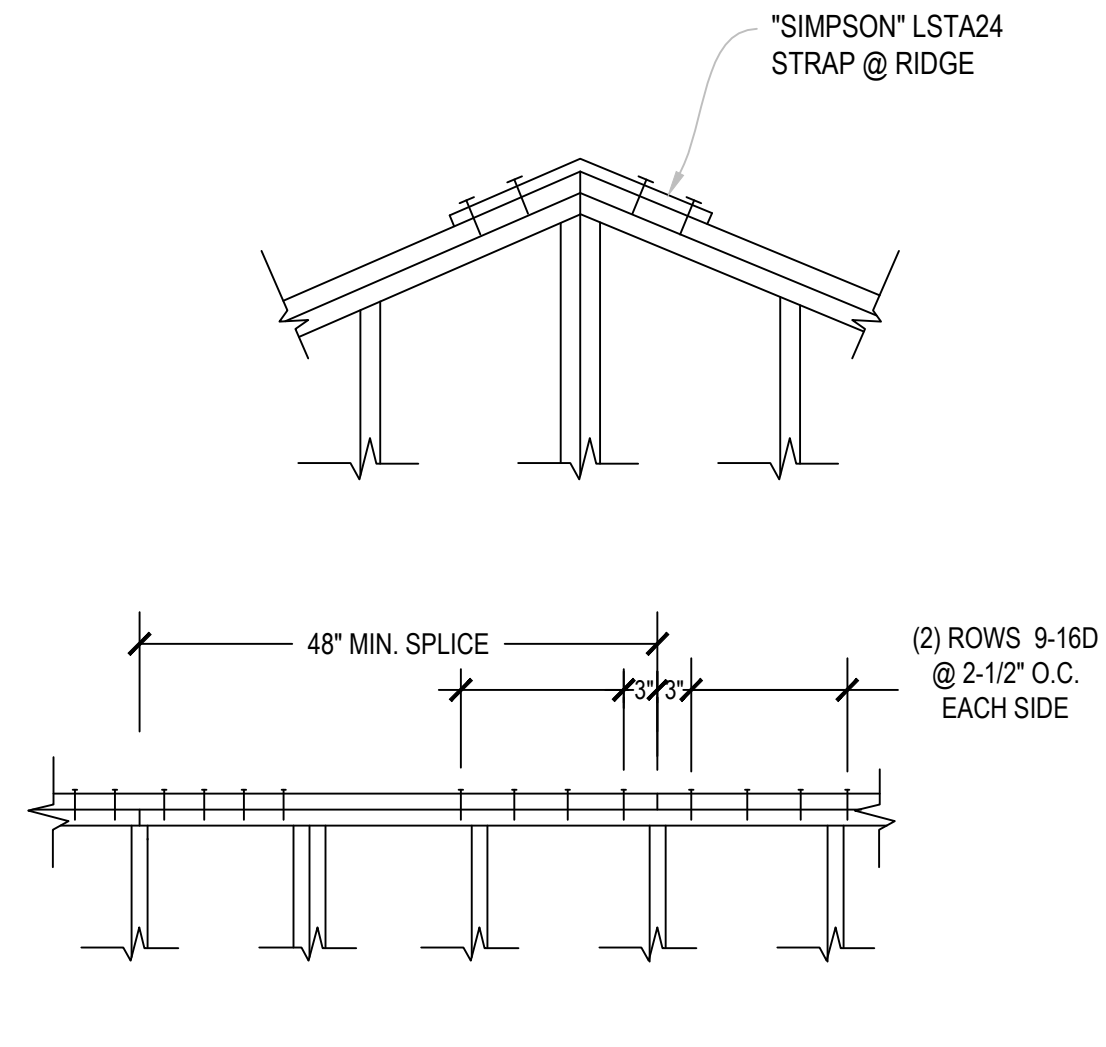
5



OPENING SIZE	4x MIN HEADER SIZE U.O.N.	TRIMMER STUDS	OPENING SIZE	6x MIN HEADER SIZE U.O.N.	TRIMMER STUDS
LESS THAN 3'-8"	(2) 2 x 4	1	LESS THAN 5'-0"	(3) 2 x 6	1
3'-8" < 5'-6"	4 x 6	2	5'-0" < 8'-0"	6 x 8	2
5'-6" < 7'-6"	4 x 8	2	8'-0" < 11'-0"	6 x 10	2
7'-6" < 9'-6"	4 x 10	3	11'-0" < 14'-0"	6 x 12	3
9'-6" < 11'-0"	4 x 12	4	14'-0" OR GREATER	SEE PLANS	4

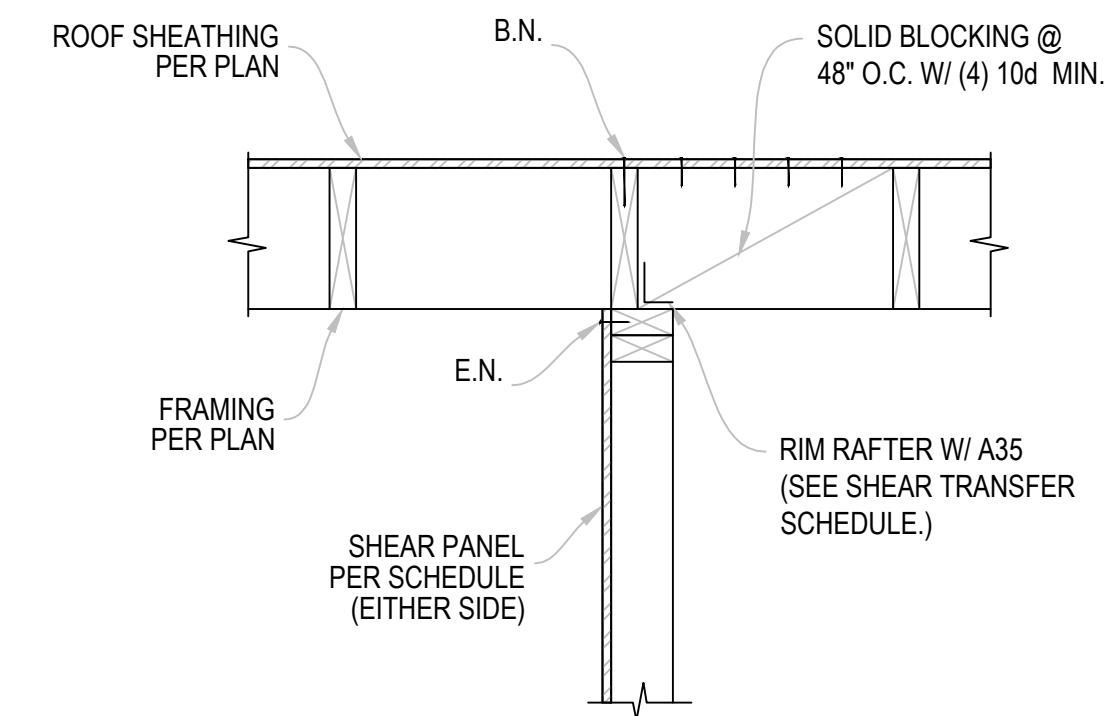
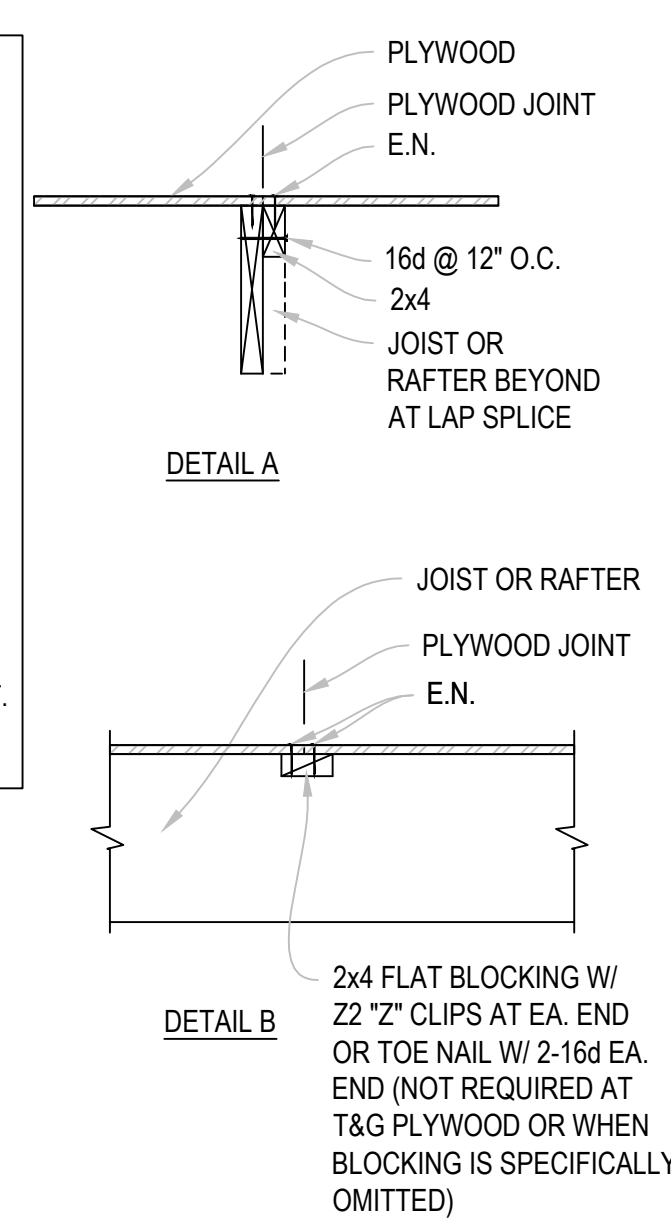
TYPICAL HEADER DETAIL

6

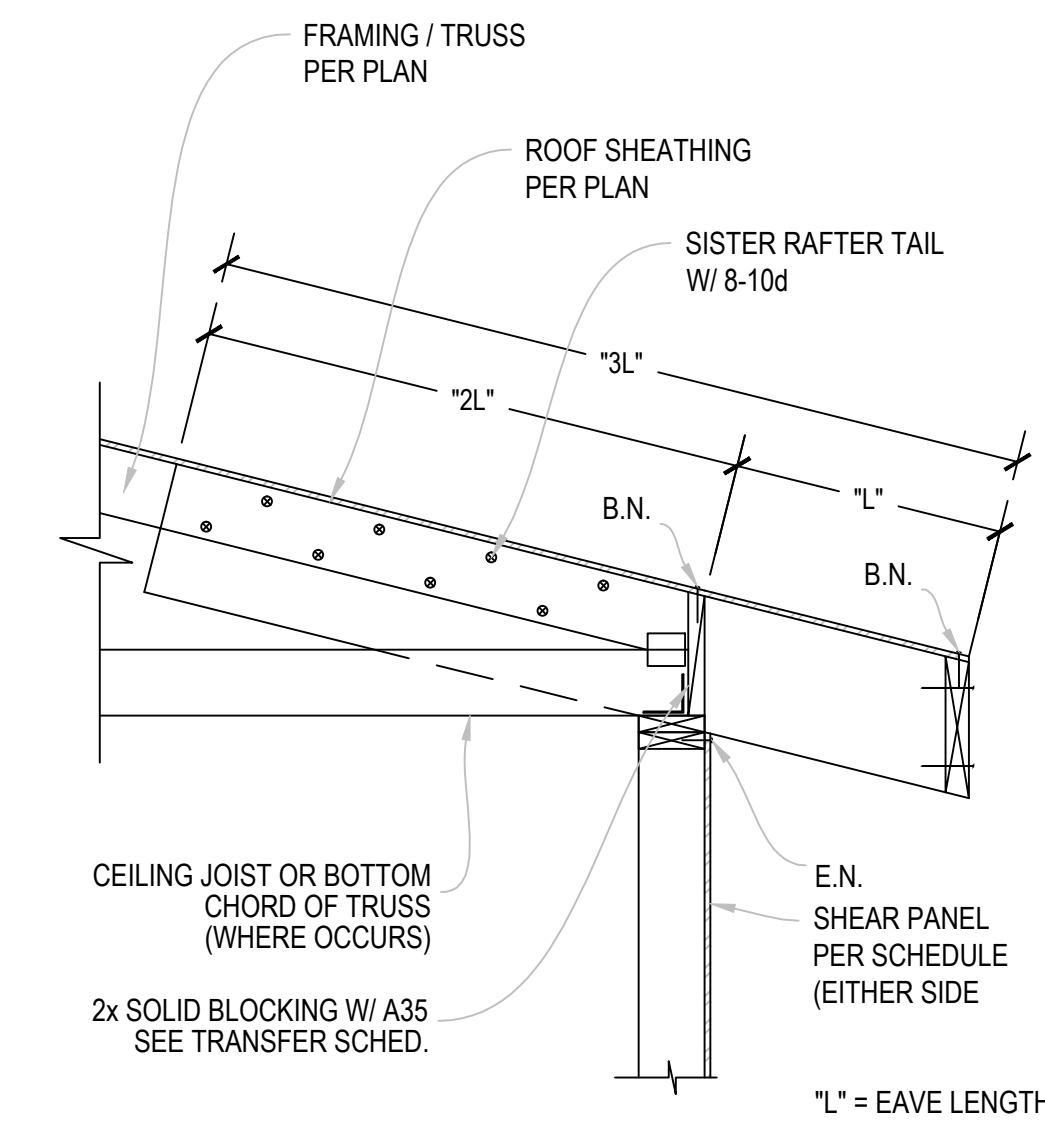


TYP. WALL PLATE SPLICE

4



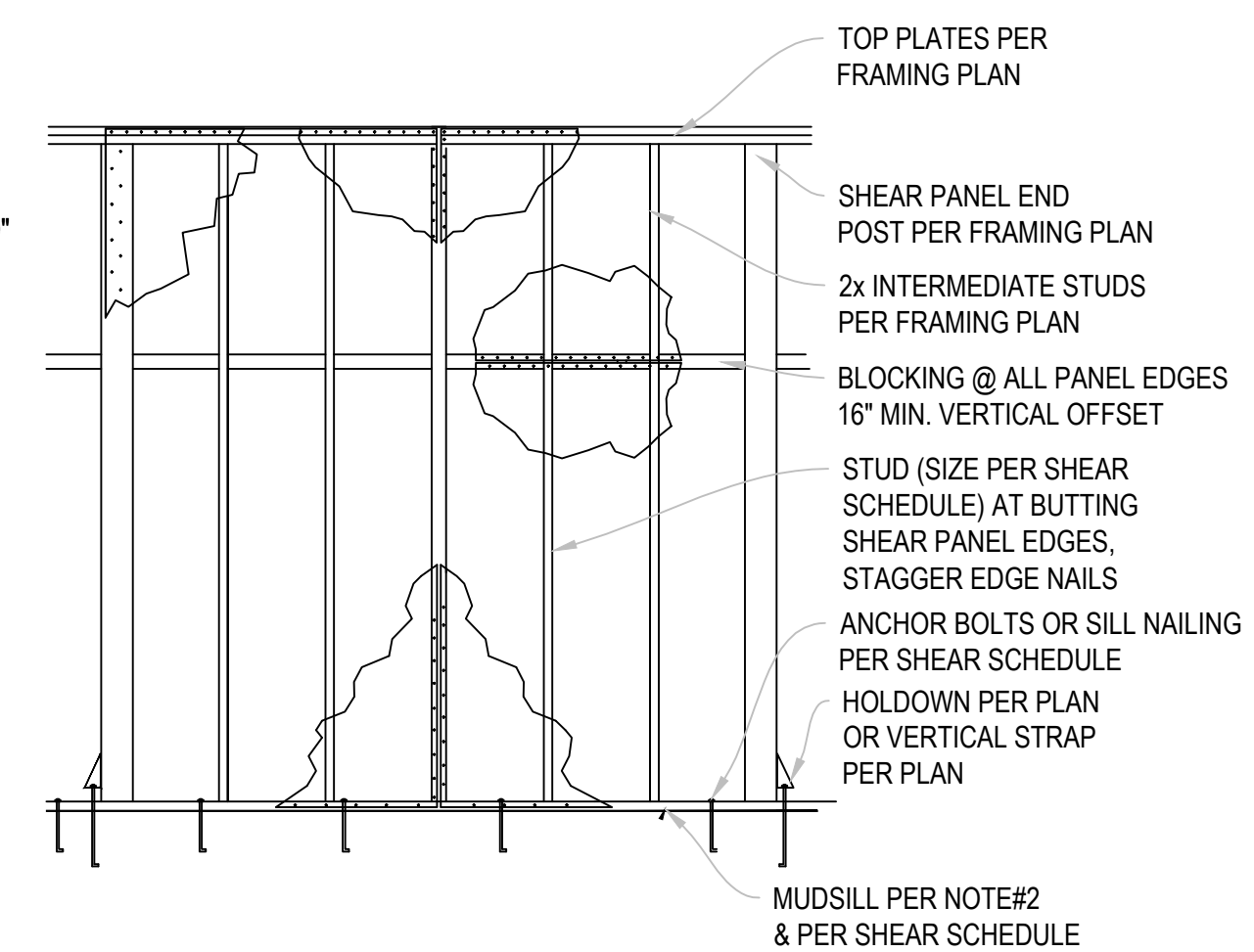
FRAMING PARALLEL TO WALL



FRAMING PERPENDICULAR TO WALL

FRAMING DETAIL

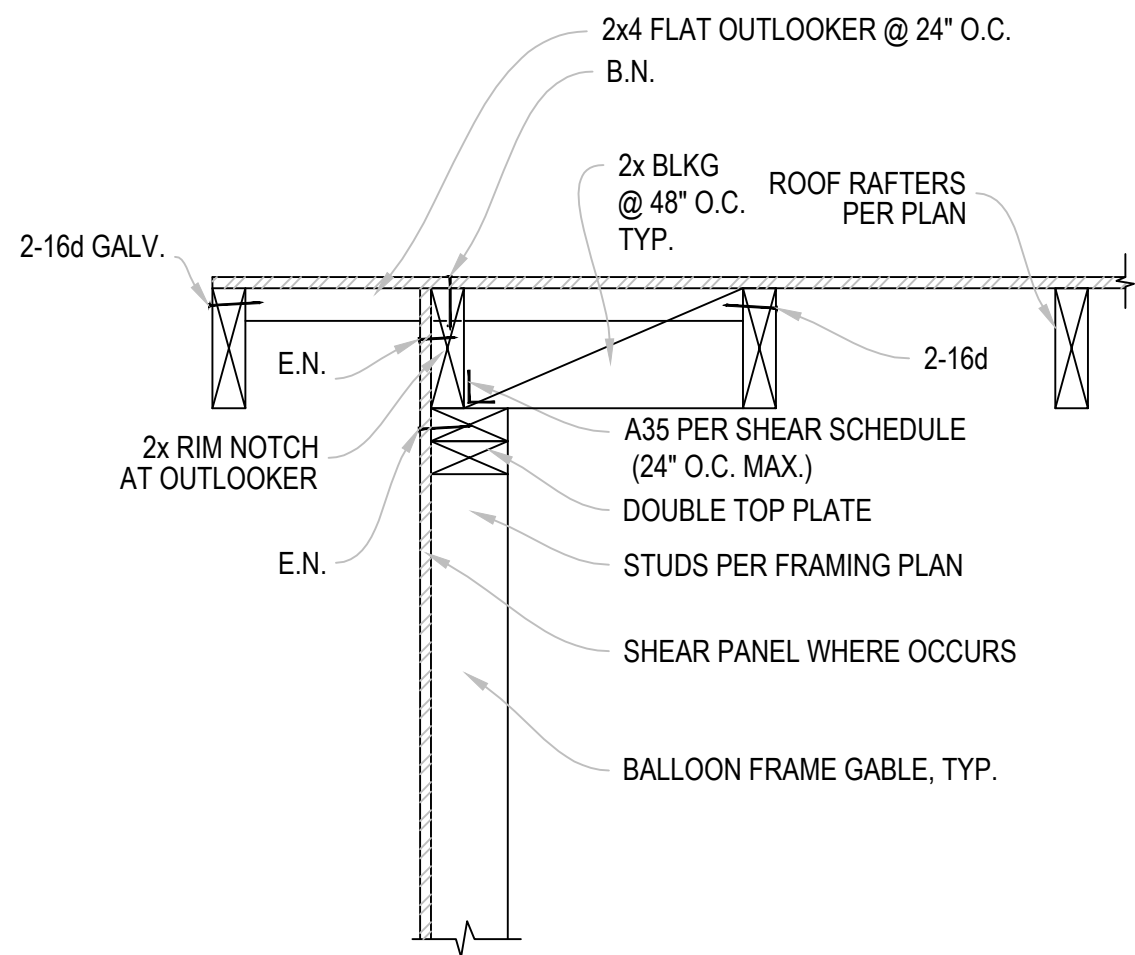
2



1. PLYWOOD JOINT AND SILL PLATE NAILING SHALL BE STAGGERED IN ALL CASES
2. WHERE 3x SILL PLATE IS REQUIRED PER SHEAR WALL SCHEDULE A 2x SILL PLATE MAY BE USED IF THE ANCHOR BOLT SPACING IS REDUCED BY 50%
3. PROVIDE 3"x3"x0.229" PLATE WASHERS AT ALL ANCHOR BOLTS

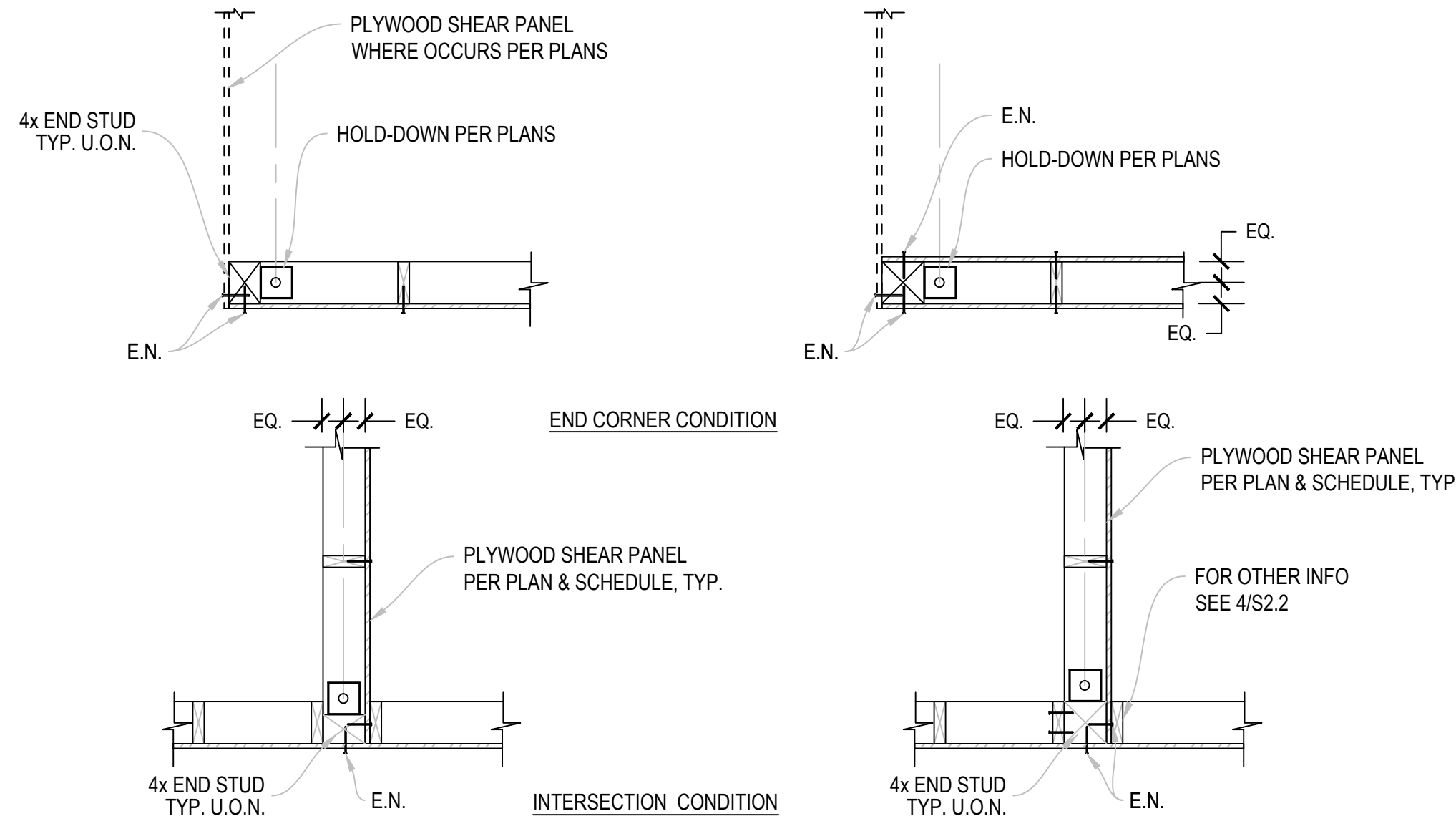
TYP. SHEAR WALL DETAIL

3



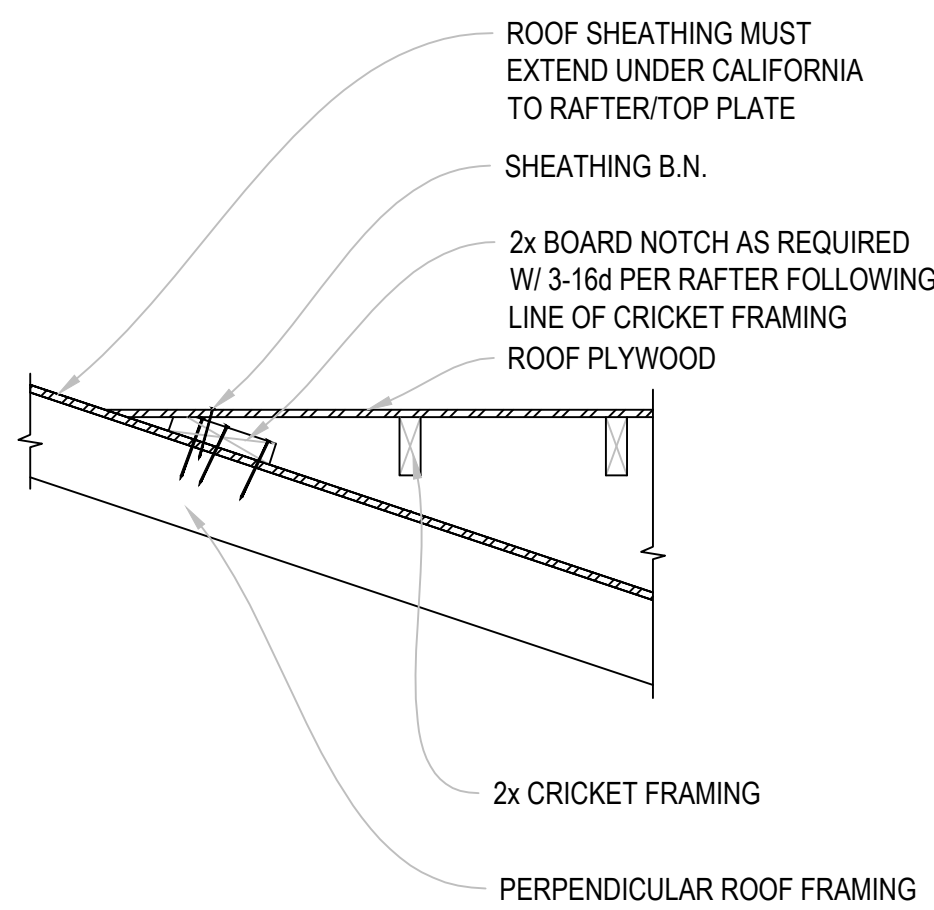
SHEAR TRANSFER, GABLE END

9



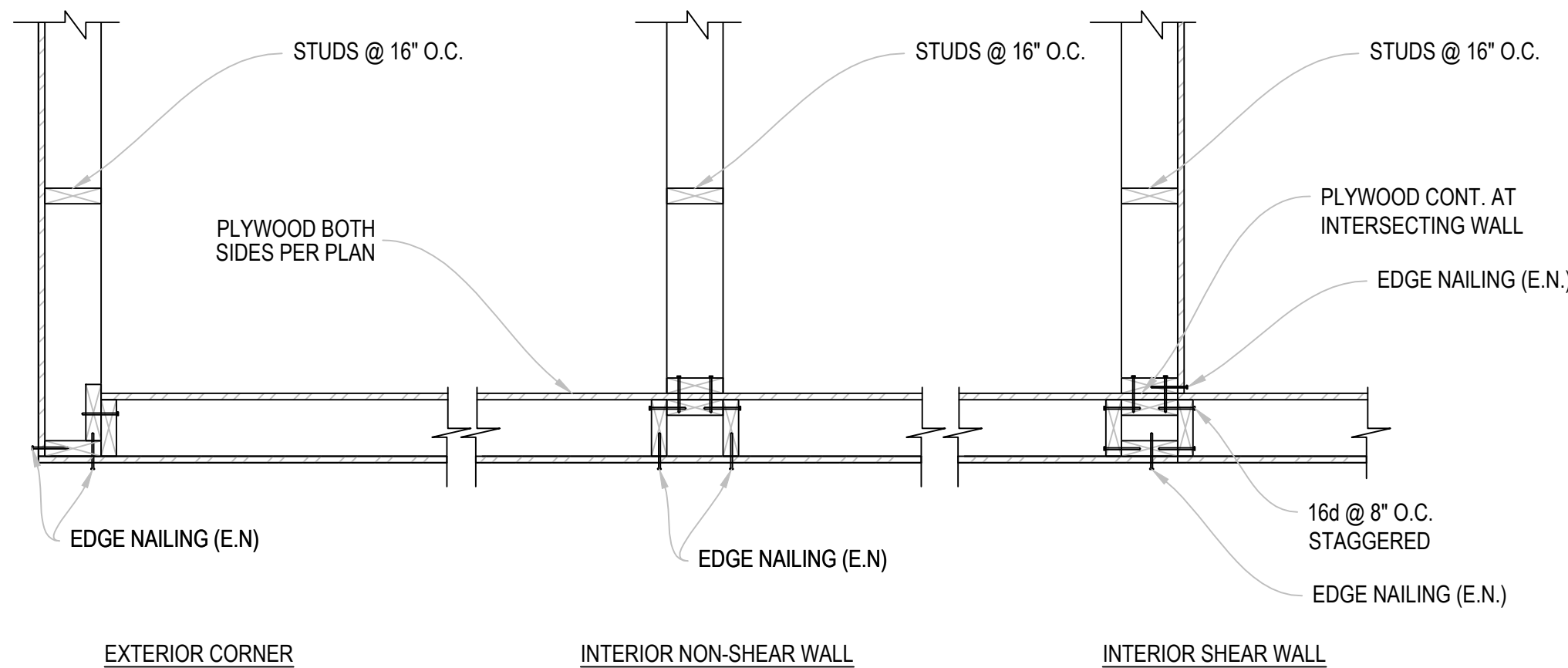
TYP. HOLD-DOWN LOCATION IN PLYWOOD SHEAR WALLS (TYP. PLAN SECTIONS)

3



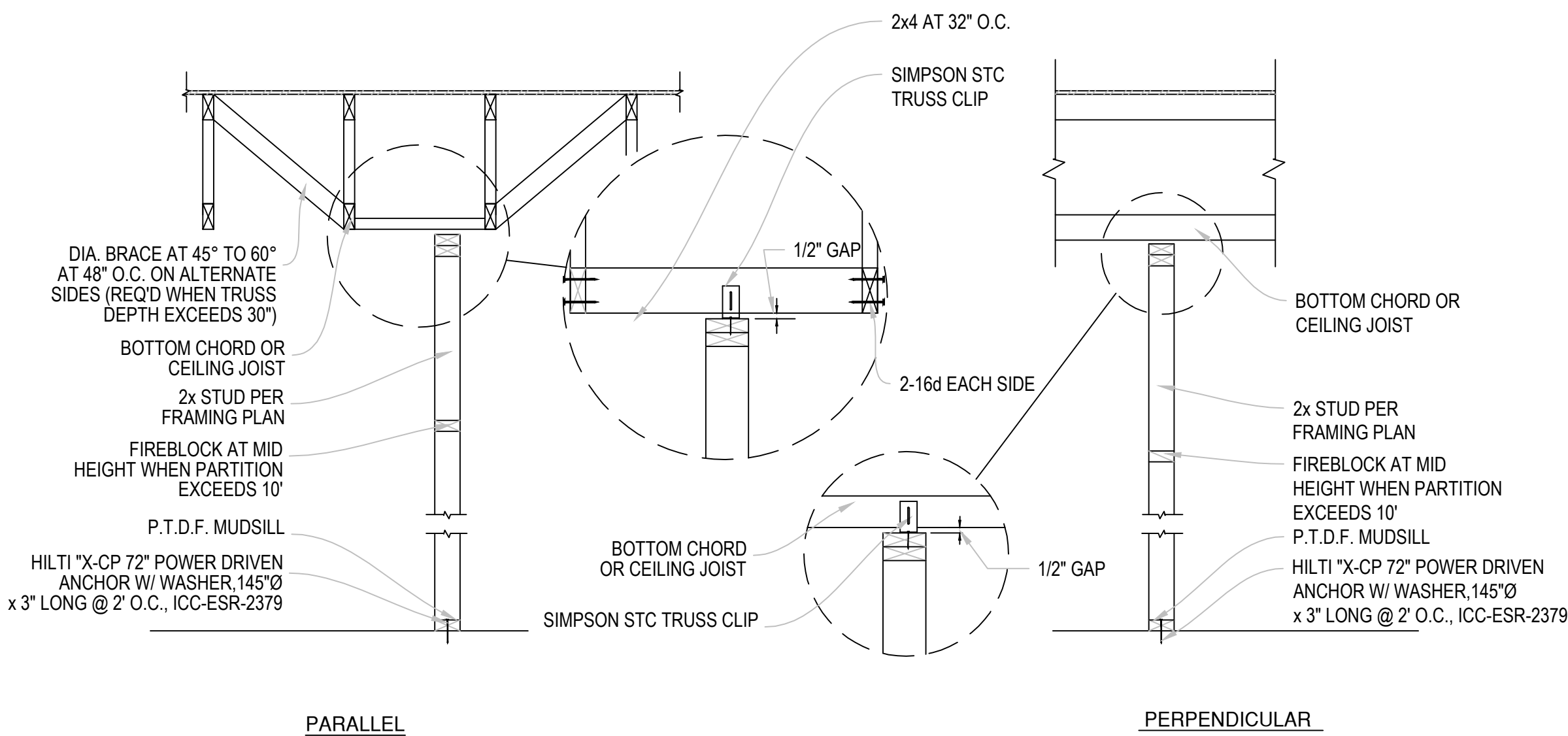
CAL-FILL SHEAR TRANSFER

10



STUD WALL INTERSECTIONS-PLAN

4



NON-BEARING PARTITION CONNECTION

5

FRAMING HANGER SCHEDULE (U.N.O.)				
MEMBER SIZE	FACE MOUNT HANGER	CAPACITY (LBS.)	TOP FLANGE HANGER	CAPACITY (LBS.)
2x4	LUS24	670	HU24TF	2090
2x6	LUS26	865	HU26TF	2275
2x8	LUS22	865	HU28TF	2335
2x10	LUS28	1100	HU210TF	2335
2x12	LUS210	1340	HU212TF	2335
4x6	HUS46	1065	HUS46TF	2700
4x8	HUS48	1595	BA48	3800
4x10	HUS410	2125	BA410	3800
4x12	HUS412	2660	BA412	3800
6x8	HUC68	2085	HU68TF	3500
6x10	HUC610	2680	HU610TF	4150
6x12	HUC612	3275	HU612TF	4550
8x8	HUC88	2085	HW88	5285
8x10	HUC810	2680	HW810	5285
8x12	HUC812	3275	HW812	5285
3-1/2x9-1/2 V.L.	HUS410	2125	LBV3.56/9.5	2590
3-1/2x11-7/8 V.L.	HUS412	2660	BA3.56/11.88	3800
5-1/4x9-1/2 V.L.	HUC610	2680	HWU5.5/9.5	6000
5-1/4x11-7/8 V.L.	HUC612	3275	HWU5.5/11.88	6000
7x9-1/2 V.L.	HUC410-2	2680	HWU7.12/9.5	6000
7x11-7/8 V.L.	HUC410-2	3275	HWU7.12/11.88	6000
3-1/8 x 10-1/2 GLB	HUC210-2	2680	WPU	4165
3-1/8 x 12 GLB	HUC212-2	3275	WPU	4165
3-1/8 x 16-1/2 GLB	HUC216-2	3870	WPU	4165
5-1/8 x 10-1/2 GLB	HUC5.125/12	3275	WPU	4165
5-1/8 x 12 GLB	HUC5.125/12	3275	WPU	4165
5-1/8 x 13-1/2 GLB	HUC5.125/13.5	3870	WPU	4165
5-1/8 x 16 GLB	HUC5.125/16	3870	WPU	4165
9-1/2 BCI 5000	IUS2.06/9.5	950	ITS 2.06/9.5	1520
9-1/2 BCI 6000	IUS2.37/9.5	950	ITS 2.37/9.5	1520
9-1/2 BCI 6500	IUS2.56/9.5	950	ITS 2.56/9.5	1520
9-1/2 BCI 60	IUS2.37/9.5	950	ITS 2.37/9.5	1520
9-1/2 BCI 90	IUS3.56/9.5	950	ITS 3.56/9.5	1520
11-7/8 BCI 5000	IUS2.06/11.88	1185	ITS 2.06/11.88	1520
11-7/8 BCI 6000	IUS2.37/11.88	1185	ITS 2.37/11.88	1520
11-7/8 BCI 6500	IUS2.56/11.88	1185	ITS 2.56/11.88	1520
11-7/8 BCI 60	IUS2.37/11.88	1185	ITS 2.37/11.88	1520
11-7/8 BCI 90	IUS3.56/11.88	1420	ITS 3.56/11.88	1520

TYPICAL FRAMING HANGER SCHEDULE

1

CEILING JOIST SCHEDULE U.N.O.	
SPACING SCHEDULE @ 12"	SCHEDULE @ 16" MAX
SIZE	SIZE
2 x 4 12" O.C. - 12'-5"	2 x 4 16" O.C. - 11'-3"
2 x 6 12" O.C. - 19'-6"	2 x 6 16" O.C. - 17'-8"
2 x 8 12" O.C. - 25'-8"	2 x 8 16" O.C. - 23'-0"
2 x 10 12" O.C. - 26'-0"	2 x 10 16" O.C. - 26'-0"
SCHEDULE @ 24"	FOOTNOTES:
2 x 4 24" O.C. - 9'-10"	1. REFER TO 2010 I.B.C. TABLE 2308.10.2(1)
2 x 6 24" O.C. - 14'-10"	2. USE DF No 2 L.L. 10 PSF, D.L. 5 PSF
2 x 8 24" O.C. - 18'-9"	3. FOR GARAGE USE @16" O.C. ONLY.
2 x 10 24" O.C. - 22'-11"	

TYPICAL CEILING JOIST SCHEDULE

2



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MEBUST RESIDENCE

5448 TAFT AVENUE
LA JOLLA, CA 92037
PROJECT #: 2013-0070

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DESIGNER:

REVISIONS

REV	REVISION	DATE

DRAWING STATUS

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- ☐ PLAN CHECK DRAWINGS.
- ☐ CONSTRUCTION DOCUMENTS.
- ☐ OTHER AS-BUILT

CONTENT

TYPICAL
FRAMING
DETAILS

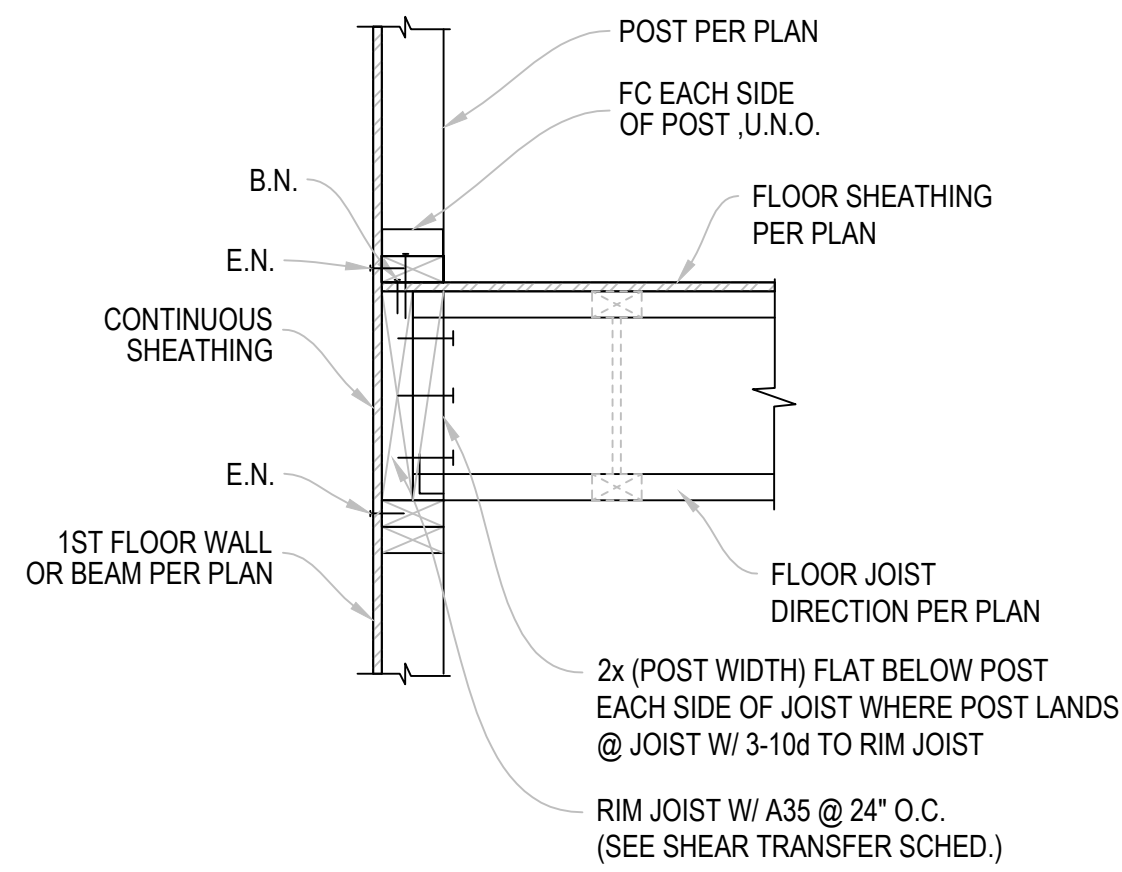
DRAWN BY: CR/CKP

CHECKED BY: CKP

DATE: SEPTEMBER 20, 2013

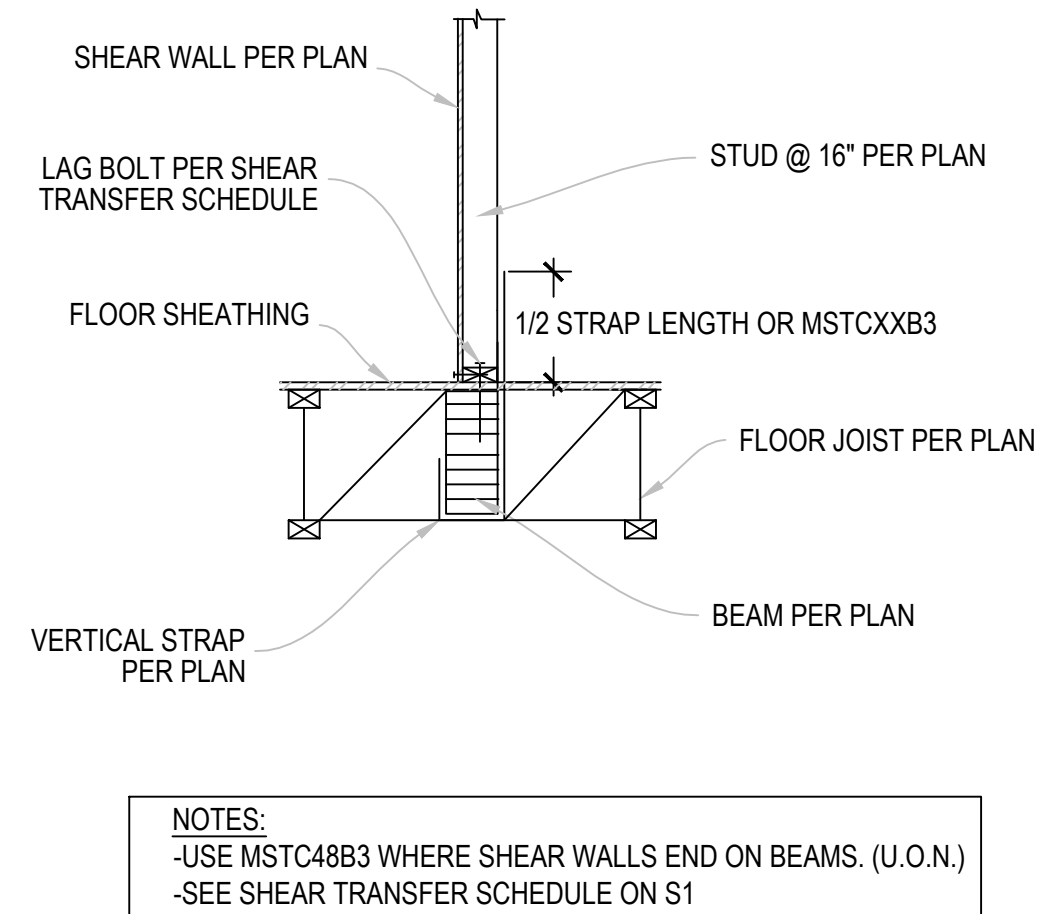
SHEET NO.

S2.2



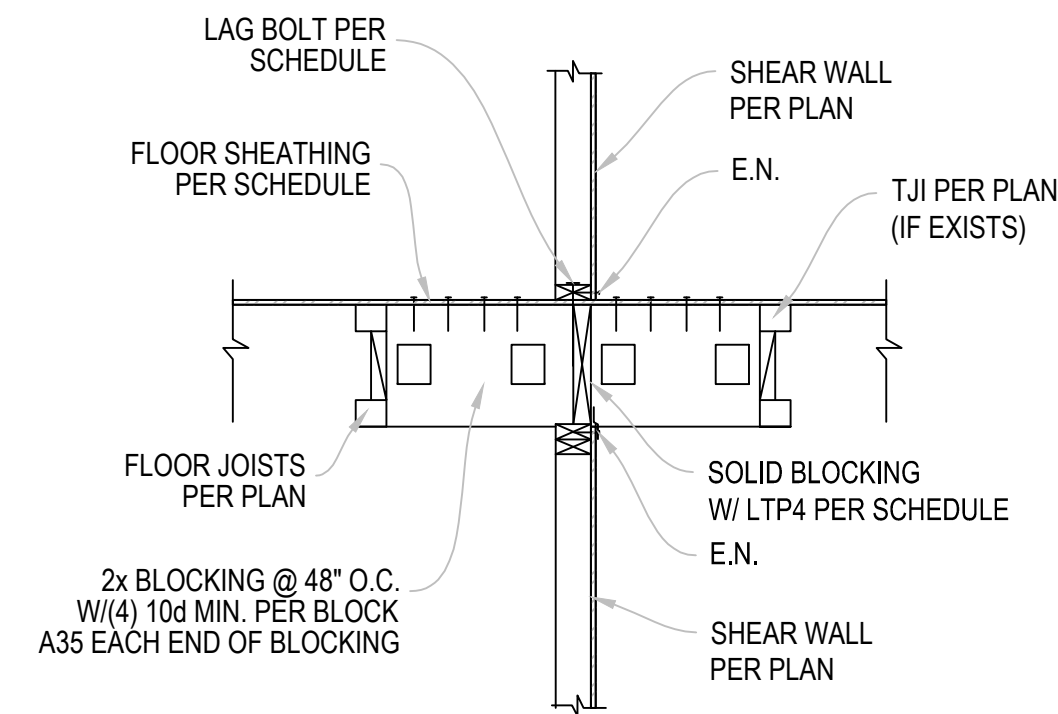
POST AT FLOOR FRAMING DETAIL

7

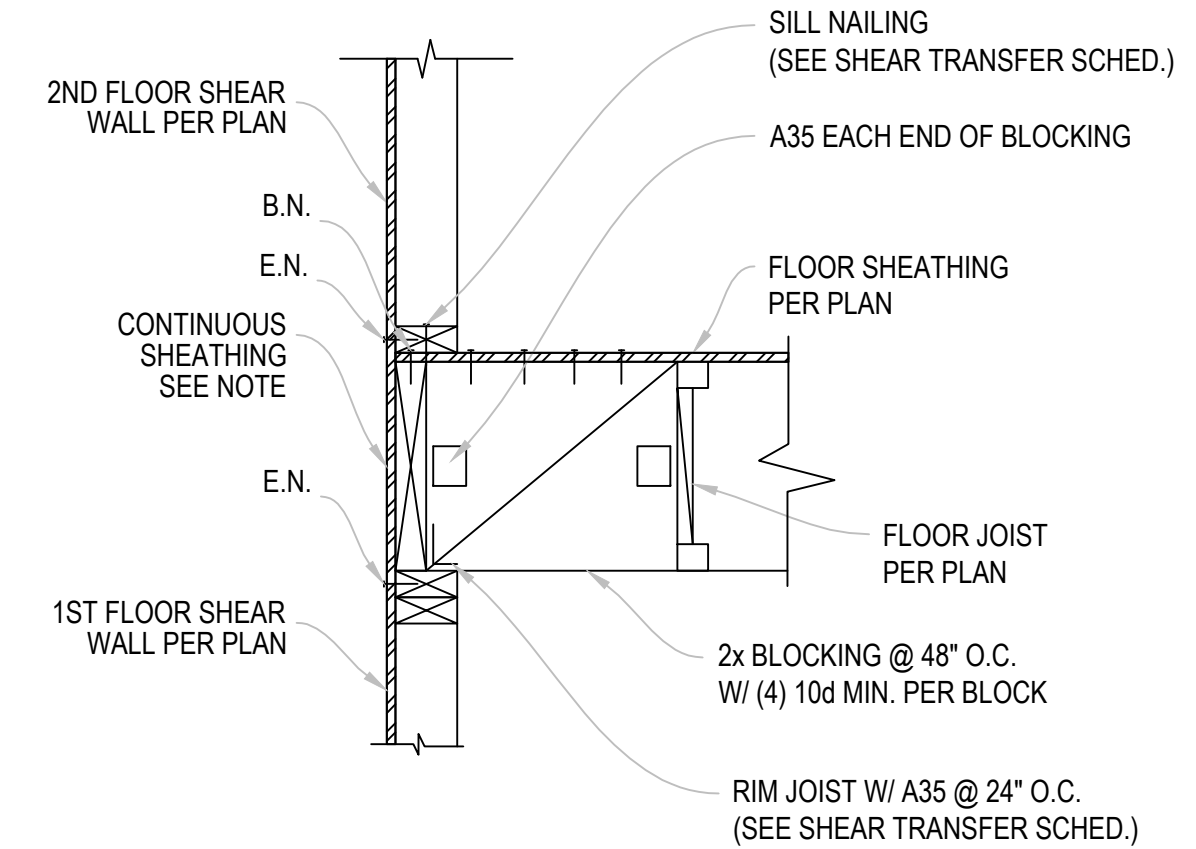


SHEAR TO DIAPHRAGM

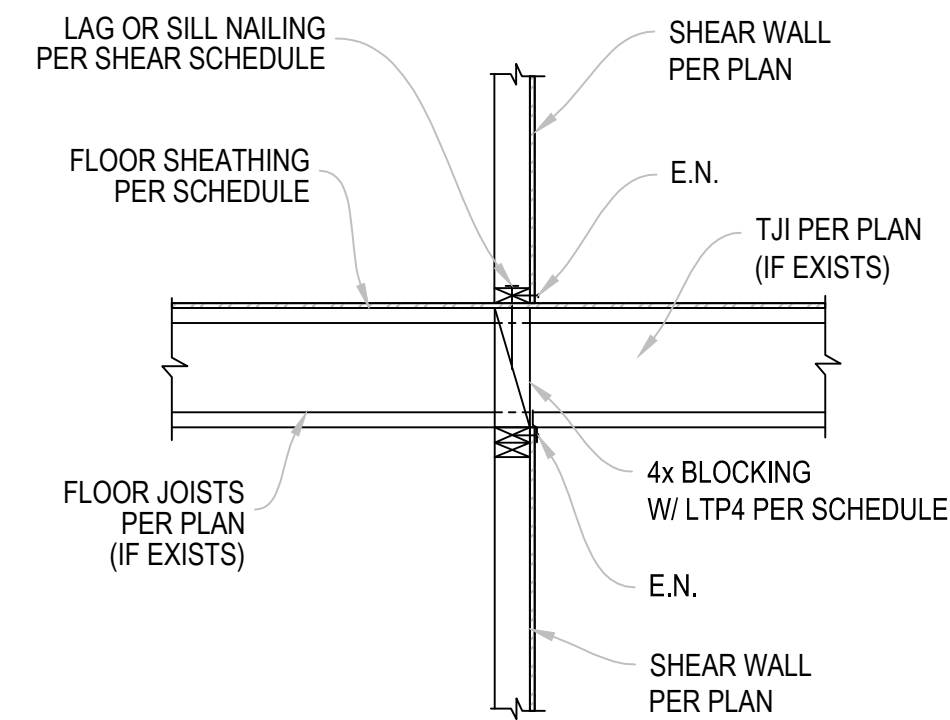
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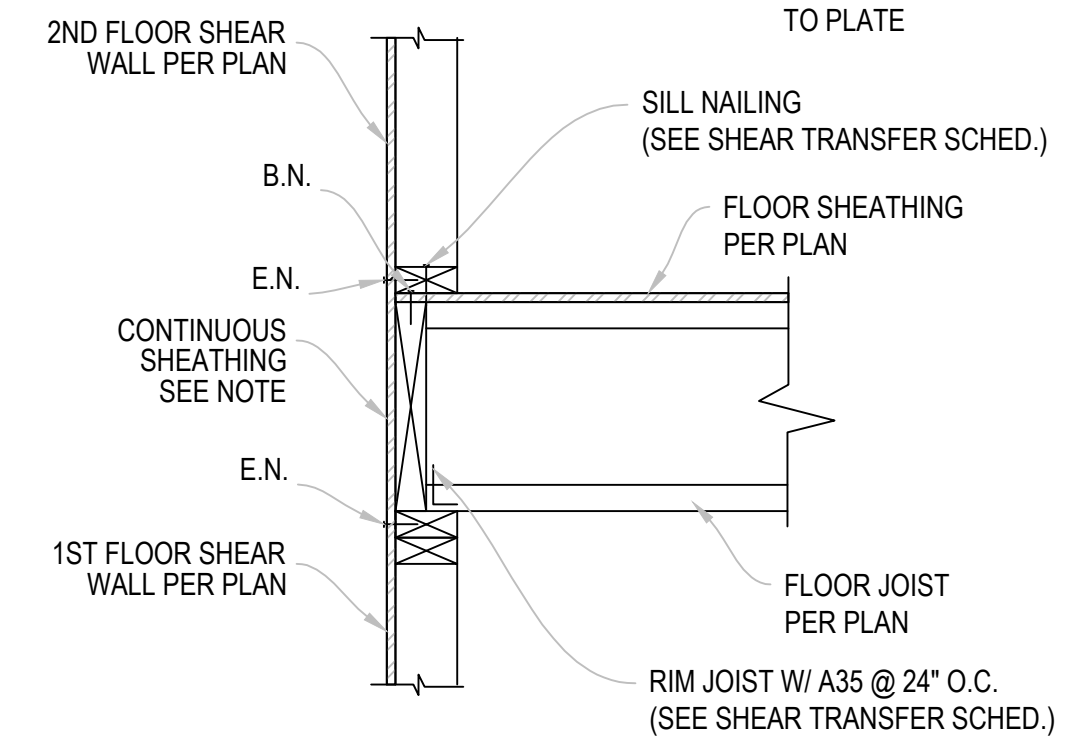
FLOOR JOIST PARALLEL TO WALL



NOTE: IF 1ST FLR. SHEAR WALL DOES NOT EXIST, SHEATHING MUST EXTEND PAST THE TOP PLATE OF THE 1ST FLR. TO ALLOW EN TO PLATE

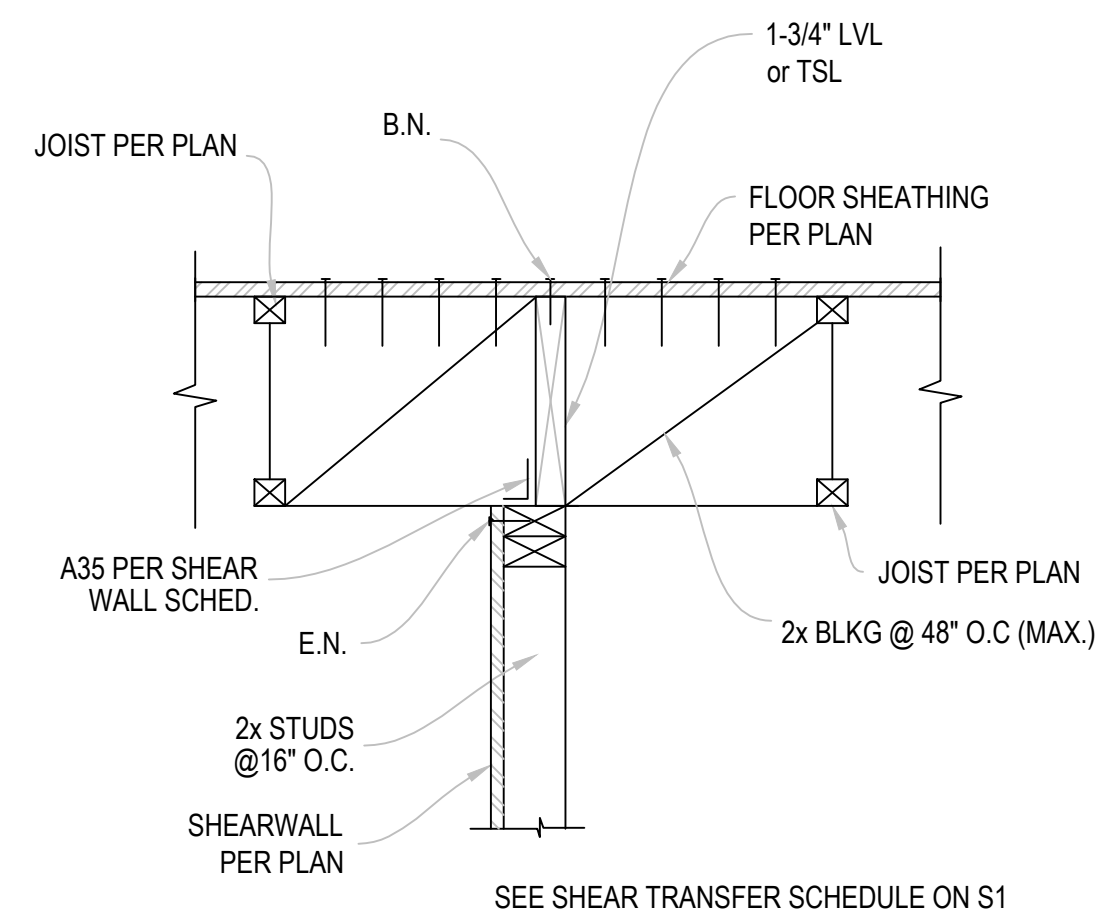


FLOOR JOIST PERPENDICULAR TO WALL



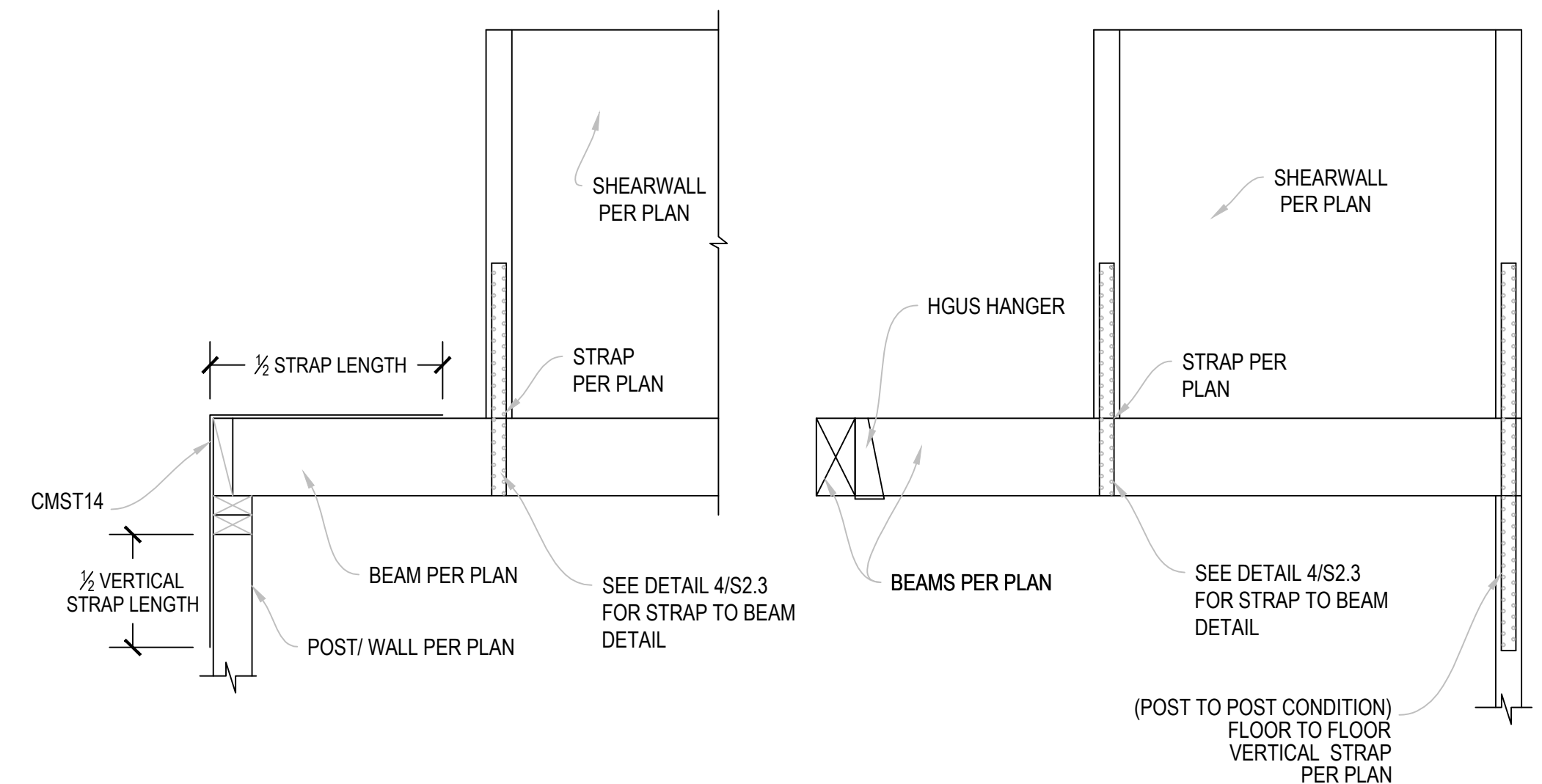
2-STORY SHEAR TRANSFER

2



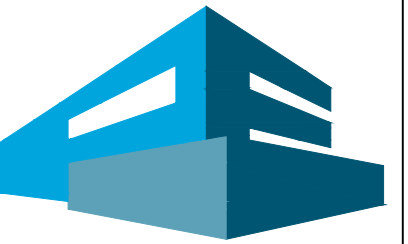
FLOOR SHEAR TRANSFER DETAIL

6



FLOOR STRAPPING

3



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- ☐ CONSTRUCTION DOCUMENTS.
- ☐ OTHER AS-BUILT

CONTENT

TYPICAL
FRAMING
DETAILS

DRAWN BY: CR/CKP

CHECKED BY: CKP

DATE: SEPTEMBER 20, 2013

SHEET NO.

S2.3

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1/4" = 1'-0"
DO NOT SCALE PLANS FOR CONSTRUCTION
DIMENSIONS. ALL CONSTRUCTION DIMENSIONS
MUST BE VERIFIED WITH THE ARCH. PLANS.

NOTES:

1. HOLDOWNS MUST BE TIED INTO PLACE PRIOR TO CALLING FOR INSPECTION. FOR HOLDOWN DETAILS SEE 12/S2.0 & 12/S4.0.
2. THE SPECIAL INSPECTOR IS TO VERIFY THE DRILLING OF ANY HOLES, THE CLEANLINESS OF THE HOLE, THE MOISTURE IN THE HOLE, MIXING THE EPOXY, THE BRAND OF EPOXY AND THE PROPER MATERIAL FOR THE ASSEMBLY.
3. SEE SHEET S-1.1 FOR ADDITIONAL REQUIREMENTS FOR FOUNDATION SPECIAL INSPECTIONS.
4. SEE SHEET S-1.0 FOR ADDITIONAL FOUNDATION NOTES.
5. ARCHITECTS DIMENSIONS TAKE PRECEDENCE. CONTACT THE ENGINEER WITH DISCREPANCIES.
6. SEE SOILS REPORT FOR ADDITIONAL SUBGRADE, WATERPROOFING, AND DRAINAGE REQUIREMENTS.
7. SHEAR WALLS AT THIS LEVEL ARE ABOVE.

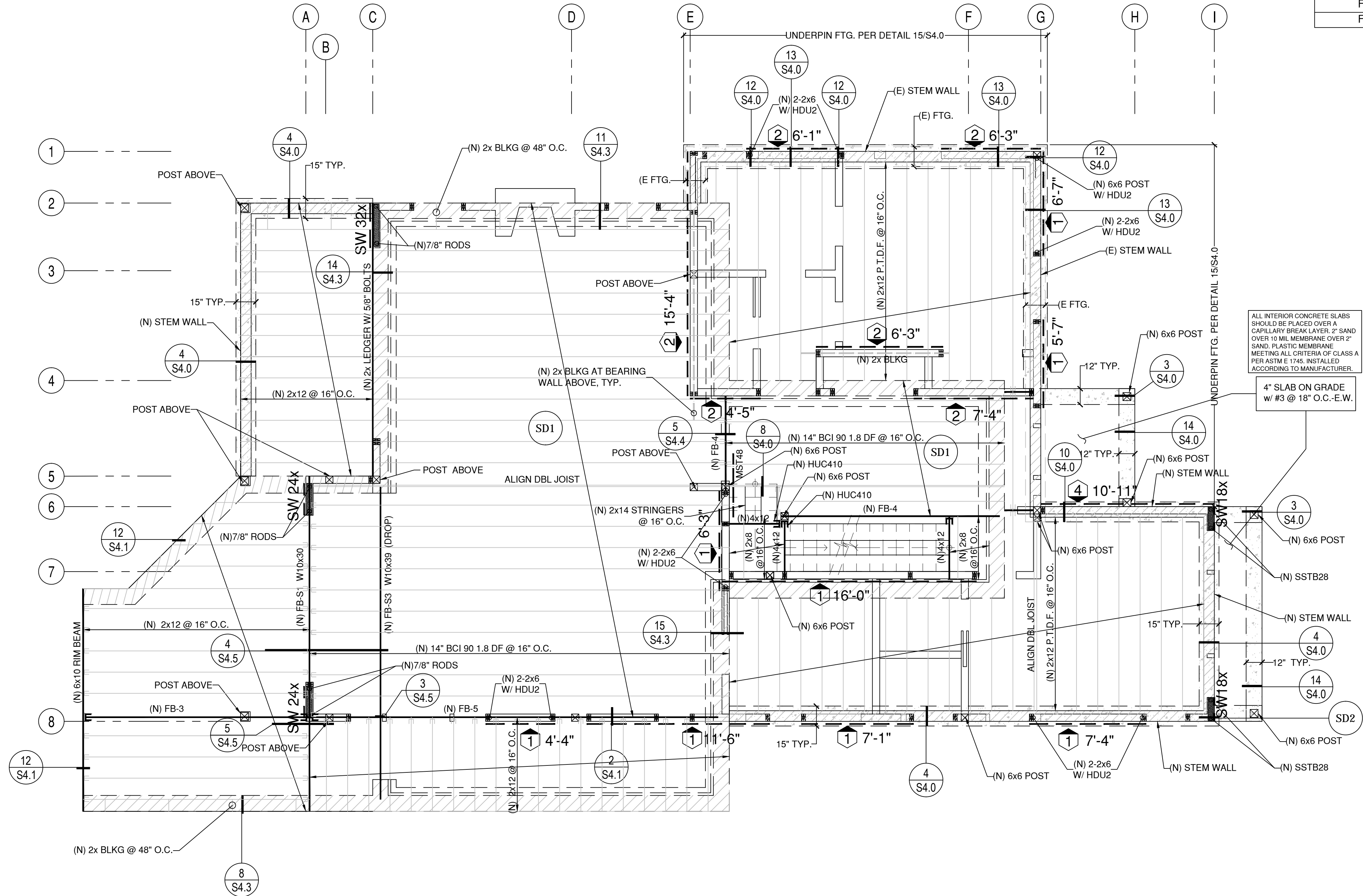
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LA JOLLA, CA 92037
PROJECT #: 2013-0070

S3.0

BEAM SCHEDULE	
RB-1	4x10
RB-2	4x12
CB-1	4x12
FB-1	4x12
FB-2	6x12
FB-3	5-1/8x12 GLB
FB-4	3-1/2x13.5 GLB
FB-5	5-1/8x13.5 GLB
FB-6	7x14 PSL
FB-7	7x16 PSL
FB-S1	W10x19
FB-S2	W10x30
FB-S3	W10x39
FB-S4	W16x57

LEGEND	
POST UP	
POST DOWN	
(N) BEARING WALLS BELOW WALLS ABOVE	
(E) BEARING WALLS BELOW NON BEARING WALLS BELOW	
FOOTING PER PLAN	
SHEAR WALL PER SHEAR PANEL SCHEDULE & 3/S2.1	
PERFORATED SHEAR WALL	
NEW HEADER PER DETAIL 6/S2.1	
THROUGH FLOOR HOLDOWN SEE DETAILS 3/S2.3 & 4/S2.3.	
DRAG STRAP PER DETAIL 9/S2.1.	
FRAMING PER PLAN	
CALIFORNIA FILL	
BLOCKED FLOOR DIAPHRAGM 4" BOUNDARY NAILING PER 5/S2.1	
TYPICAL	TYP.
SIMILAR	SIM.
HOLDOWN PER DETAIL 12/S2.0 & 12/S4.0	
PAD FOOTING PER PLAN SEE SCHEDULE FOR ADDITIONAL INFORMATION	

- NOTES:**
- HOLDOWNS MUST BE TIED INTO PLACE PRIOR TO CALLING FOR INSPECTION. FOR HOLDOWN DETAILS SEE 12/S2.0 & 12/S4.0.
 - THE SPECIAL INSPECTOR IS TO VERIFY THE DRILLING OF ANY HOLES, THE CLEANLINESS OF THE HOLE, THE MOISTURE IN THE HOLE, MIXING THE EPOXY, THE BRAND OF EPOXY AND THE PROPER MATERIAL FOR THE ASSEMBLY.
 - SEE SHEET S-1.1 FOR ADDITIONAL REQUIREMENTS FOR FOUNDATION SPECIAL INSPECTIONS.
 - SEE SHEET S-1.0 FOR ADDITIONAL FOUNDATION NOTES.
 - ARCHITECTS DIMENSIONS TAKE PRECEDENCE. CONTACT THE ENGINEER WITH DISCREPANCIES.
 - SEE SOILS REPORT FOR ADDITIONAL SUBGRADE, WATERPROOFING, AND DRAINAGE REQUIREMENTS.
 - SHEAR WALLS AT THIS LEVEL ARE ABOVE.

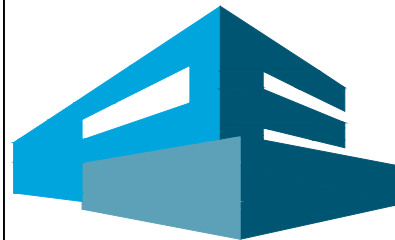


S3.1

FIRST FLOOR FOUNDATION PLAN

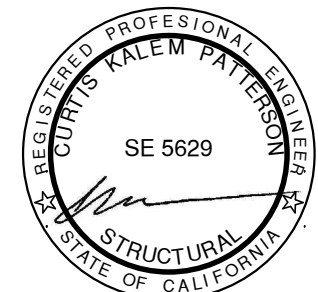
1/4" = 1'-0"

DO NOT SCALE PLANS FOR CONSTRUCTION DIMENSIONS. ALL CONSTRUCTION DIMENSIONS MUST BE VERIFIED WITH THE ARCH. PLANS.



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MEBUST RESIDENCE

5448 TAFT AVENUE
LA JOLLA, CA 92037
PROJECT #: 2013-0070

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DESIGNER:

REVISIONS

REV	REVISION	DATE

DRAWING STATUS

- ☐ INFORMATION DRAWINGS. (NOT FOR CONSTRUCTION)
- ☐ PRELIMINARY DRAWINGS. (NOT FOR CONSTRUCTION)
- ☐ CHECK SET. (NOT FOR CONSTRUCTION)
- ☐ PLAN CHECK DRAWINGS.
- ☐ CONSTRUCTION DOCUMENTS.
- ☐ OTHER AS-BUILT

CONTENT

FIRST FLOOR
FOUNDATION PLAN

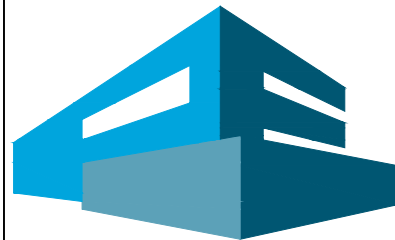
DRAWN BY: CR/CKP

CHECKED BY: CKP

DATE: SEPTEMBER 20, 2013

SHEET NO.

S3.1



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- ☐ CONSTRUCTION DOCUMENTS.
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AS-BUILT

CONTENT

SECOND FLOOR
ROOF / DECK
FRAMING PLAN

DRAWN BY: CR/CKP

CHECKED BY: CKP

DATE: SEPTEMBER 20, 2013

SHEET NO.

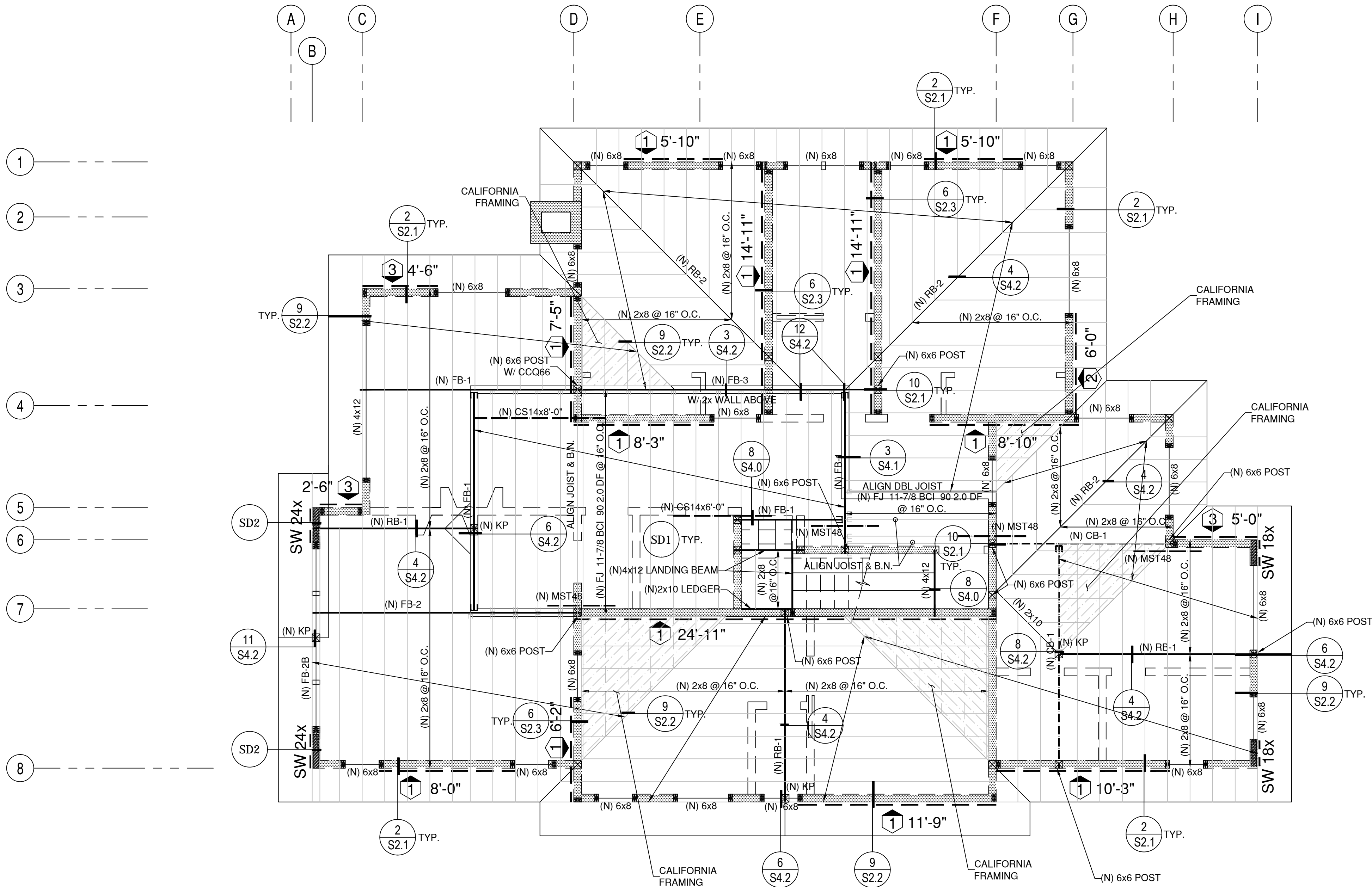
S3.3

BEAM SCHEDULE	
RB-1	4x10
RB-2	4x12
CB-1	4x12
FB-1	4x12
FB-2	6x12
FB-3	5-1/8x12 GLB
FB-4	3-1/2x13.5 GLB
FB-5	5-1/8x13.5 GLB
FB-6	7x14 PSL
FB-7	7x16 PSL
FB-S1	W10x19
FB-S2	W10x30
FB-S3	W10x30

LEGEND	
POST UP POST DOWN	
(N) BEARING WALLS BELOW WALLS ABOVE (E) BEARING WALLS BELOW NON BEARING WALLS BELOW	
FOOTING PER PLAN	
SHEAR WALL PER SHEAR PANEL SCHEDULE & 3/S2.1	
PERFORATED SHEAR WALL PER 1/S4.1	
NEW HEADER PER DETAIL 6/S2.1	
THROUGH FLOOR HOLDOWN SEE DETAILS 3/S2.3 & 4/S2.3.	
DRAG STRAP PER DETAIL 9/S2.1.	
FRAMING PER PLAN	
CALIFORNIA FILL PER DETAIL 10/S2.2	
BLOCKED FLOOR DIAPHRAGM 4" BOUNDARY NAILING PER 5/S2.1	
TYPICAL	TYP.
SIMILAR	SIM.
HOLDOWN PER DETAIL 12/S2.0 & 12/S4.0	
PAD FOOTING PER PLAN SEE SCHEDULE FOR ADDITIONAL INFORMATION	

GENERAL NOTES.

- 1.- ALL LOAD BEARING CEILING BEAMS & FLOOR BEAMS SHALL HAVE A MINIMUM OF 4x4 DF#1 POSTS WITH SIMPSON FC4 @ TOP & BOTTOM PLATES TO POST CONNECTIONS. SIZE OF POST IS DETERMINED BY THE WIDTH OF THE BEAM BEING SUPPORTED. (UNLESS OTHERWISE NOTED IN PLAN OR DETAIL) ALL ISOLATED POST SHALL HAVE SIMPSON CC OR ECC & CB CONNECTORS.
- 2.- DIAPHRAGM SHEATHING NAILS OR OTHER APPROVED SHEATHING CONNECTORS SHALL BE DRIVEN SO THAT THEIR HEAD OR CROWN IS FLUSH WITH THE SURFACE OF THE SHEATHING.
- 3.- BOTH SIDES OF EACH SHEAR WALL SHALL HAVE A MINIMUM OF A VERTICAL MST48 INSTALLED AT THE FLOOR LEVEL, U.O.
- 4.- LAP SPLICE PLATES A MIN OF 48" W/ 2 ROWS 9-16d @ 2-1/2" O.C. OR USE MST48 W/16d. SEE DETAIL 4/S2.1.
- 5.- FASTEN COILED STRAPS A MINIMUM OF 3'-0" TO ADJOINING SHEAR WALLS
- 6.- FOR BLOCKED DIAPHRAGMS WITH NAILING LESS THAN 4": FRAMING AT ADJOINING PANEL EDGES SHALL BE 3" NOMINAL OR WIDER AND NAILS SHALL BE STAGGERED.
- 7.- REFER TO DETAIL 2/S2.2 FOR CEILING JOIST SCHEDULE.
- 8.- REFER TO DETAIL 1/S2.2 FOR FRAMING HANGER SCHEDULE.

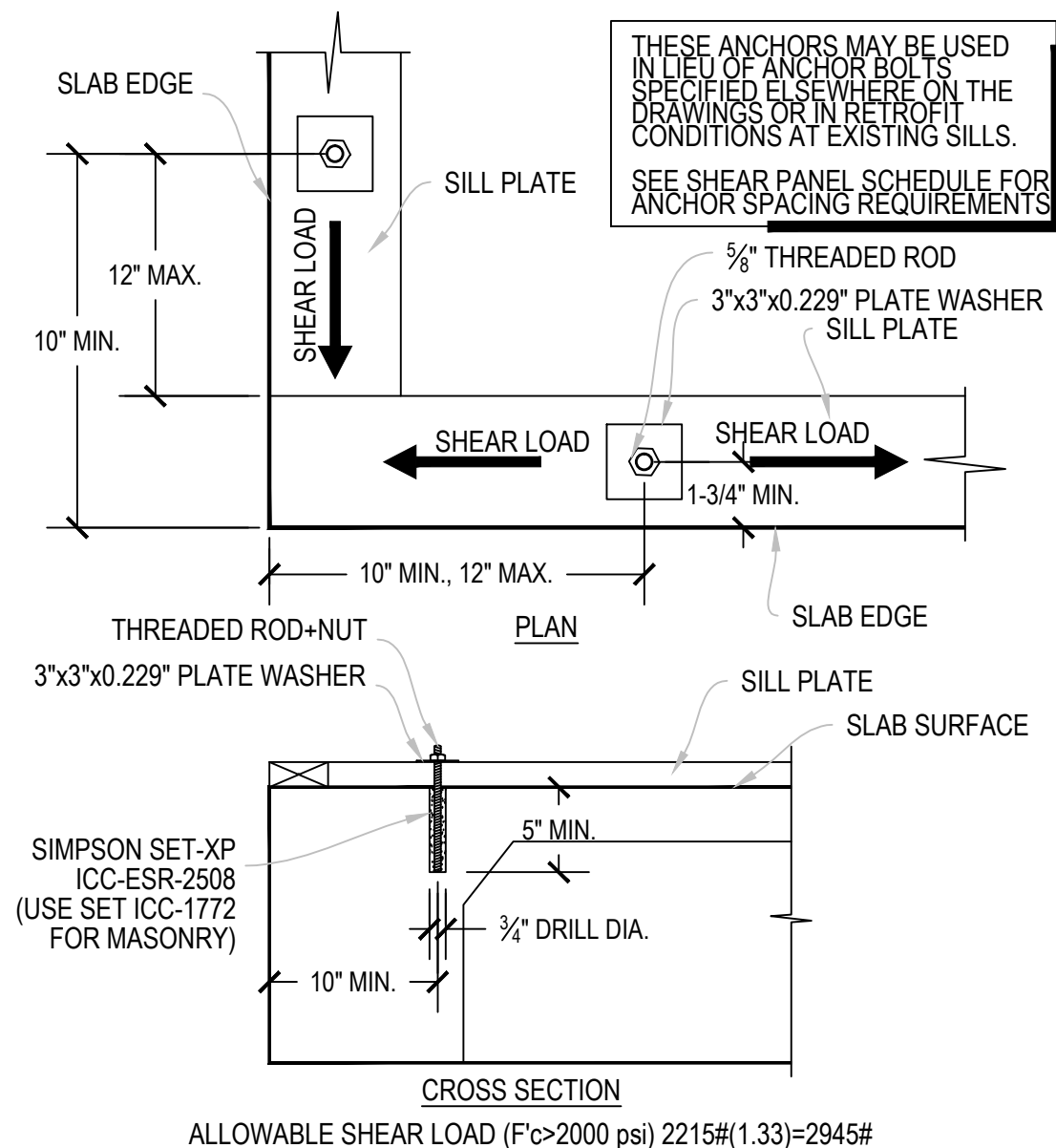


S3.3

SECOND FLOOR ROOF DECK FRAMING PLAN

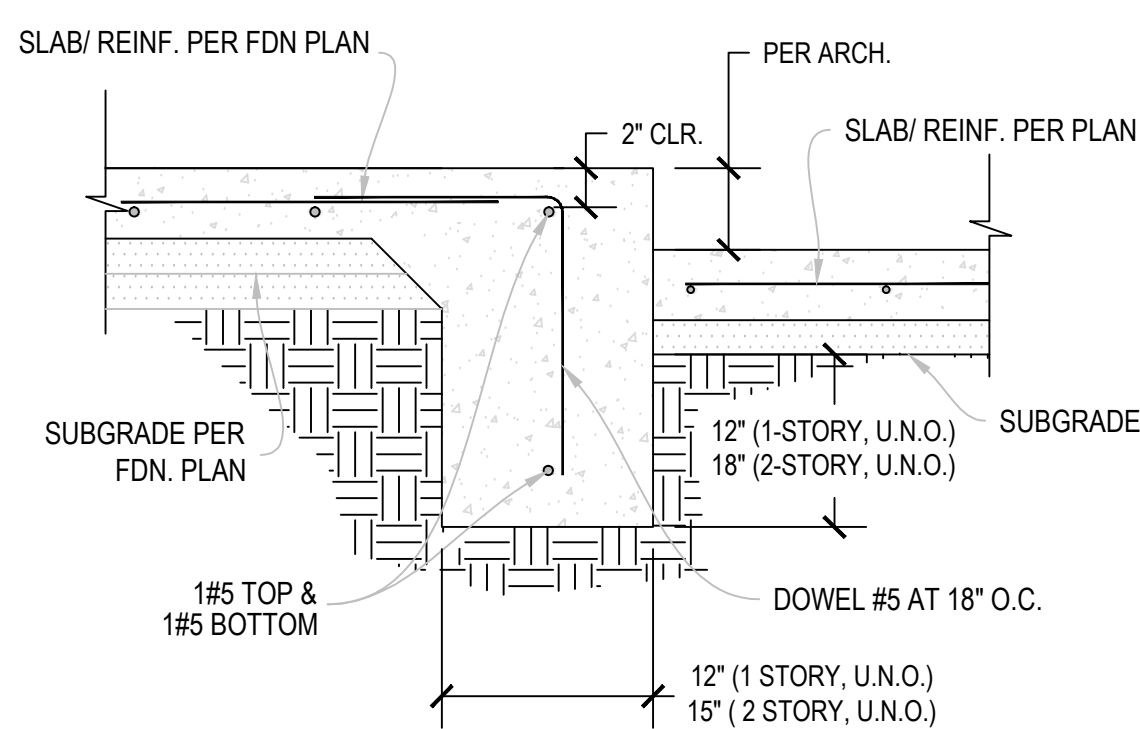
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DIMENSIONS. ALL CONSTRUCTION DIMENSIONS
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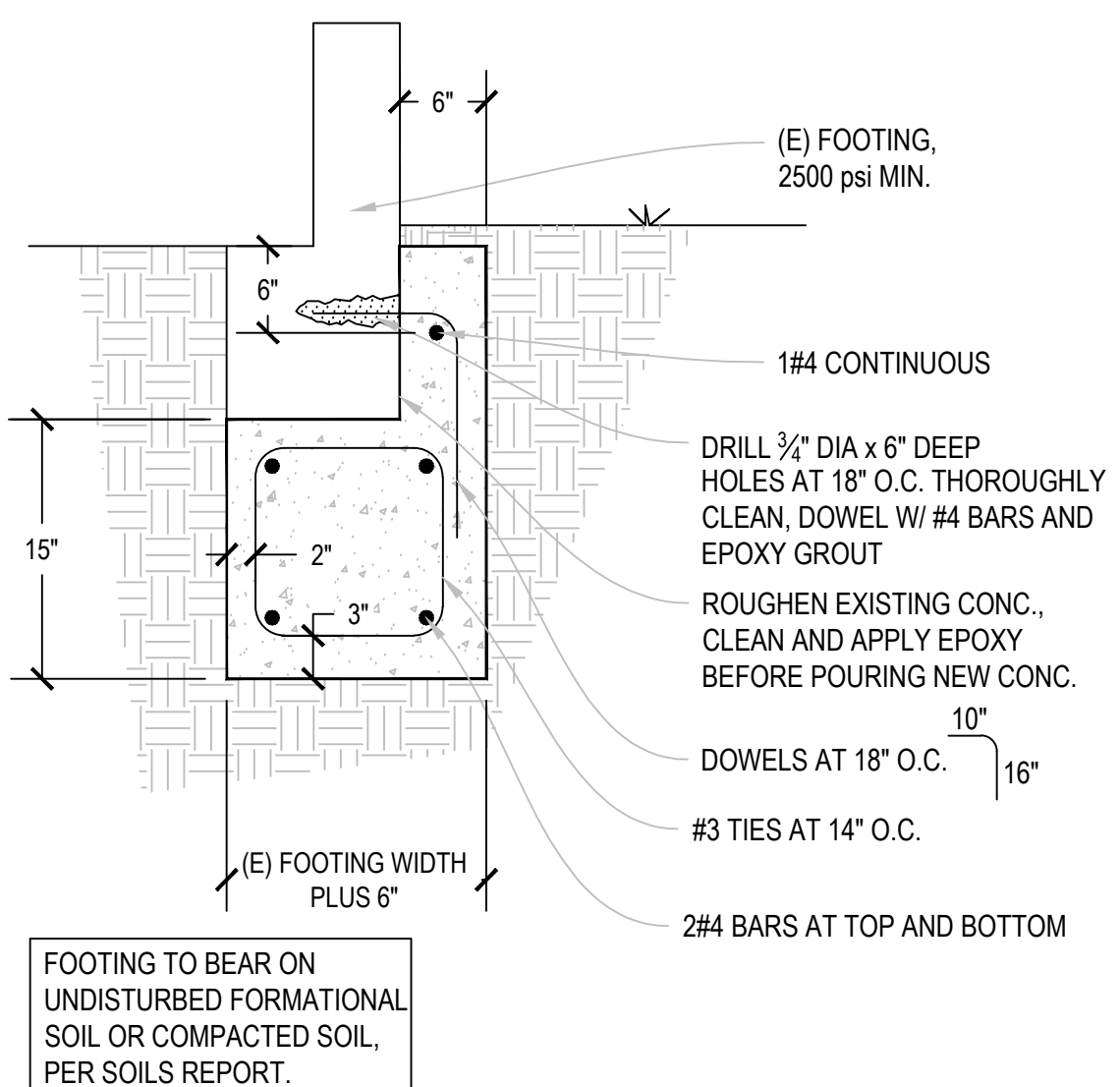
THREAD ROD RETROFIT

13



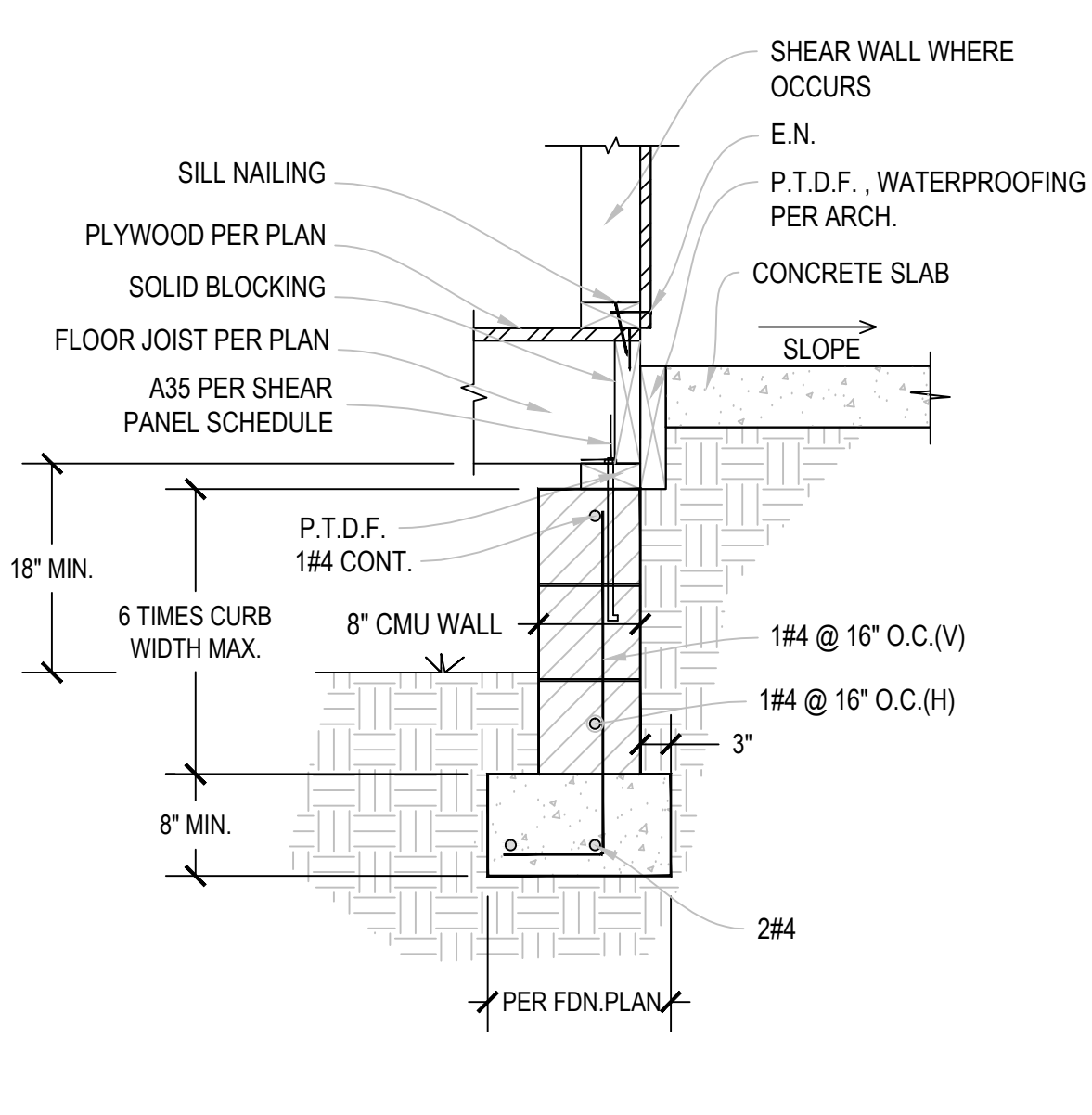
EXTERIOR FOOTING DETAIL

14



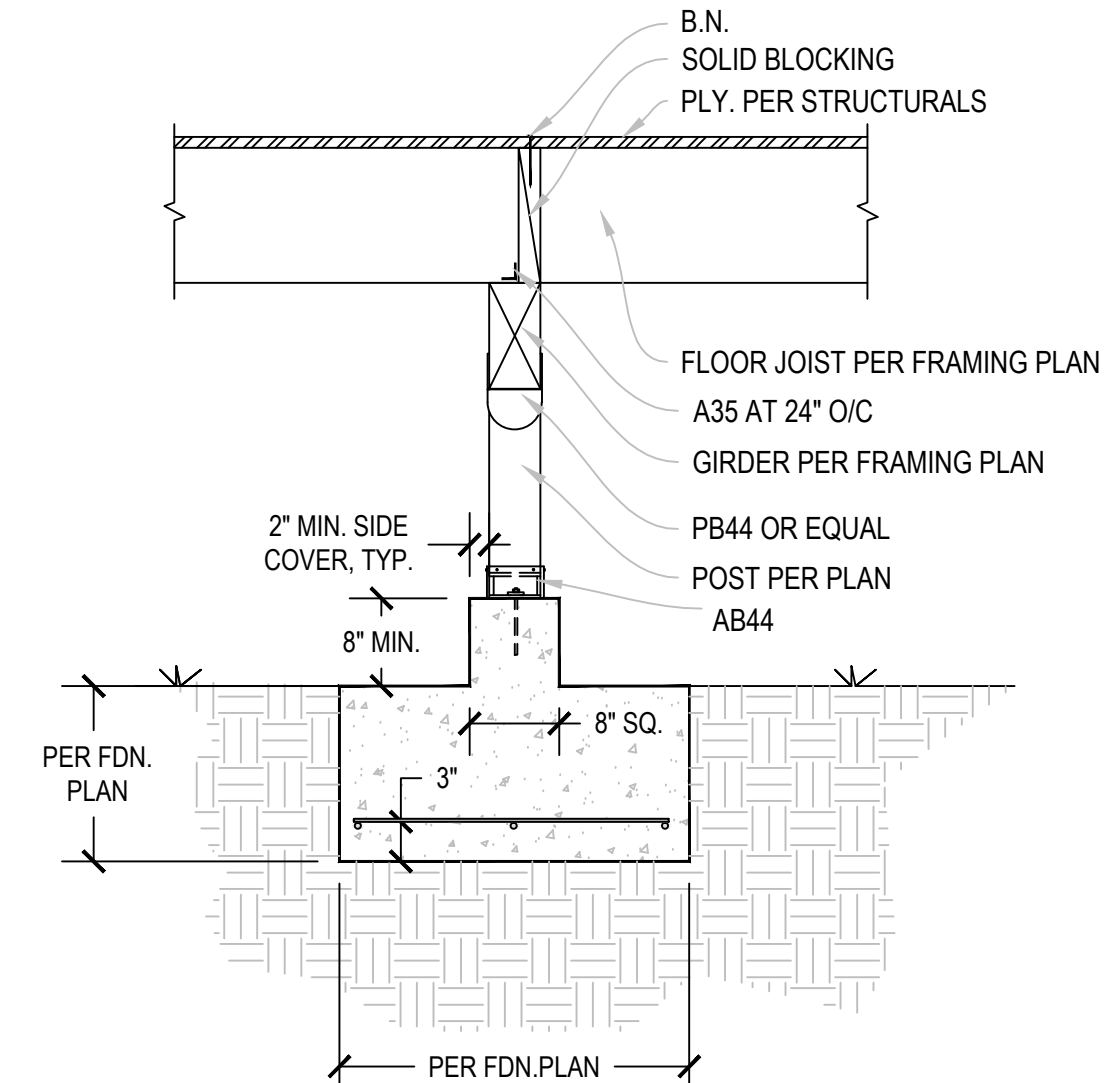
UNDERPINNED FOOTING

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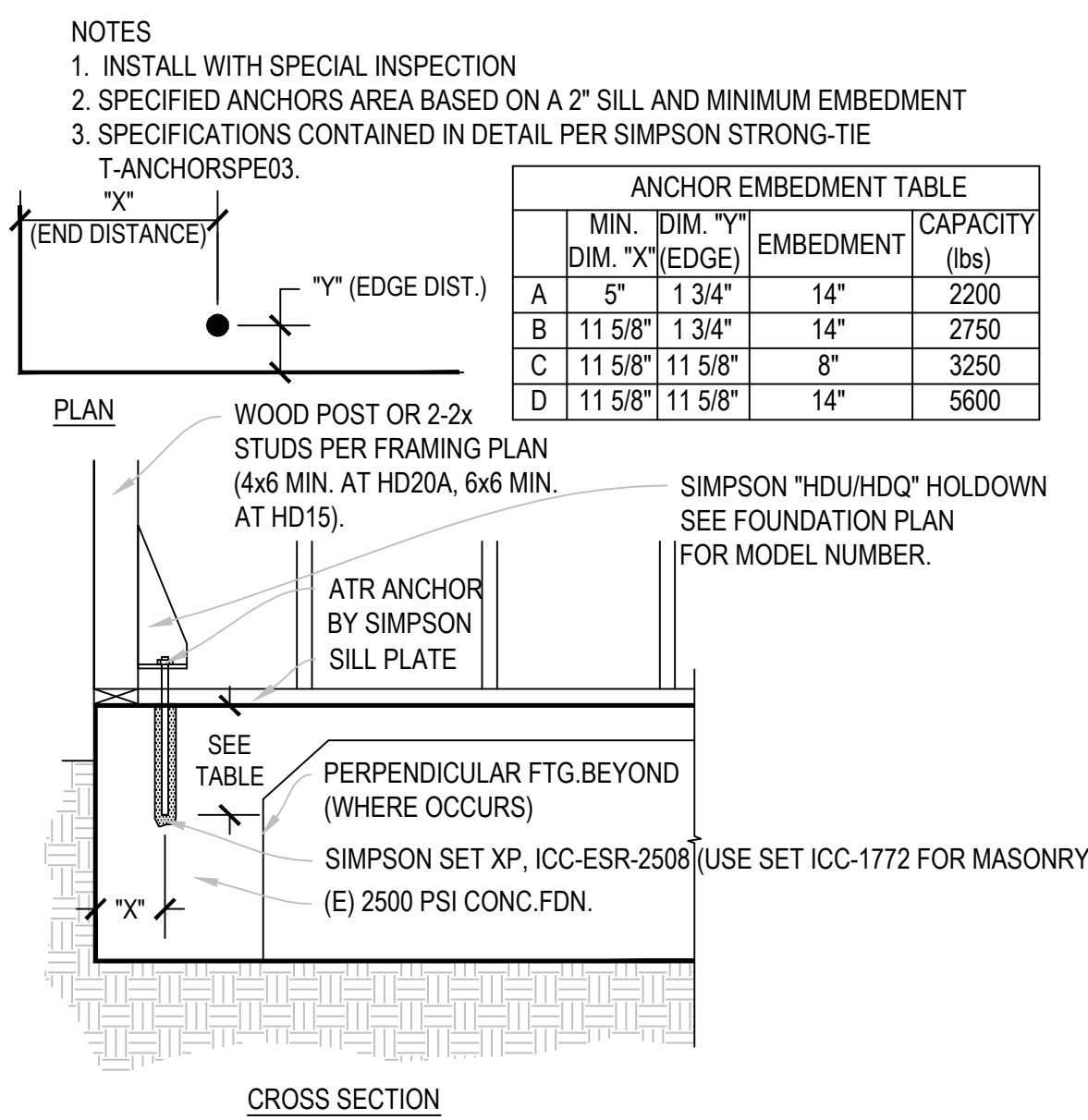
SLAB/RAISED FLOOR TRANSITION

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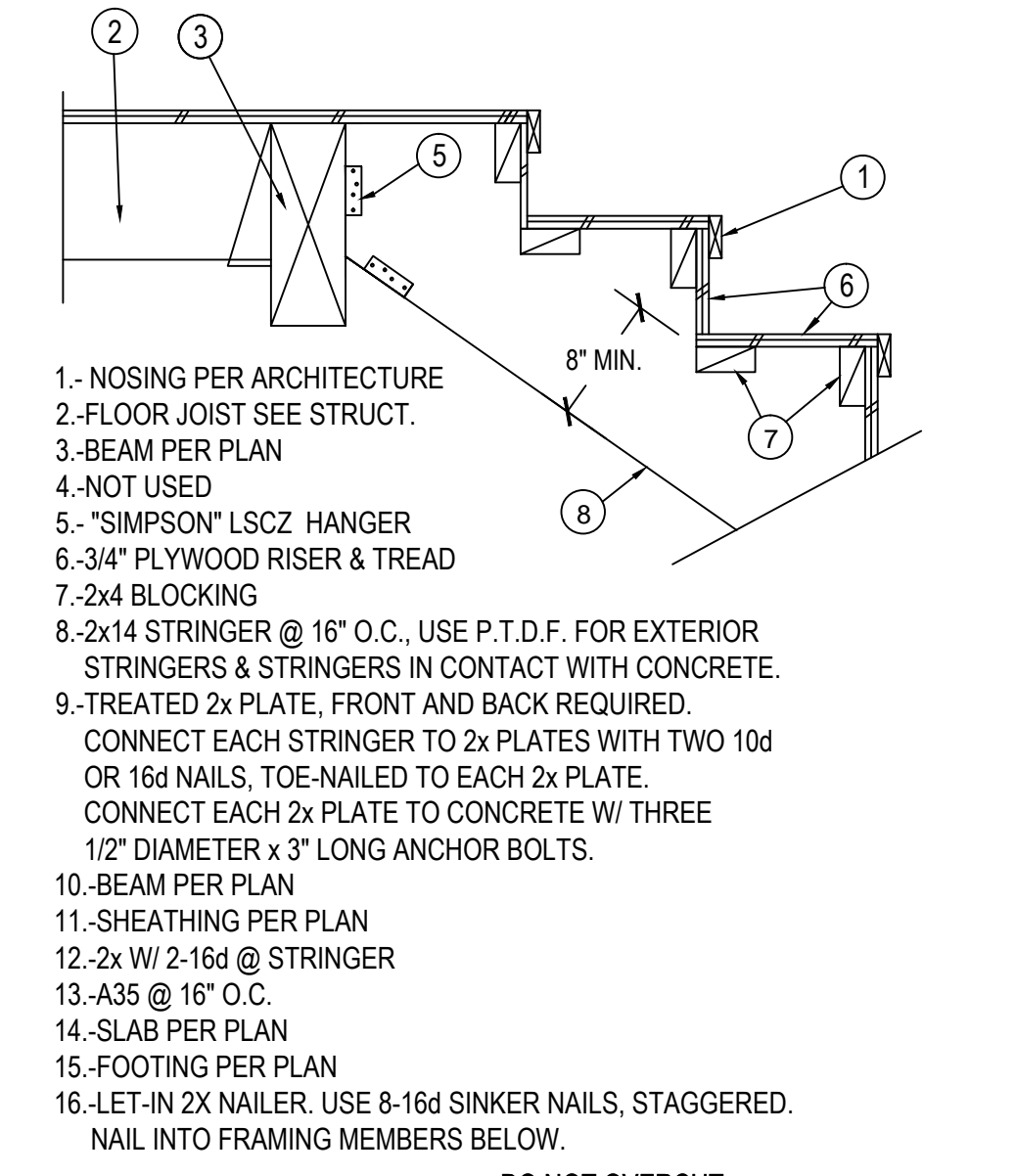
FOOTING INTERIOR PIER

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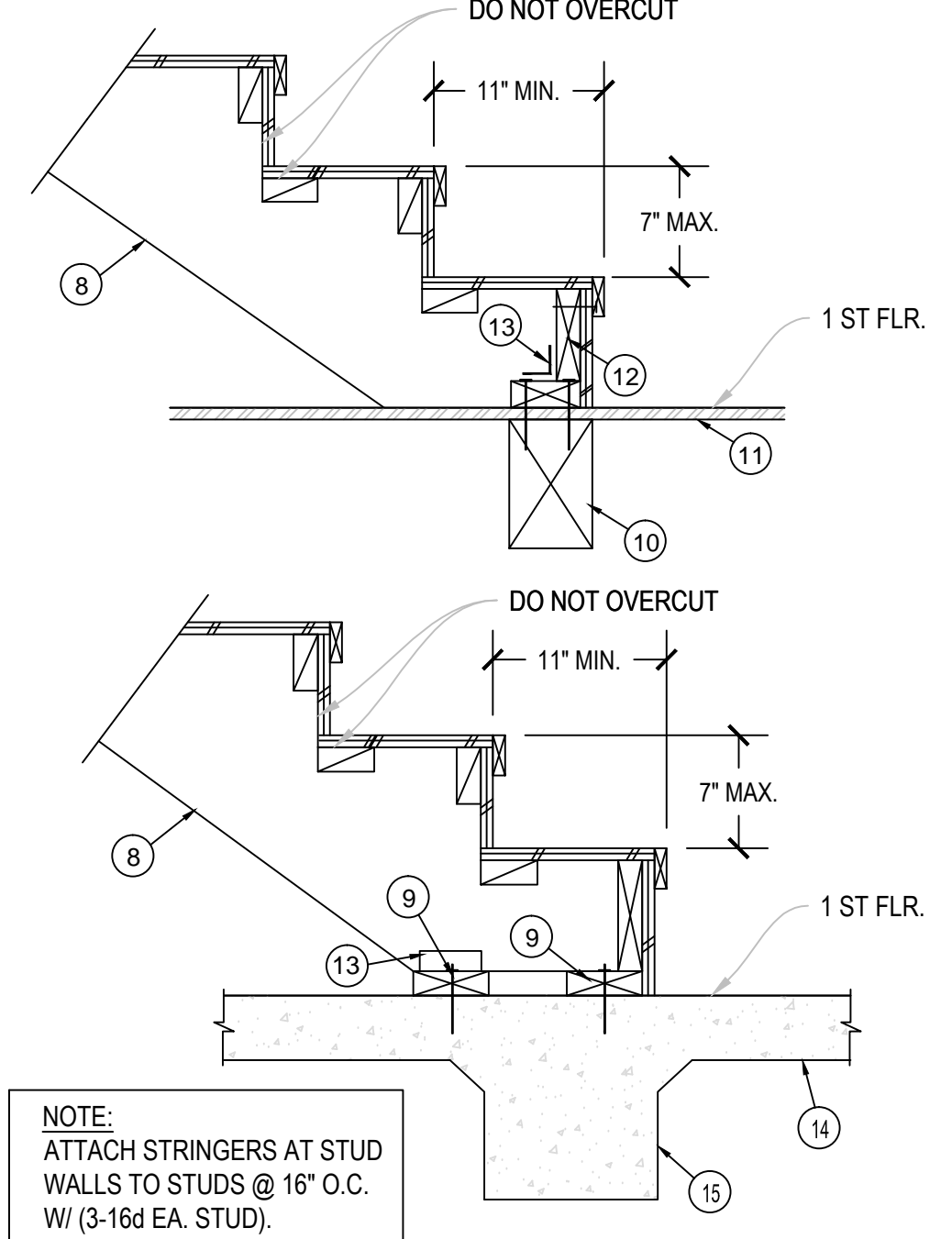
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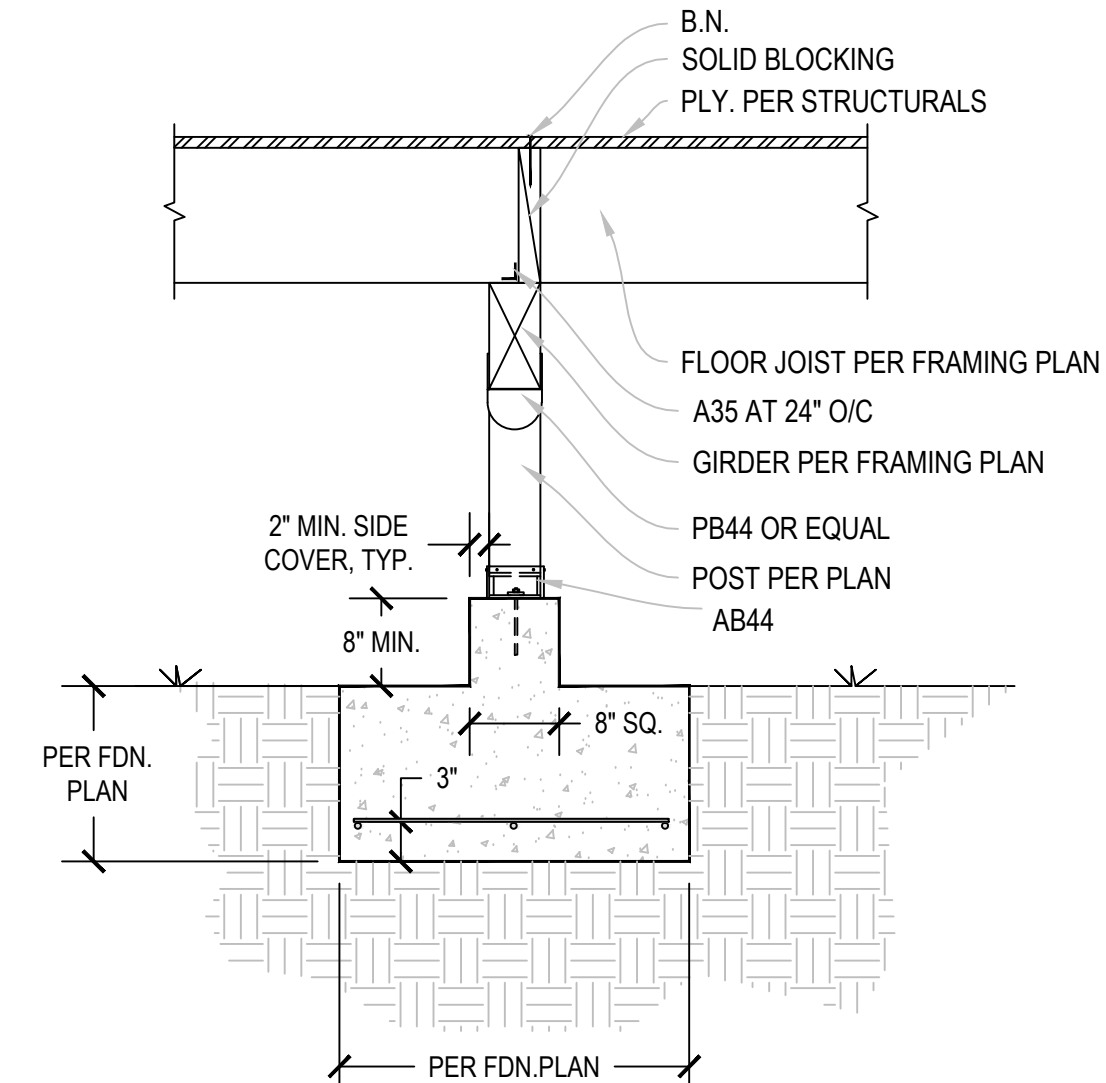
STAIR DETAIL

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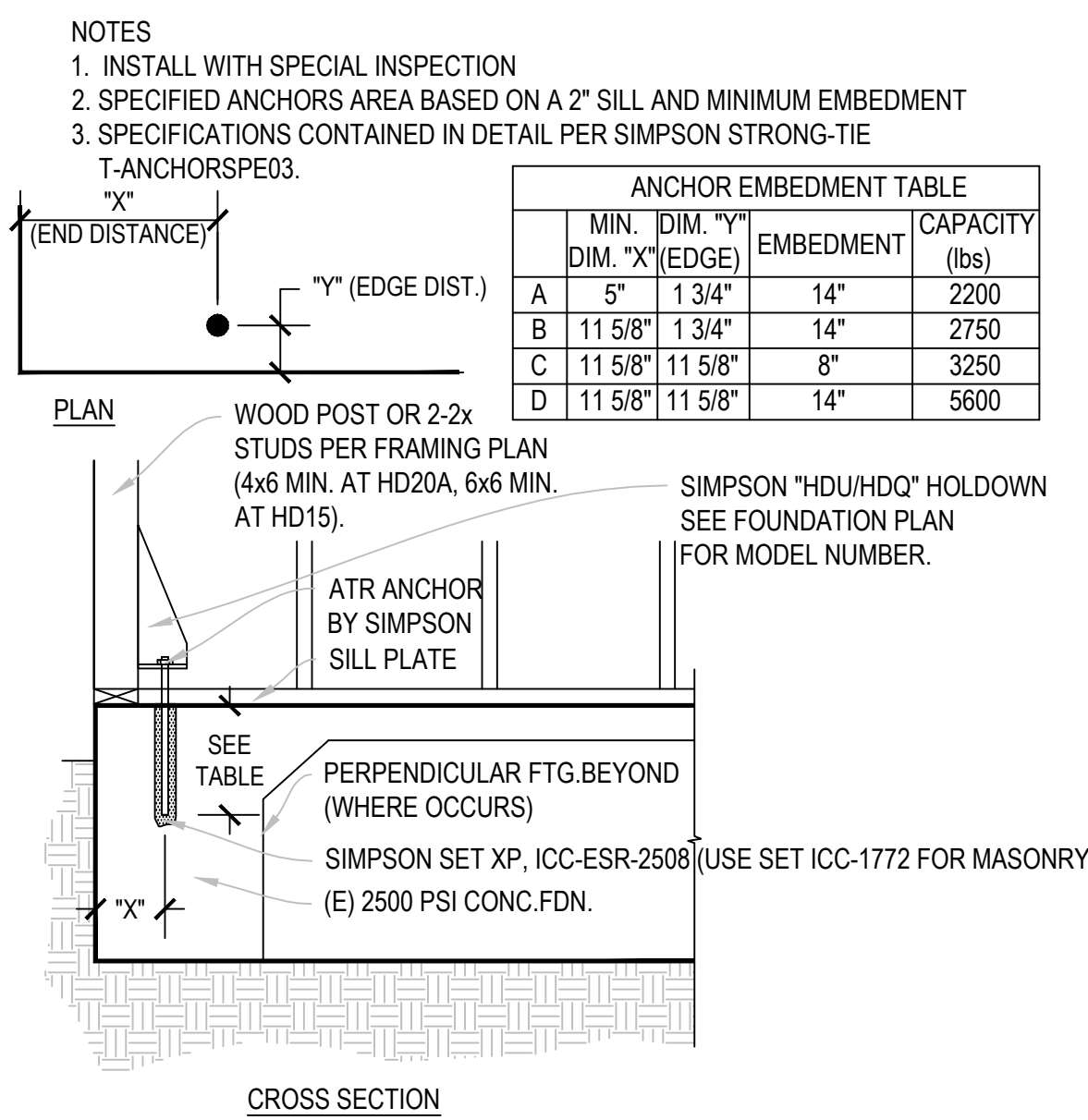
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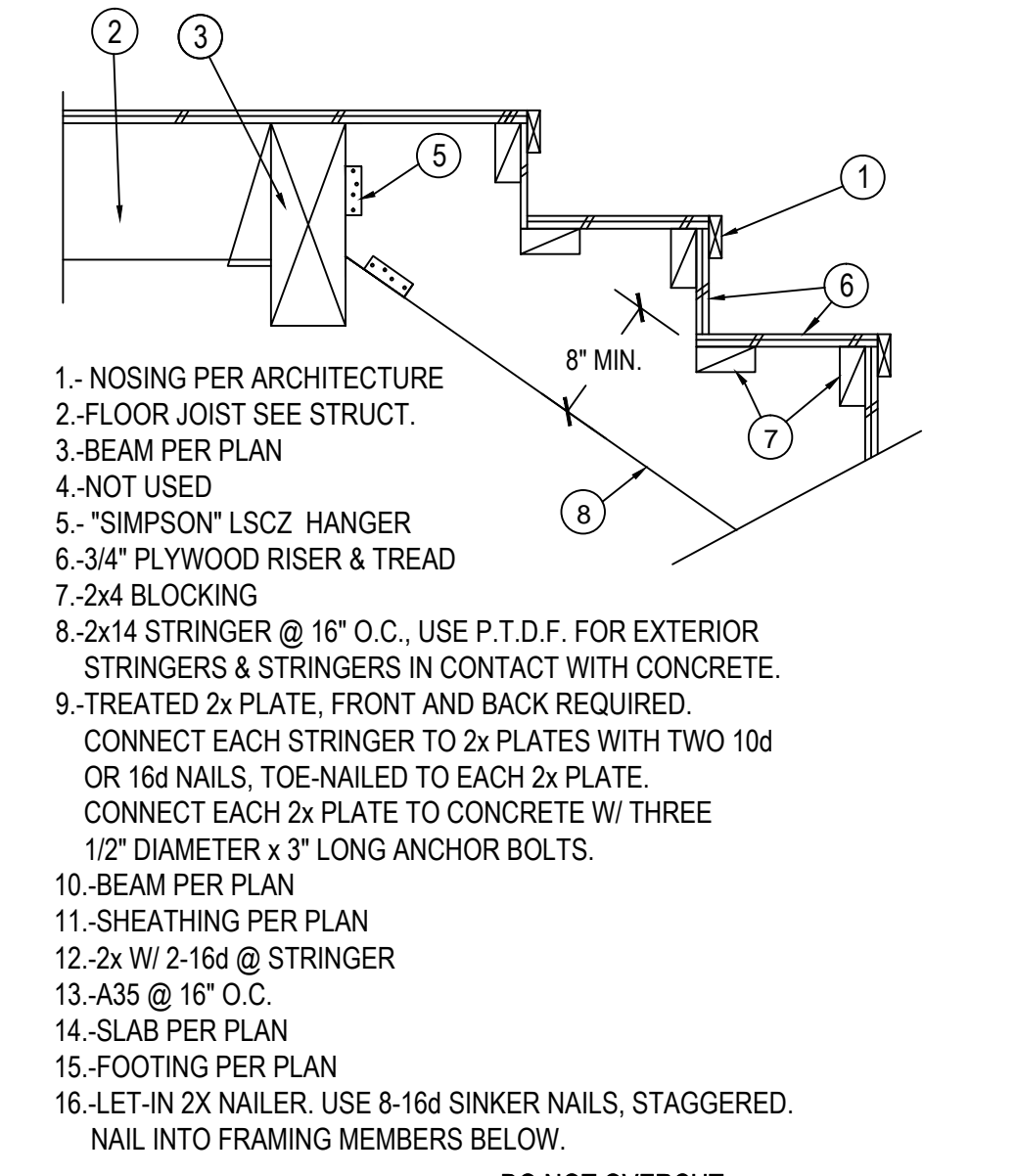
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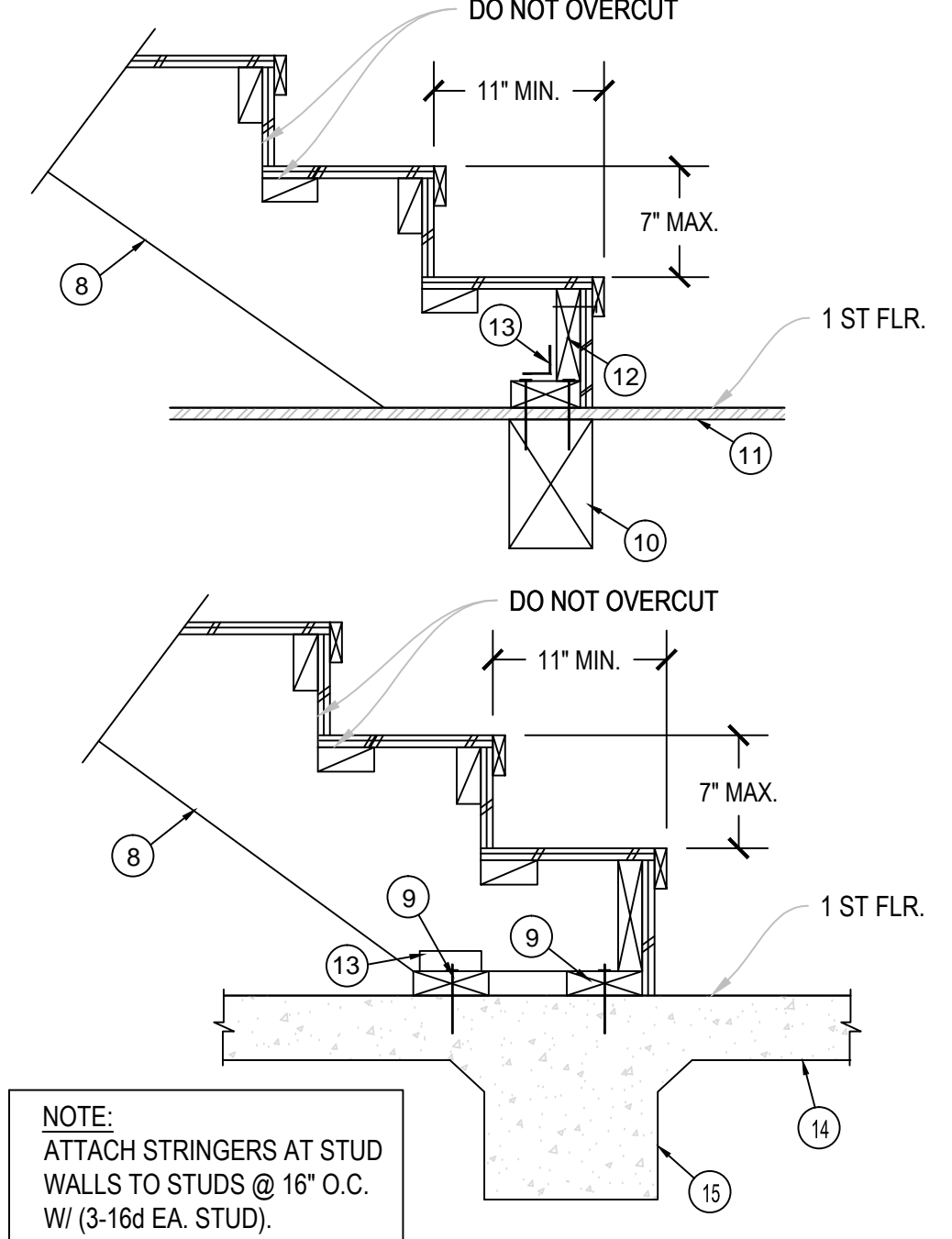
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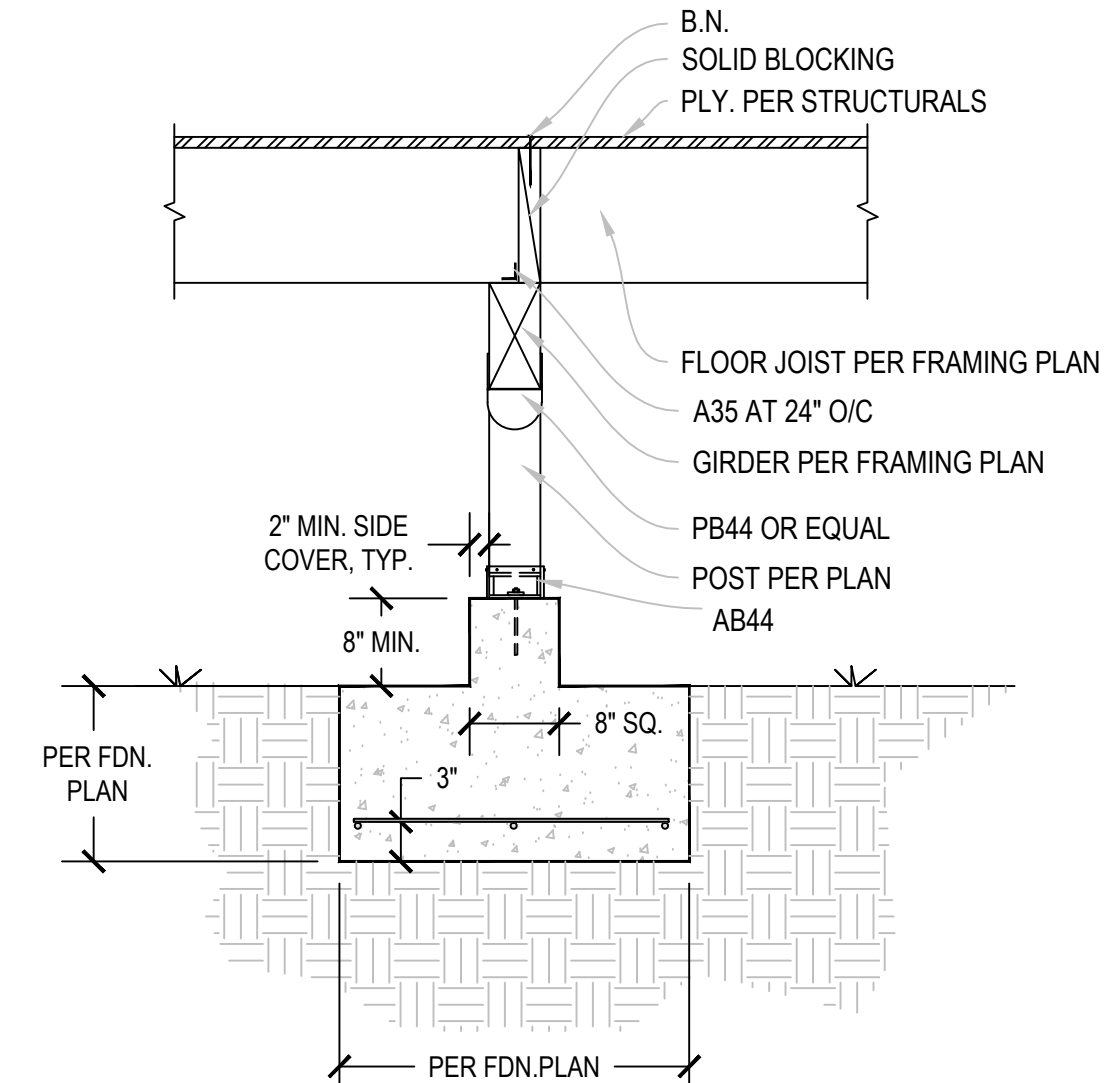
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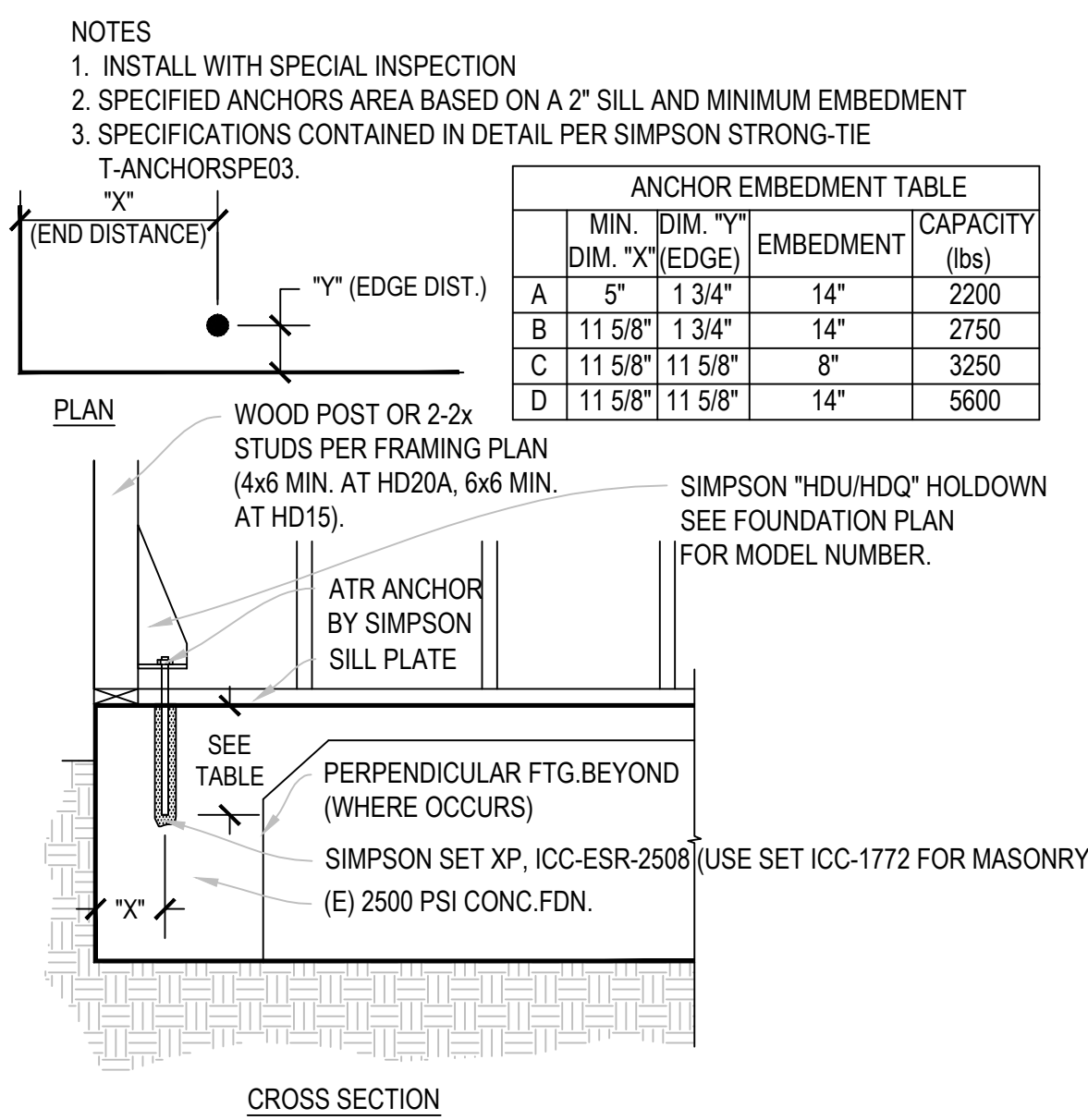
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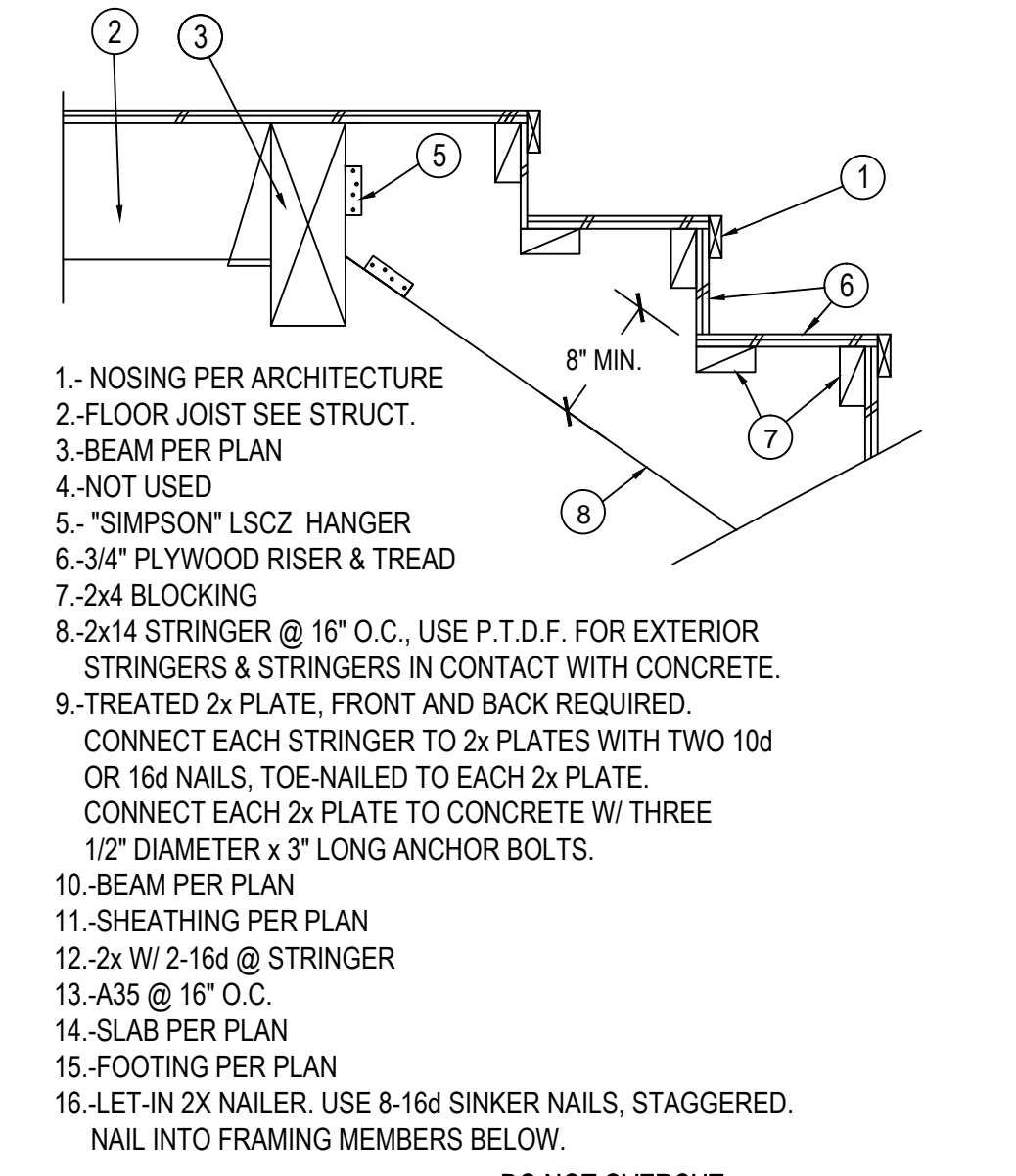
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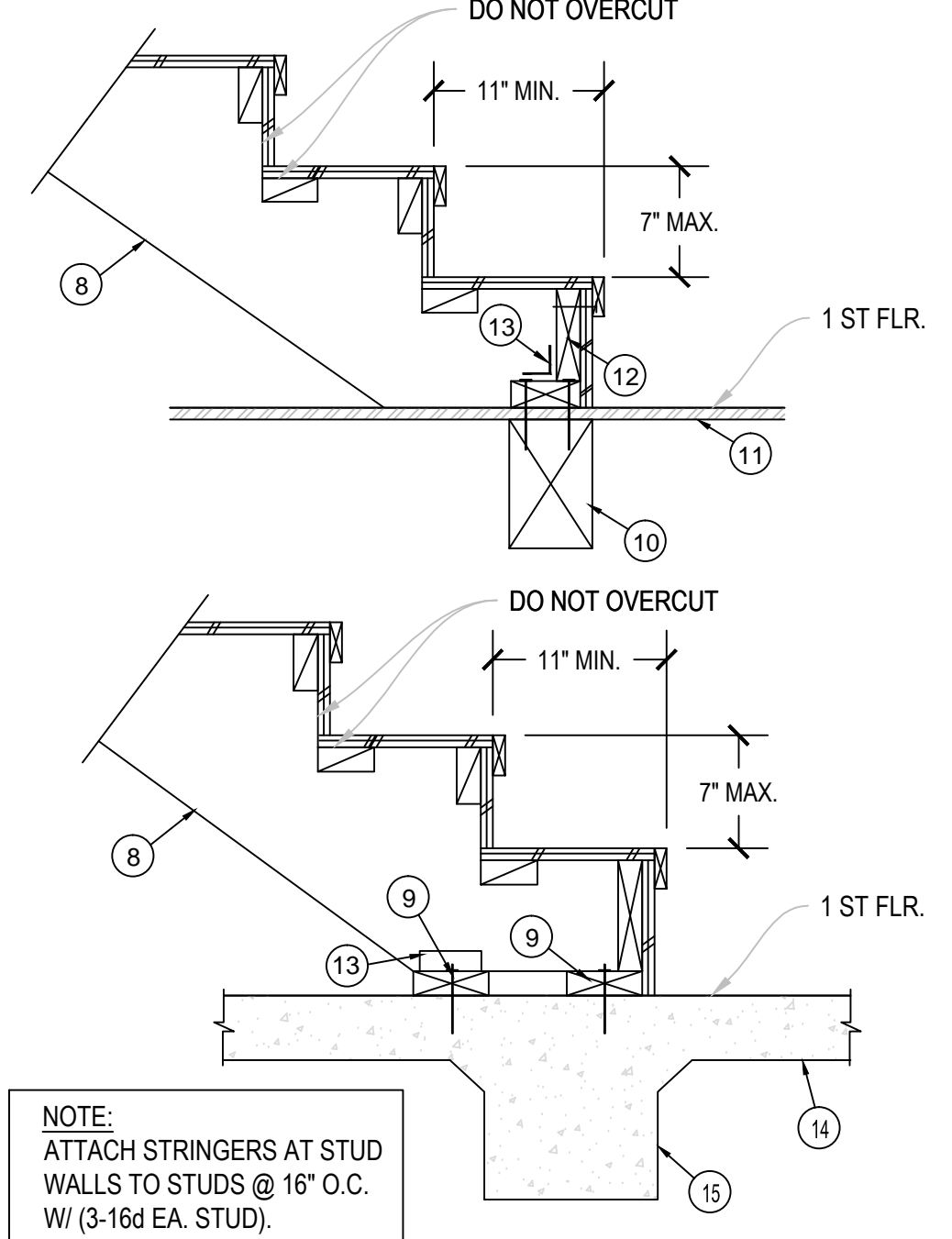
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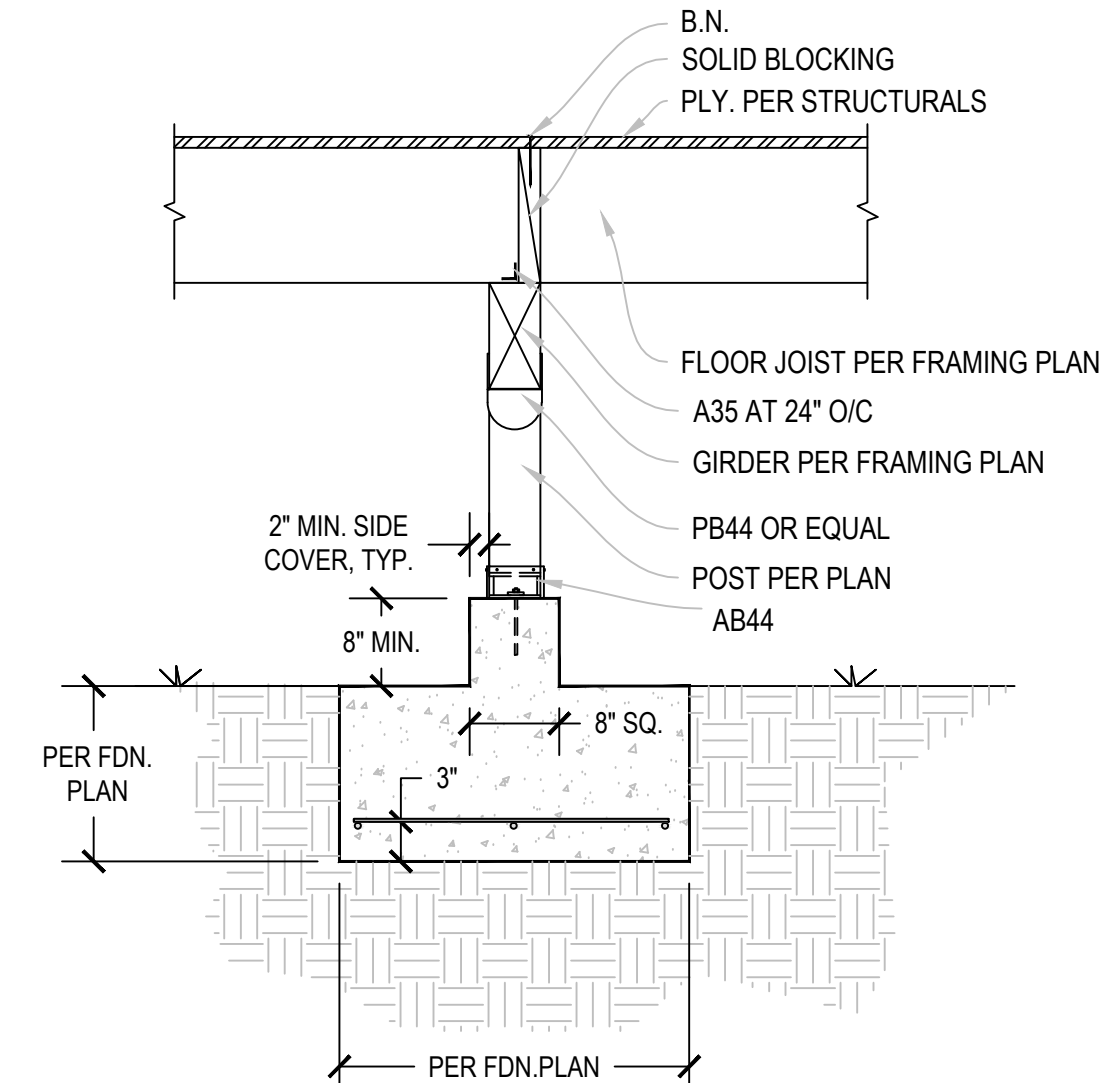
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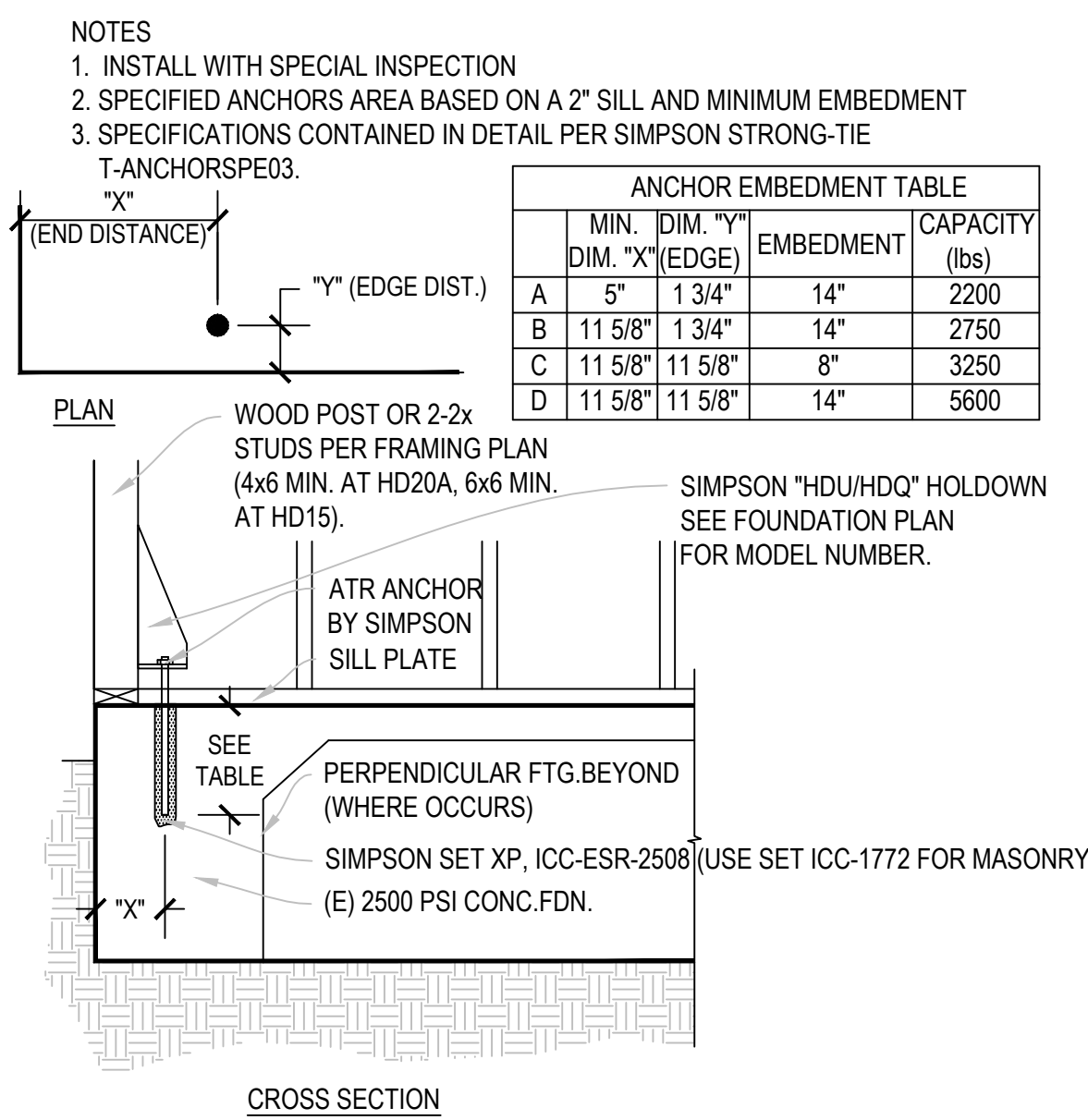
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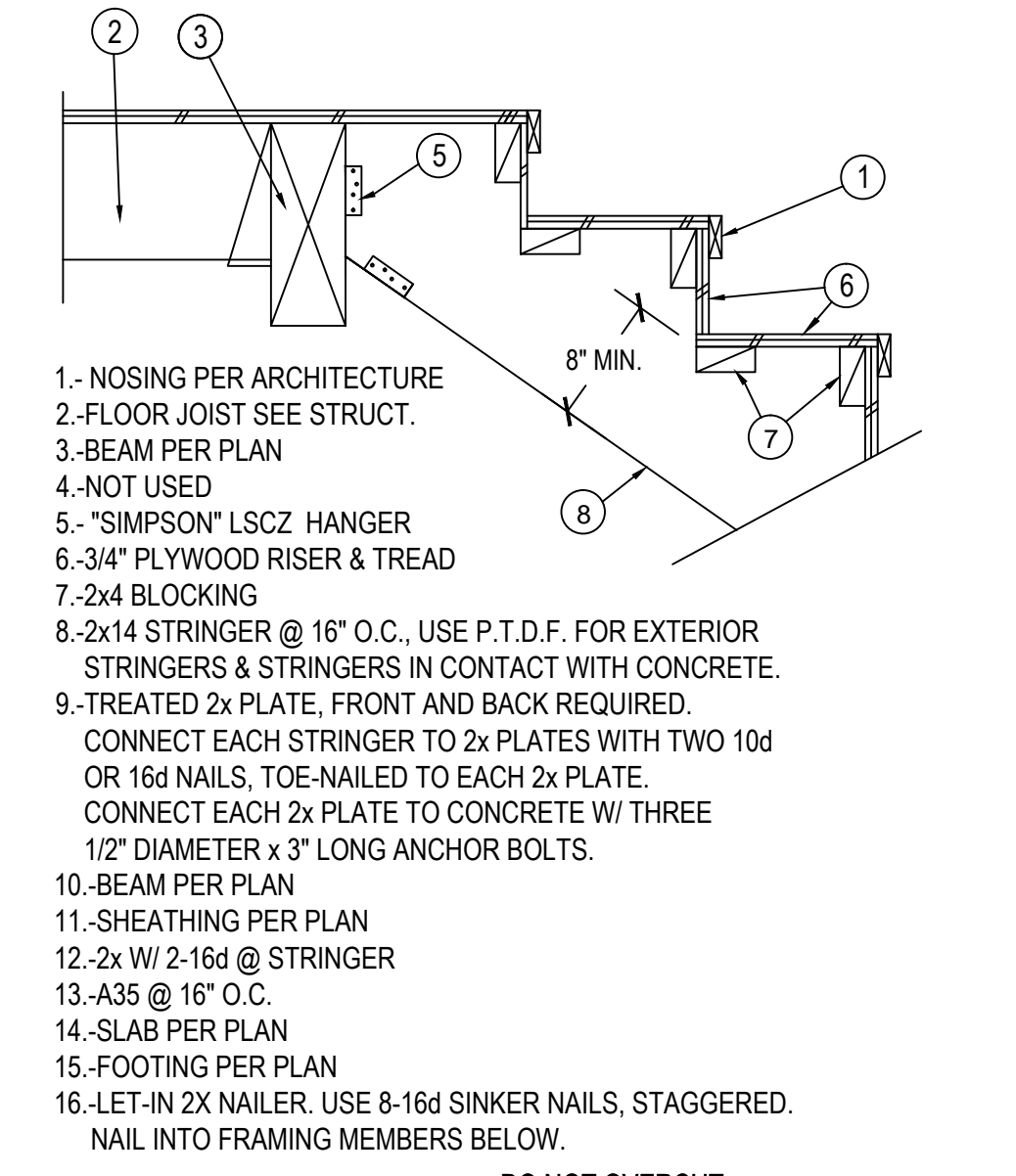
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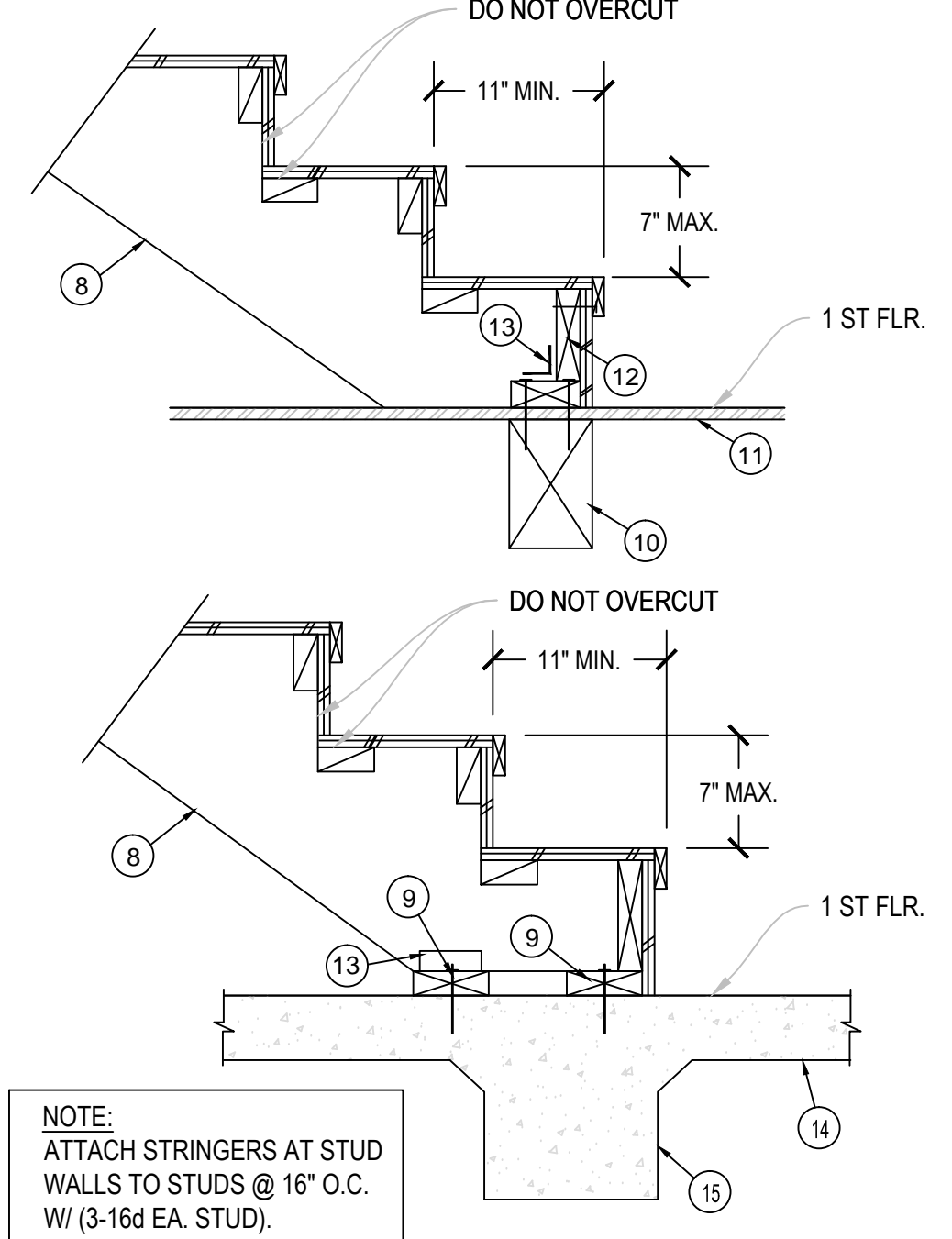
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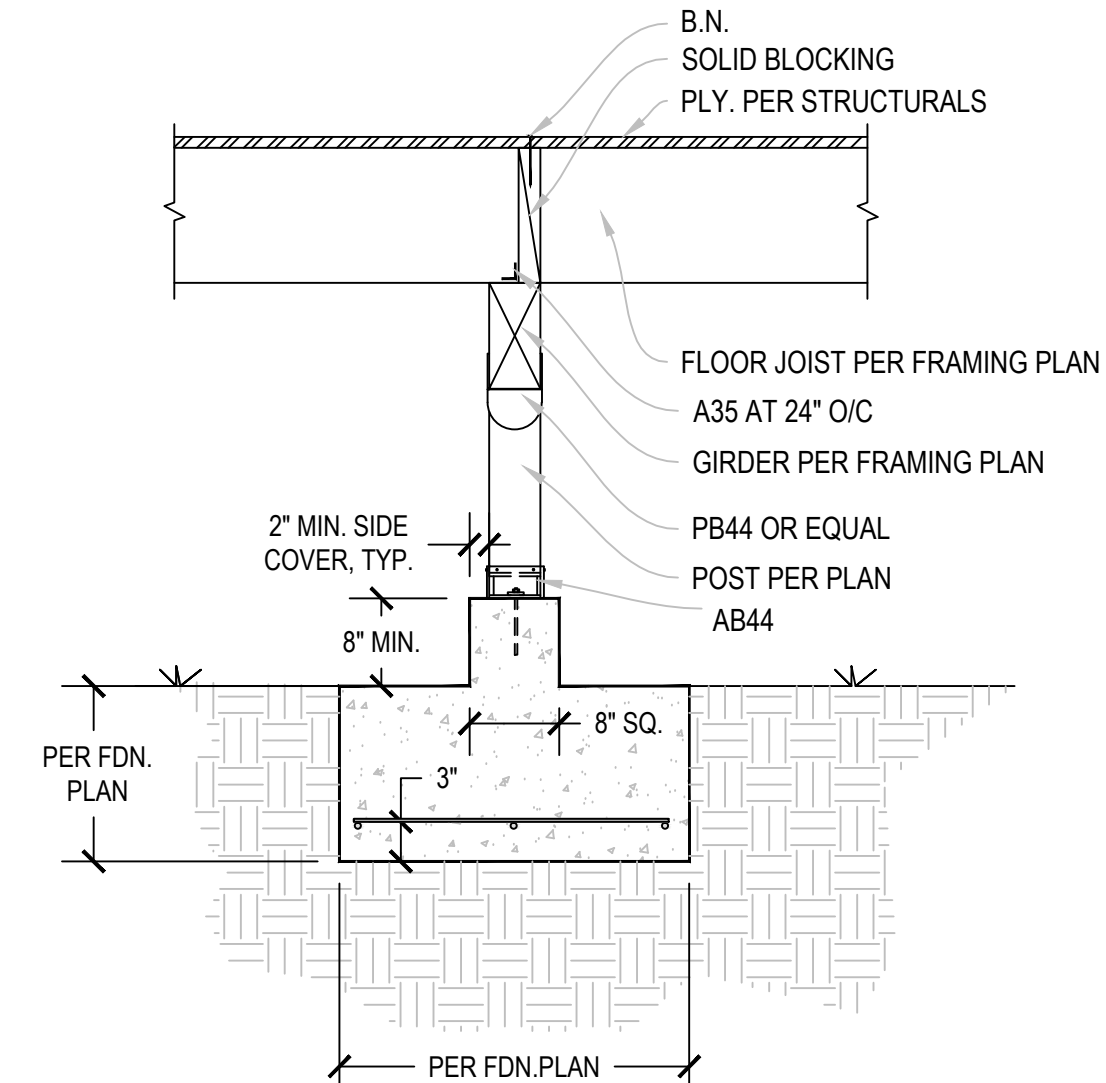
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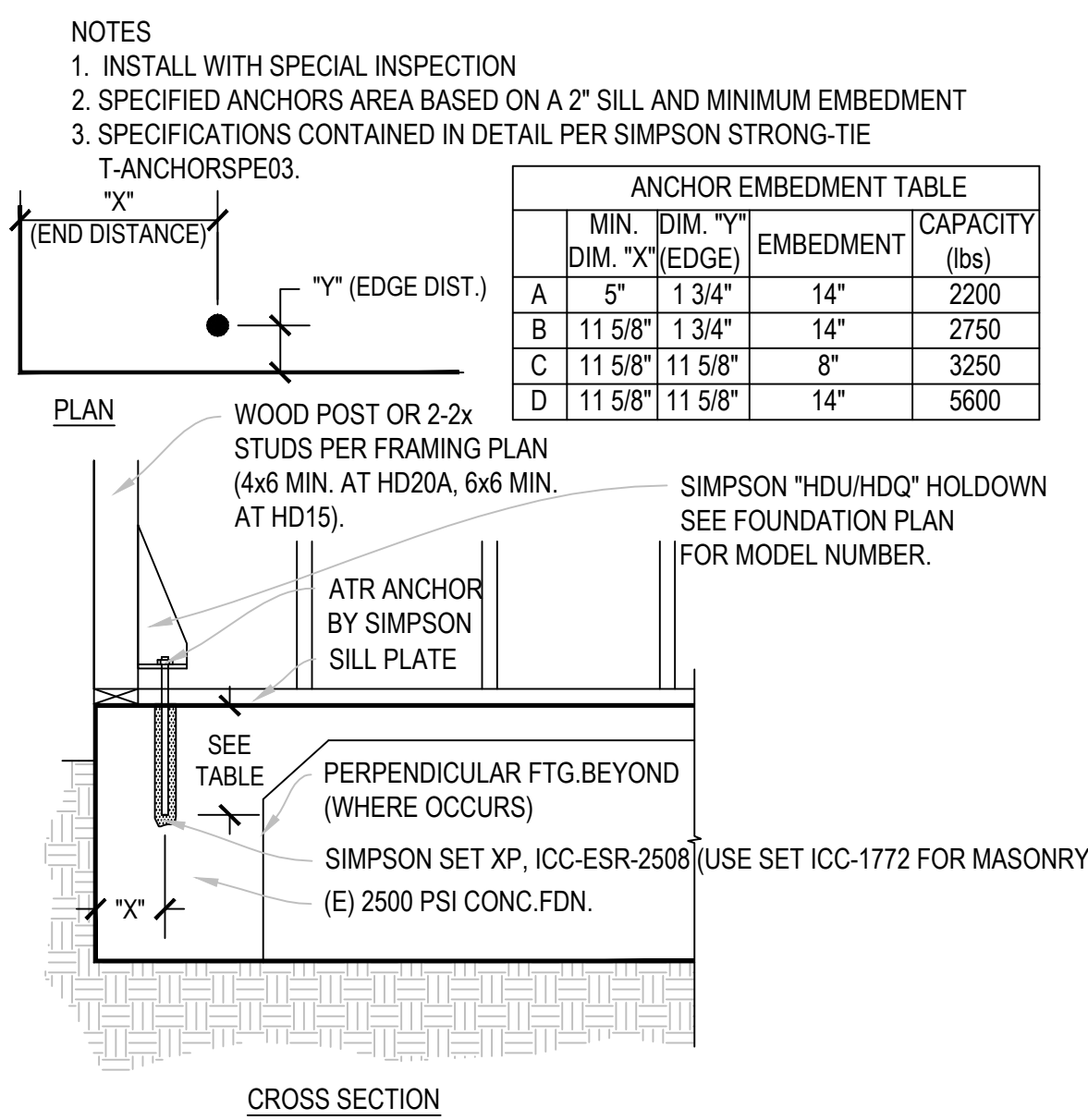
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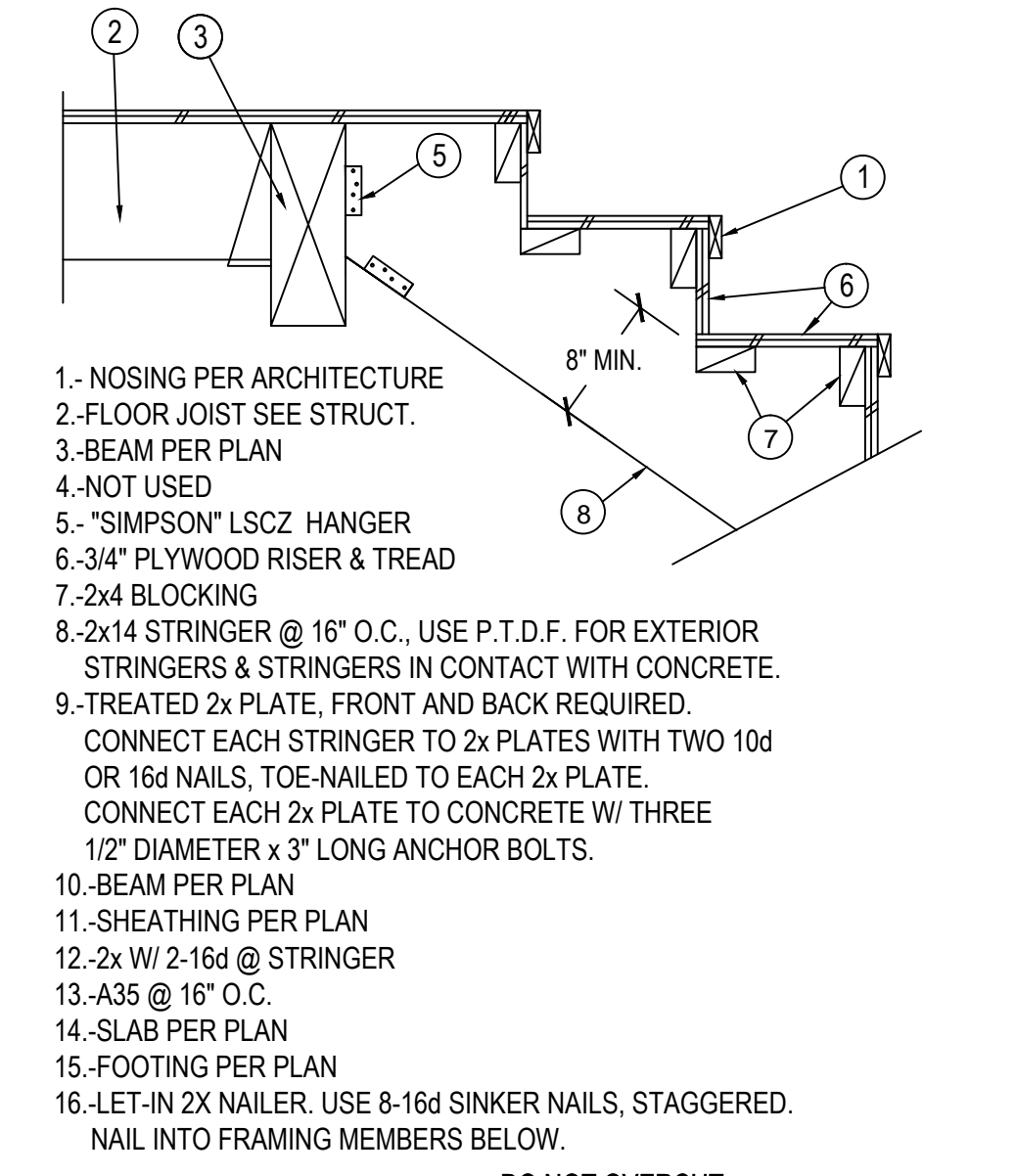
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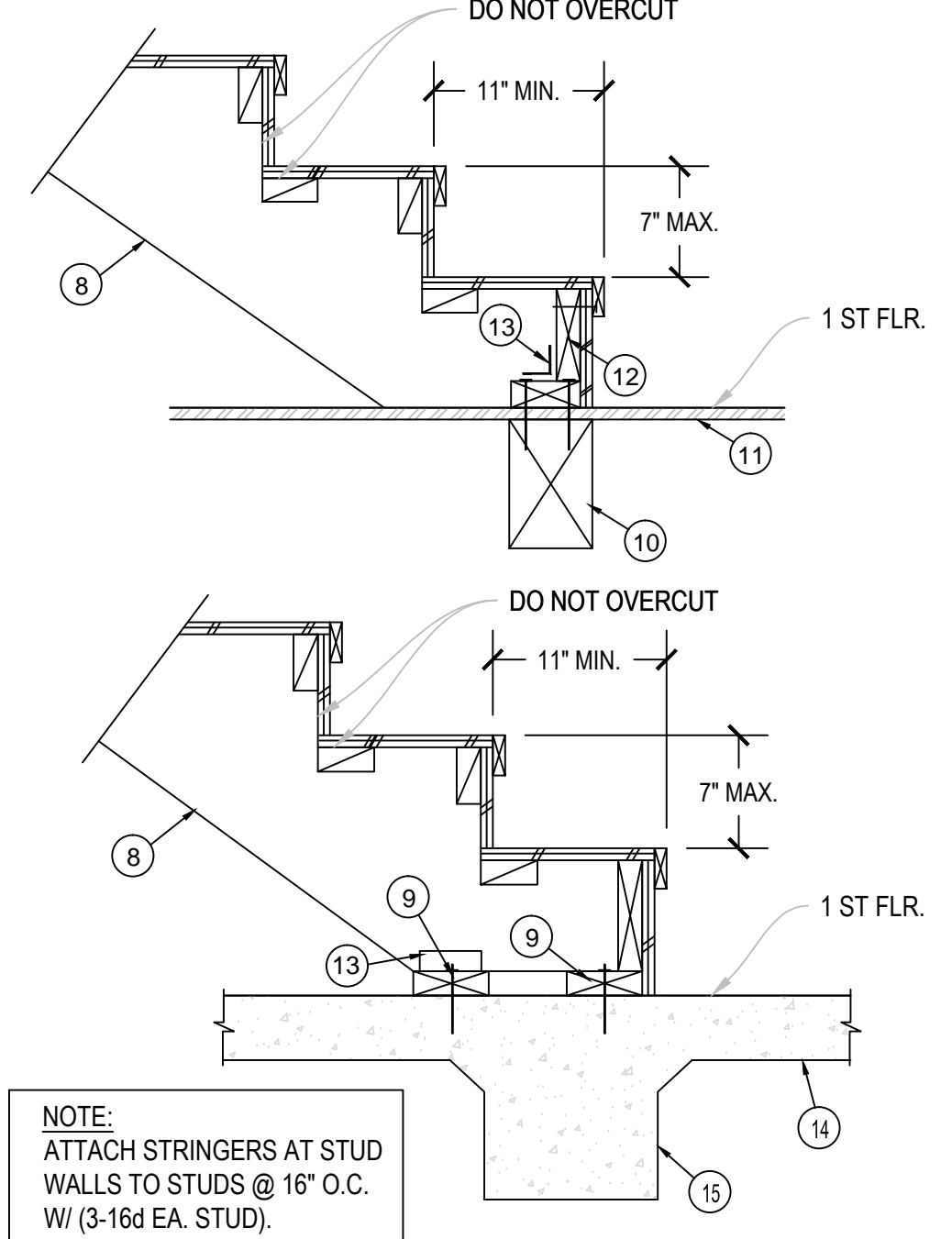
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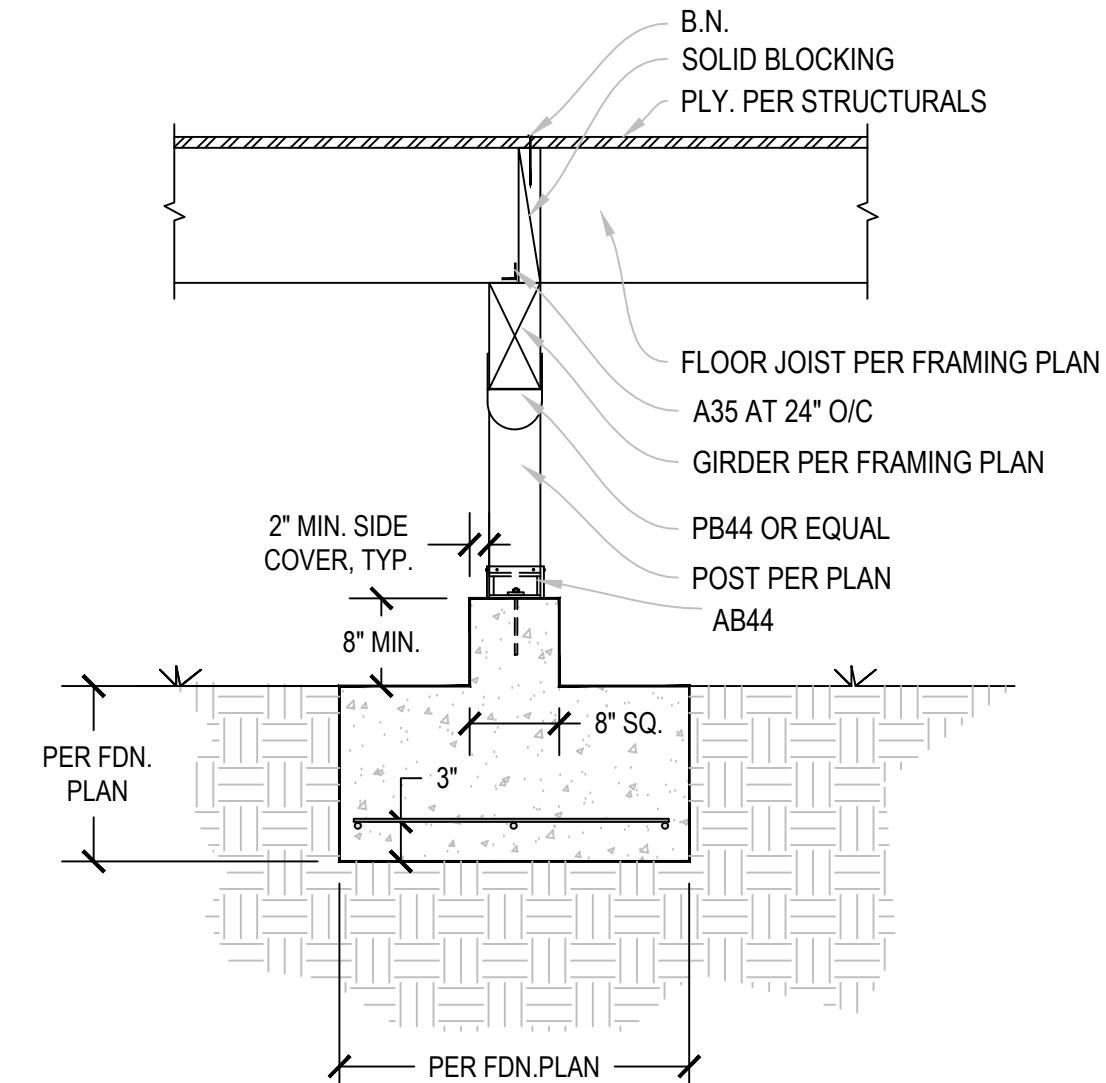
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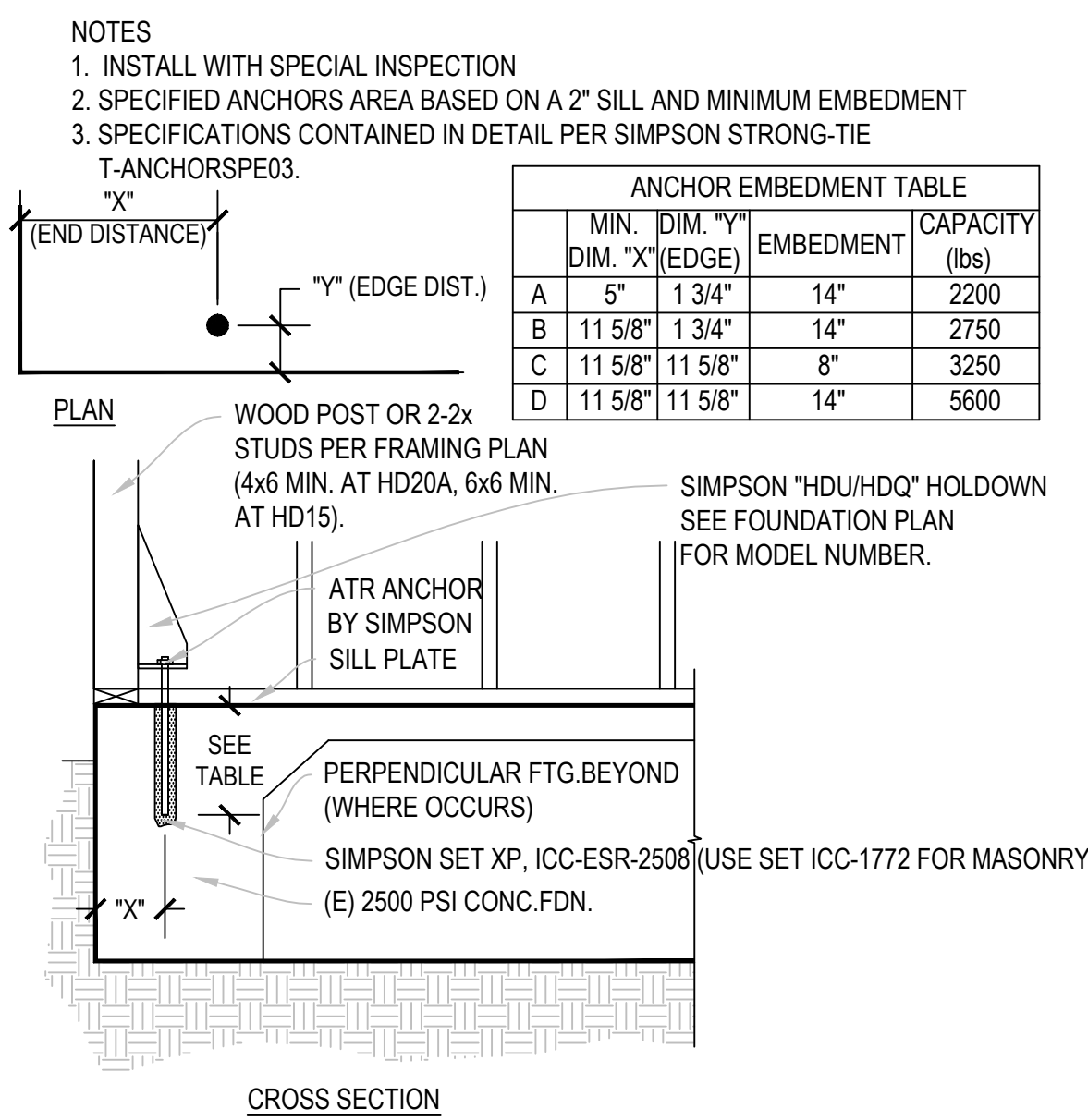
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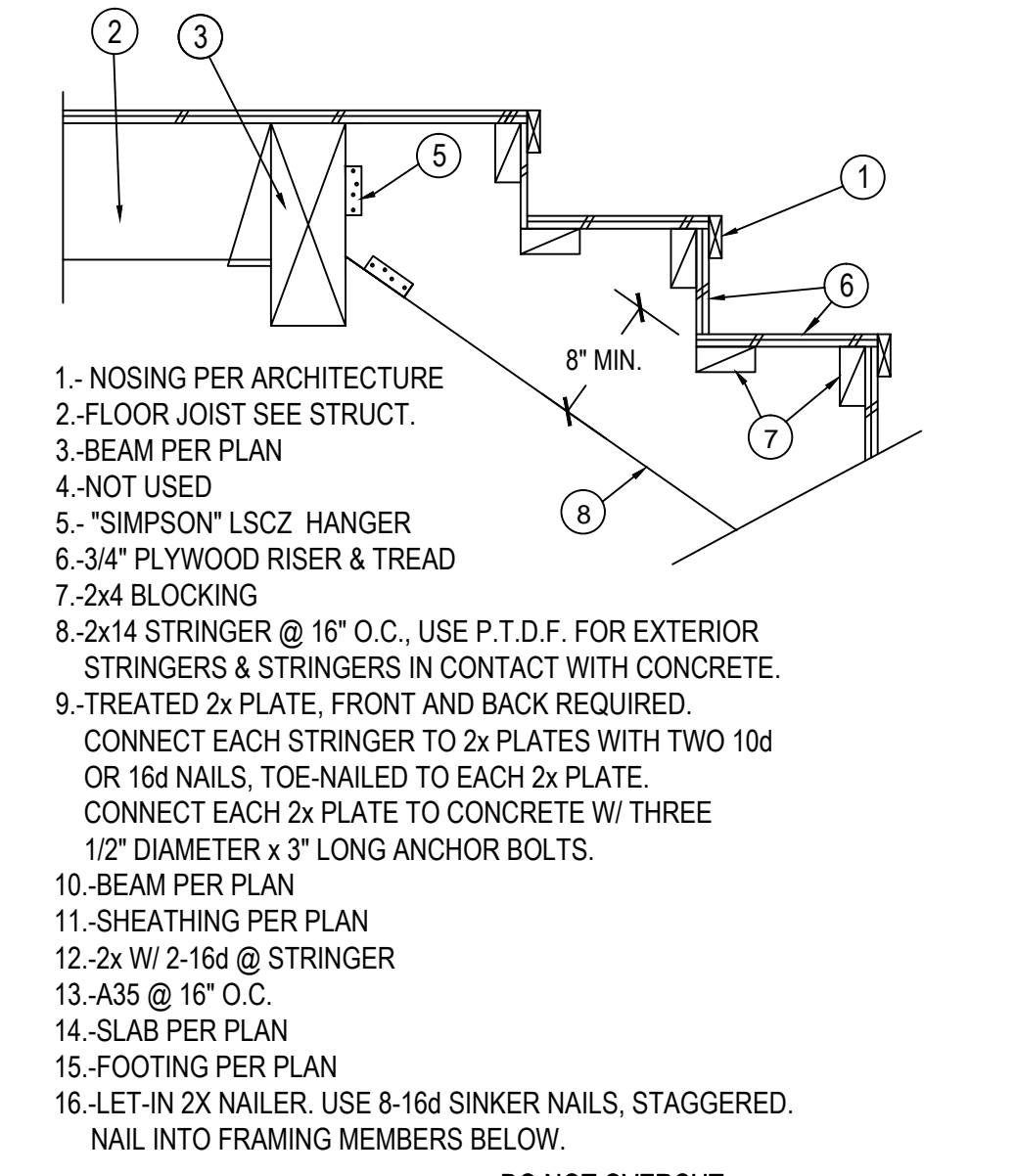
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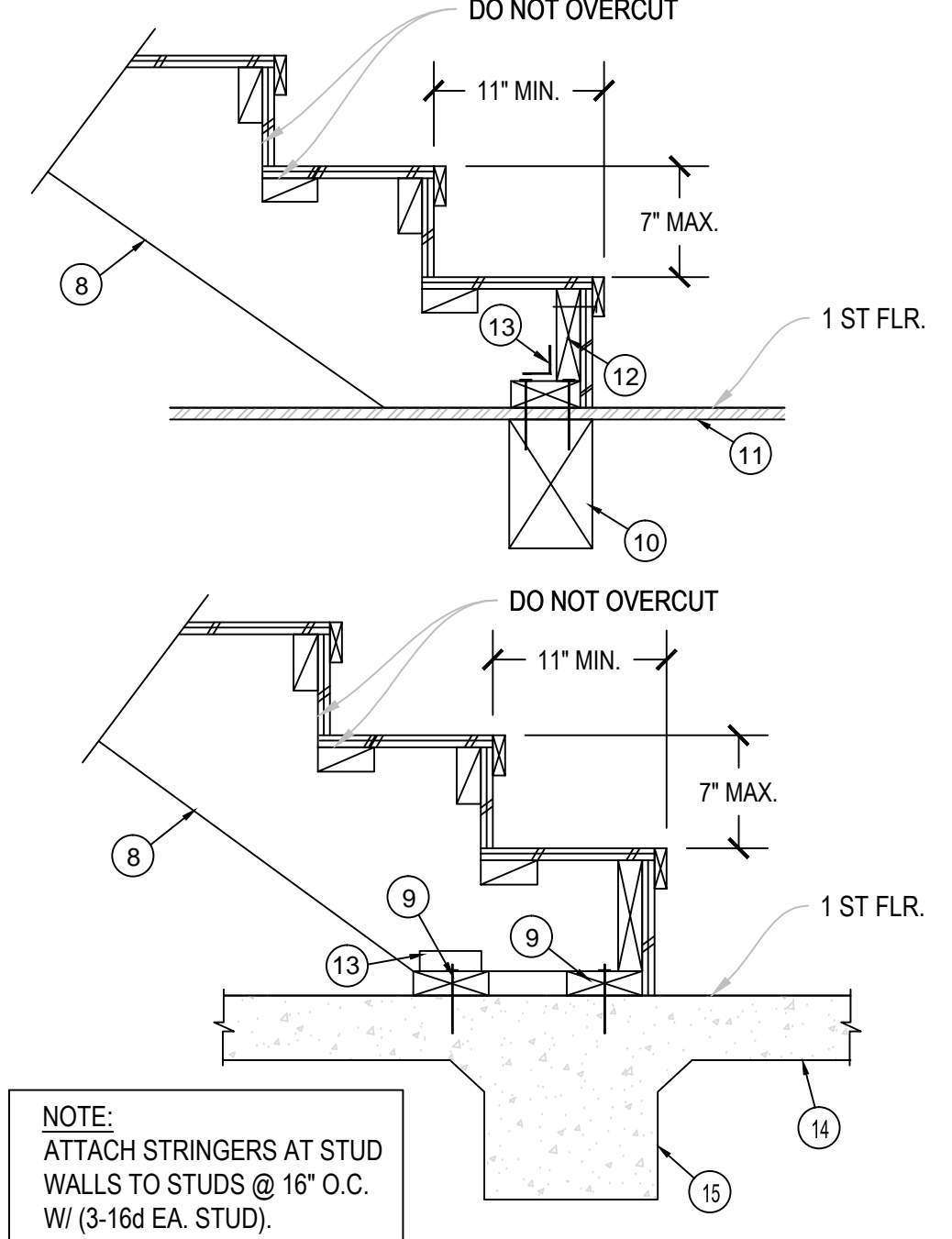
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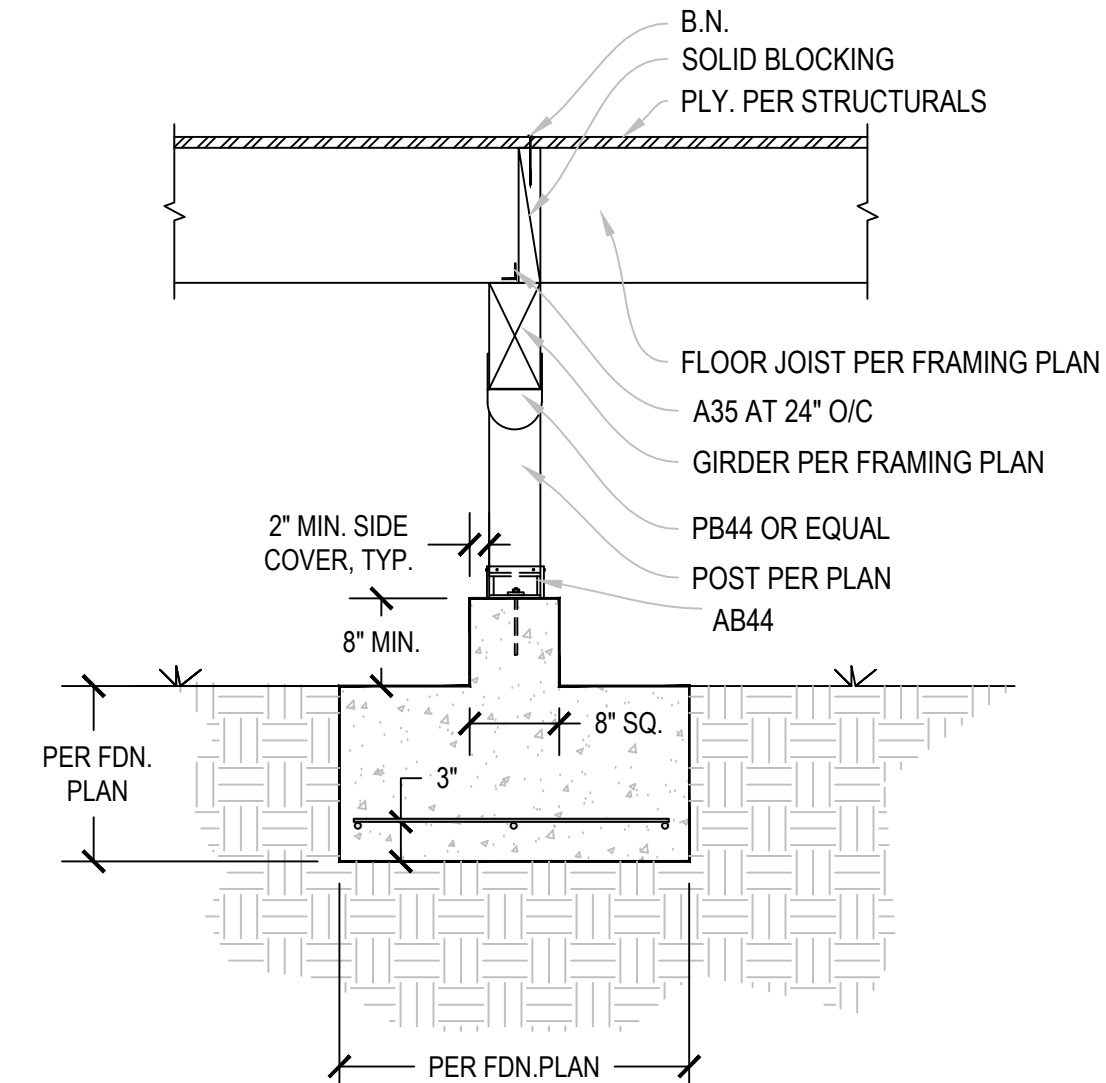
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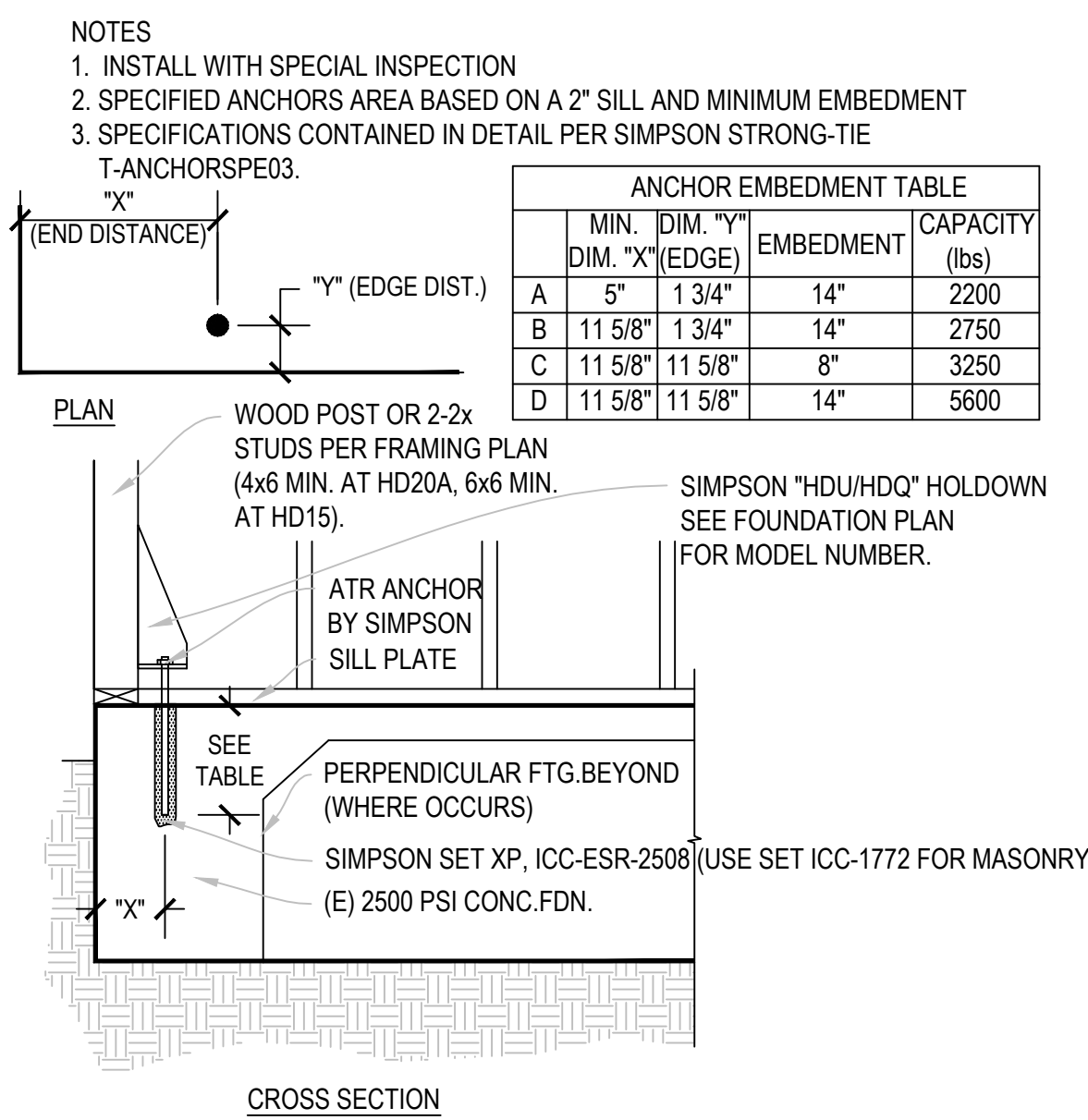
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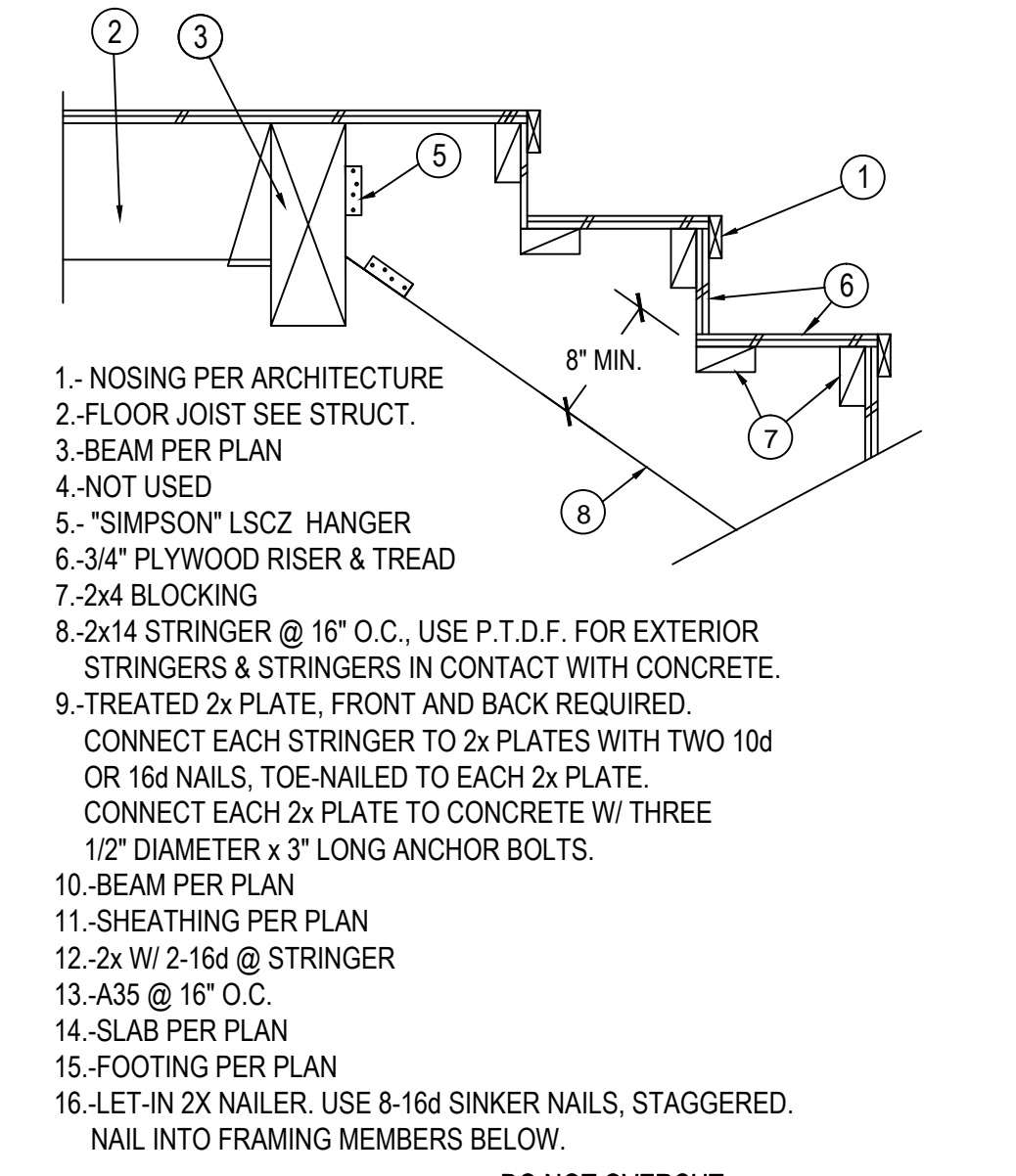
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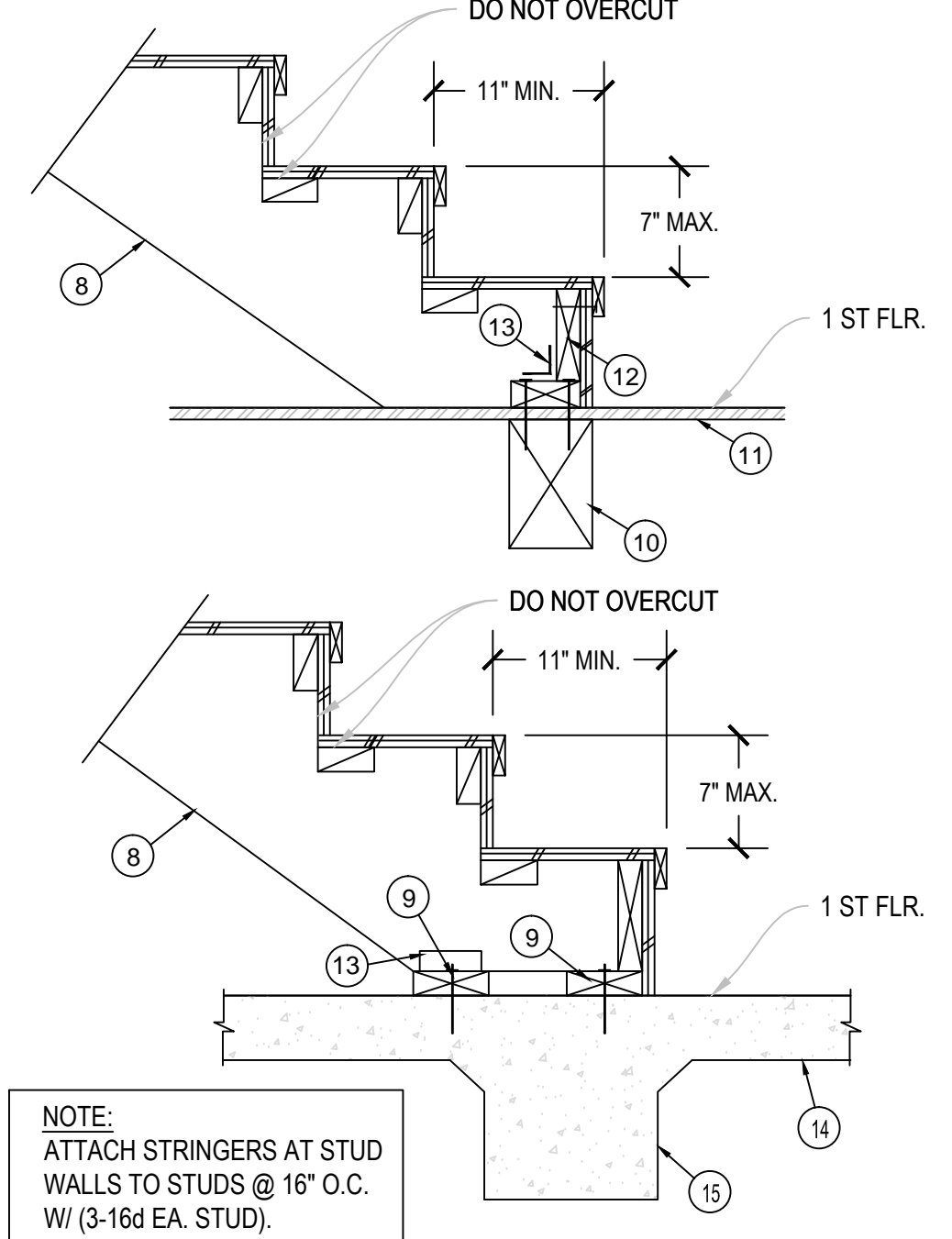
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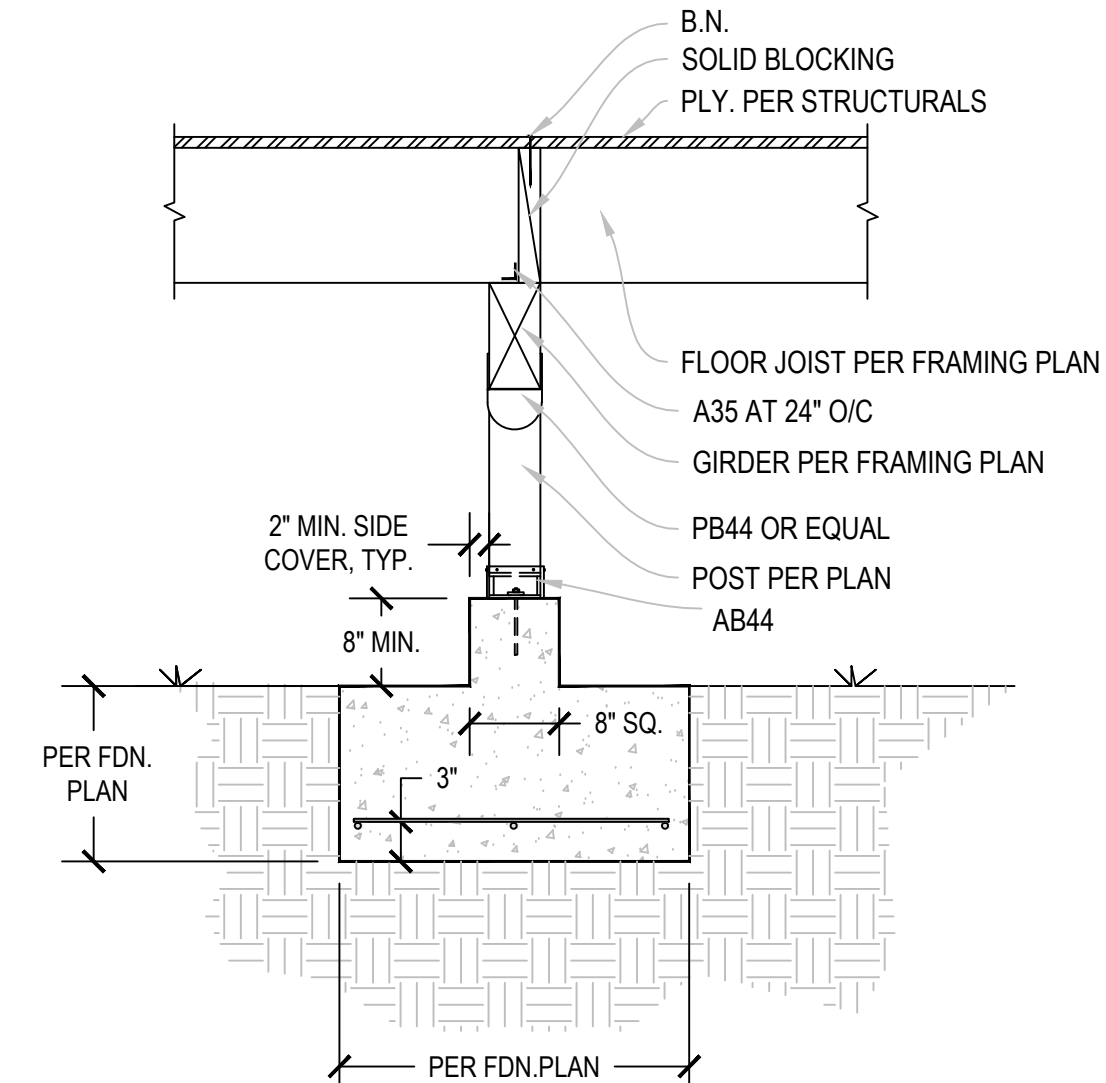
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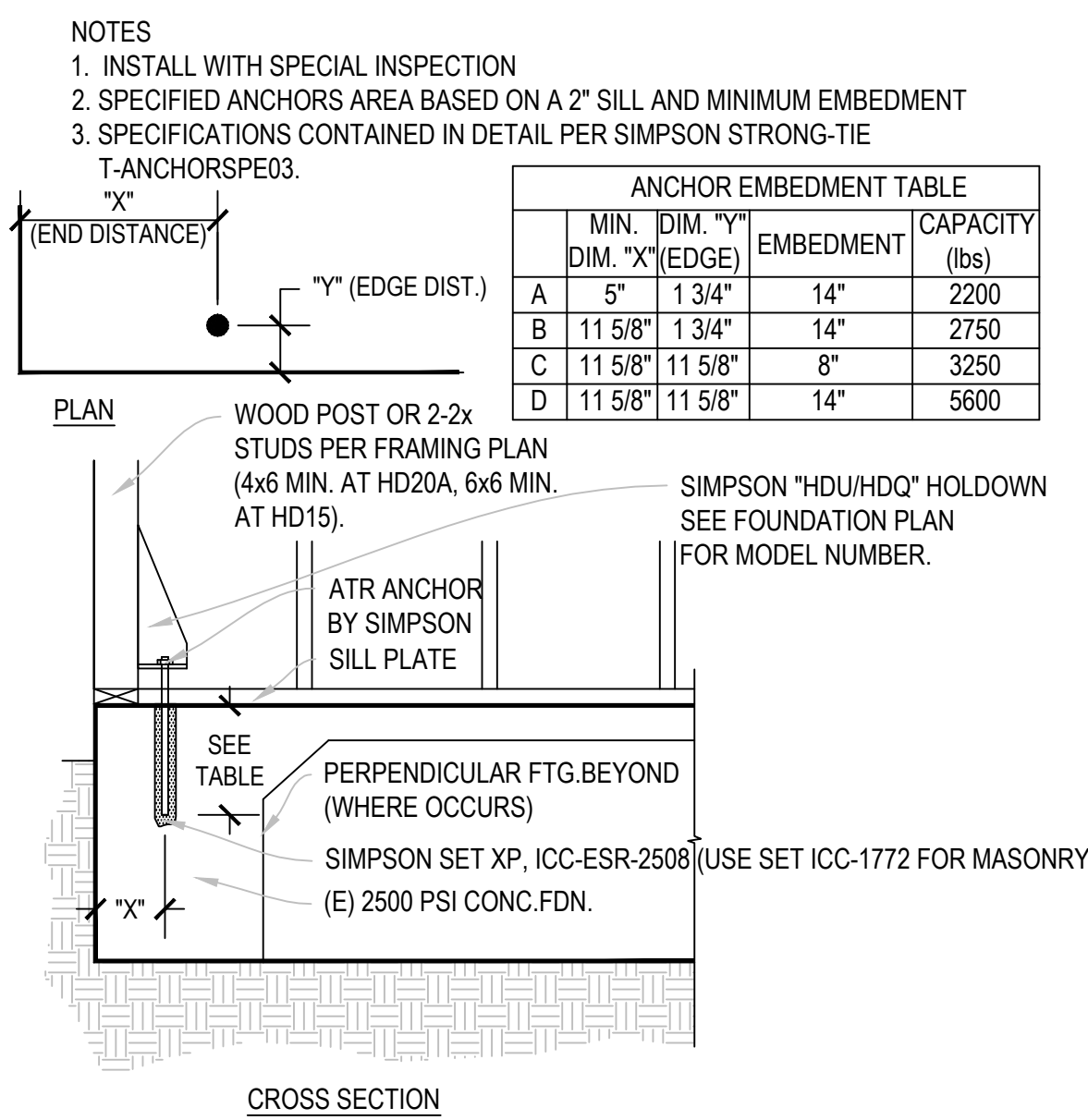
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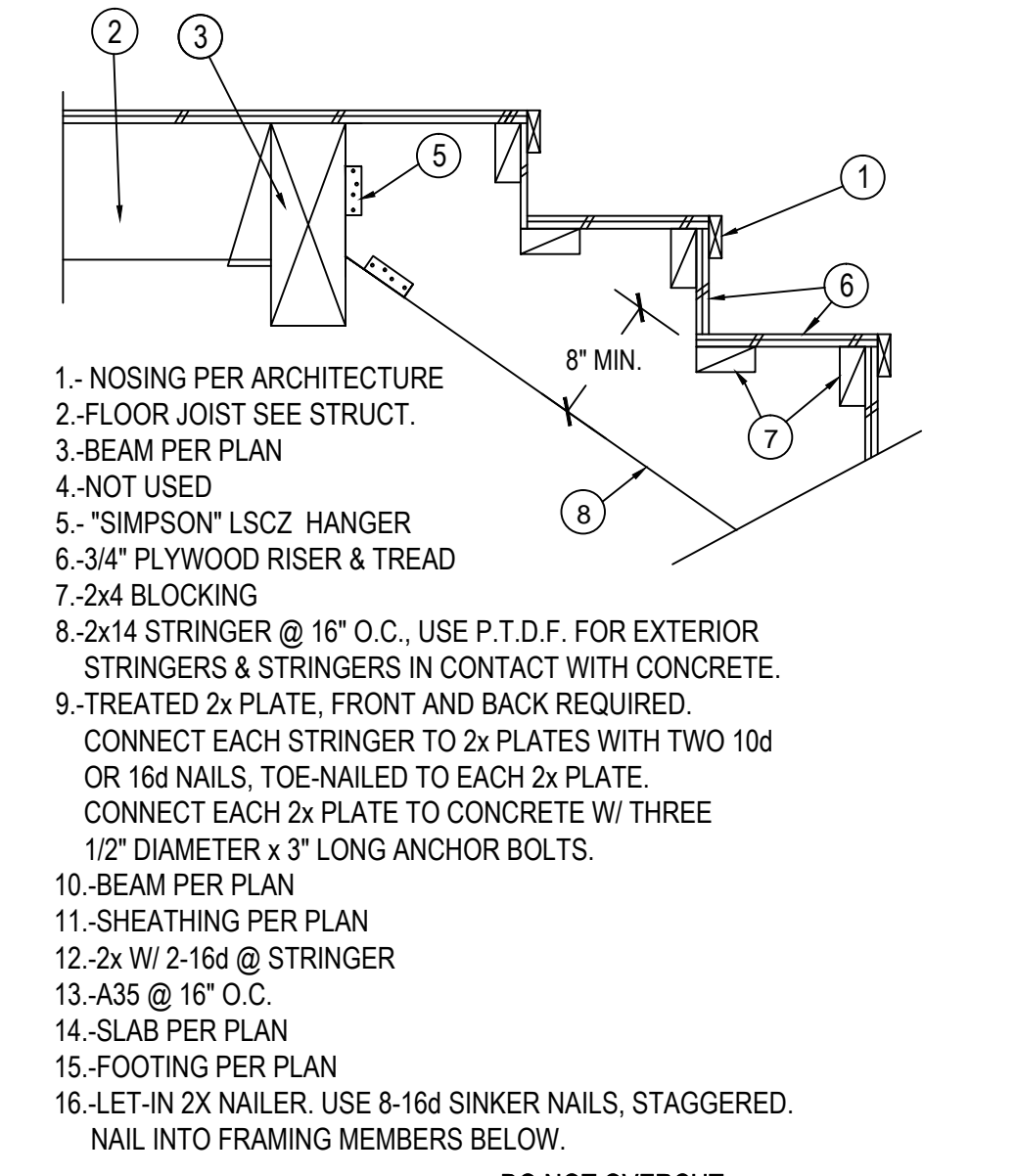
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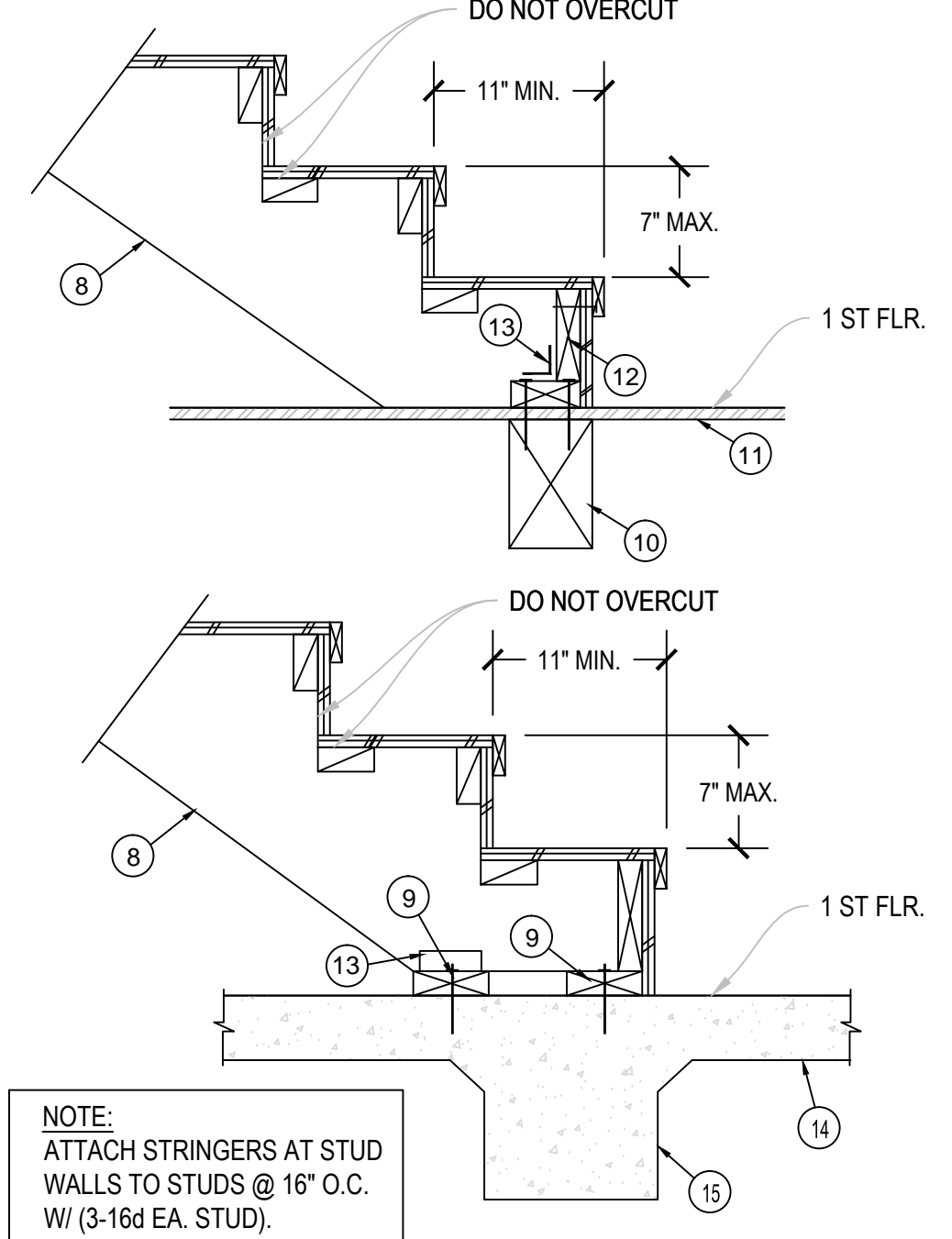
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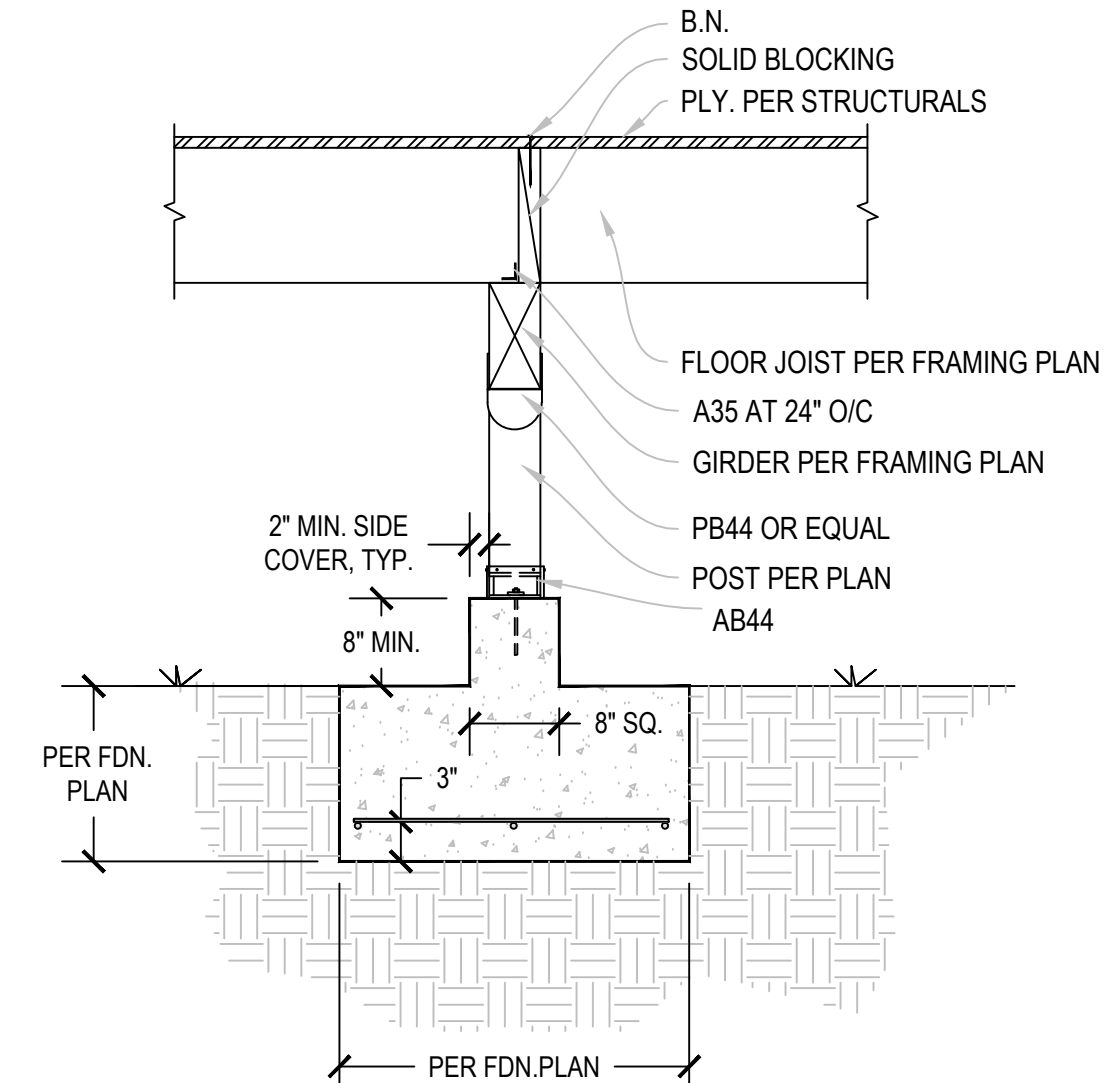
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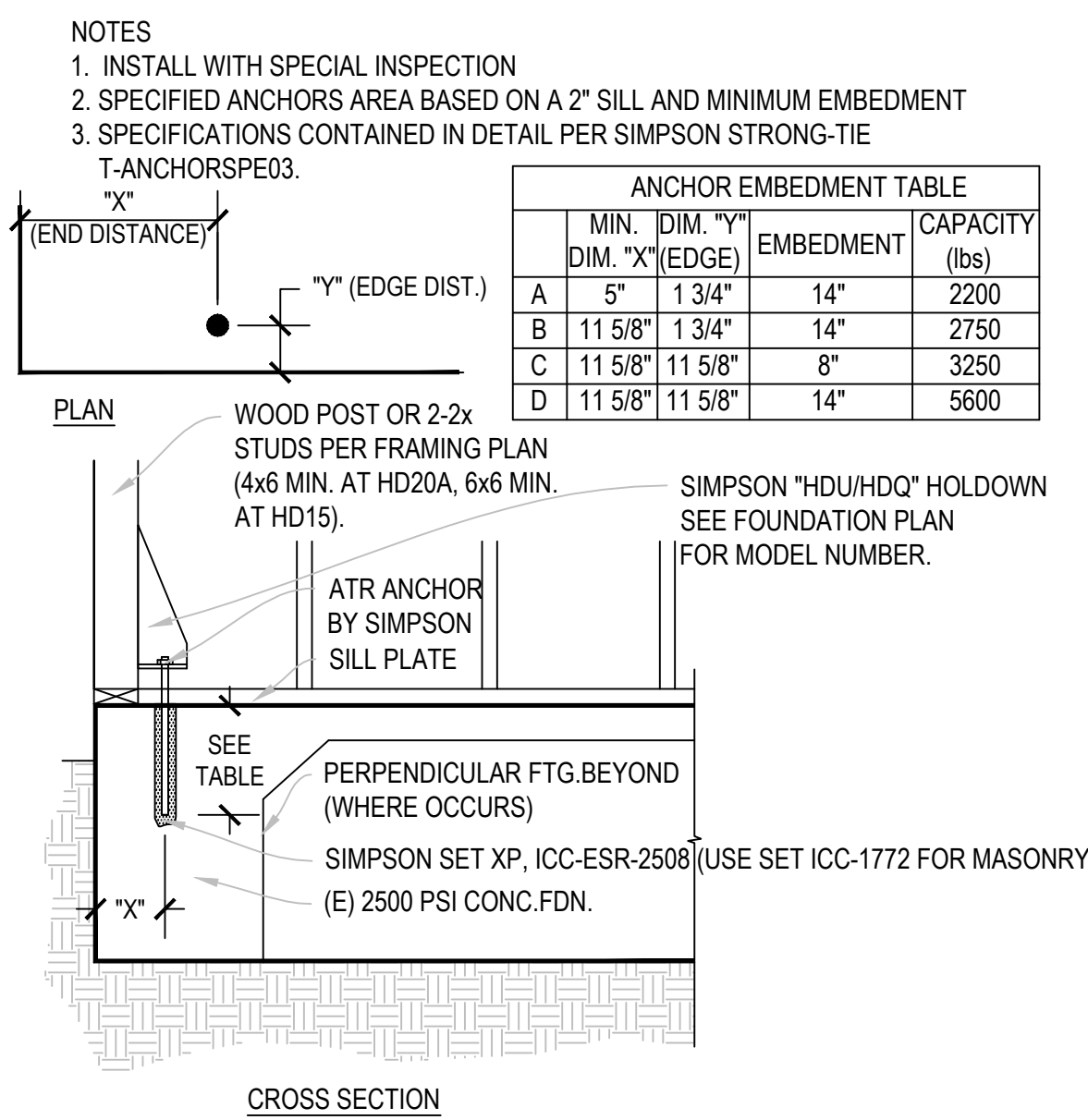
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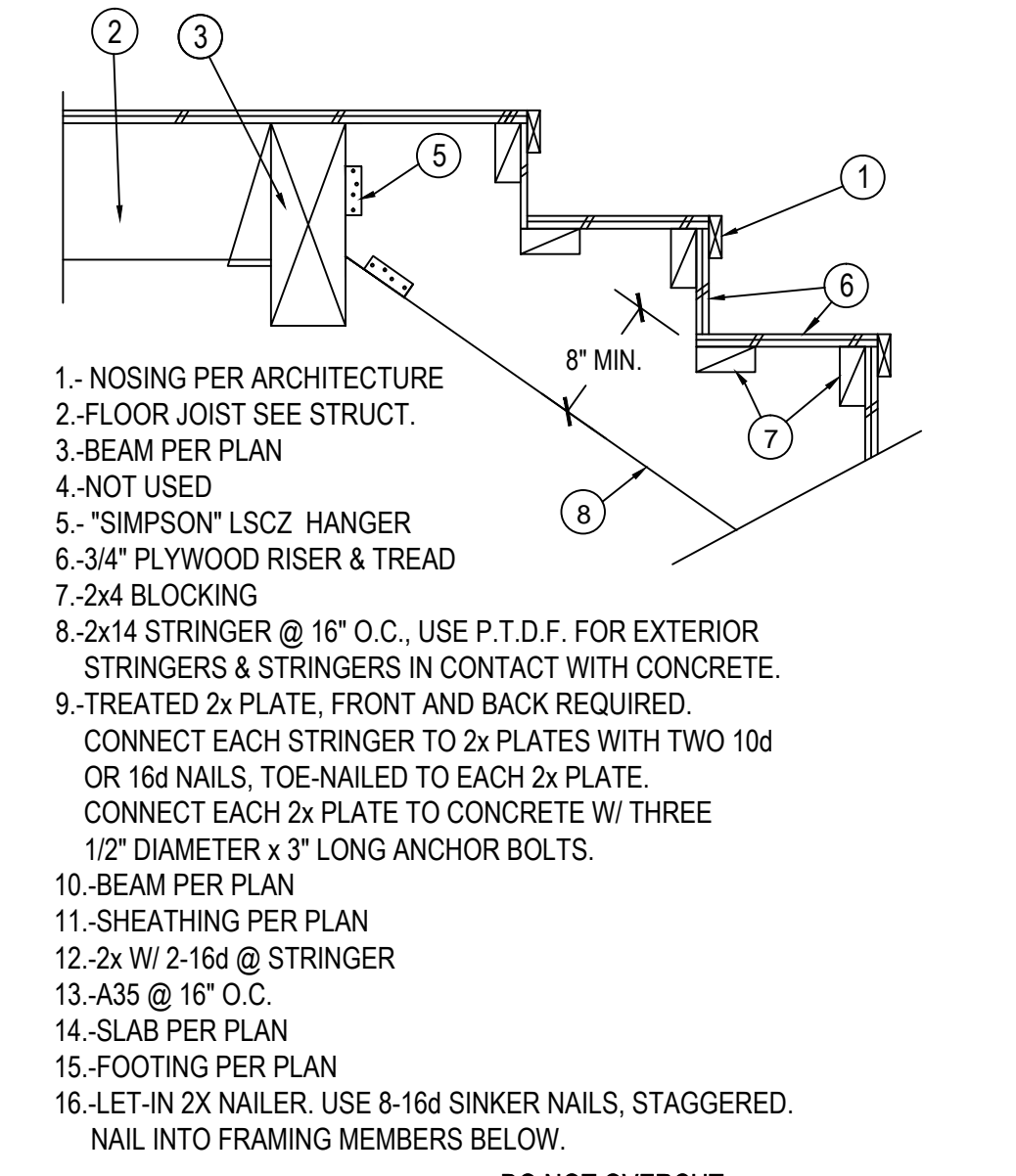
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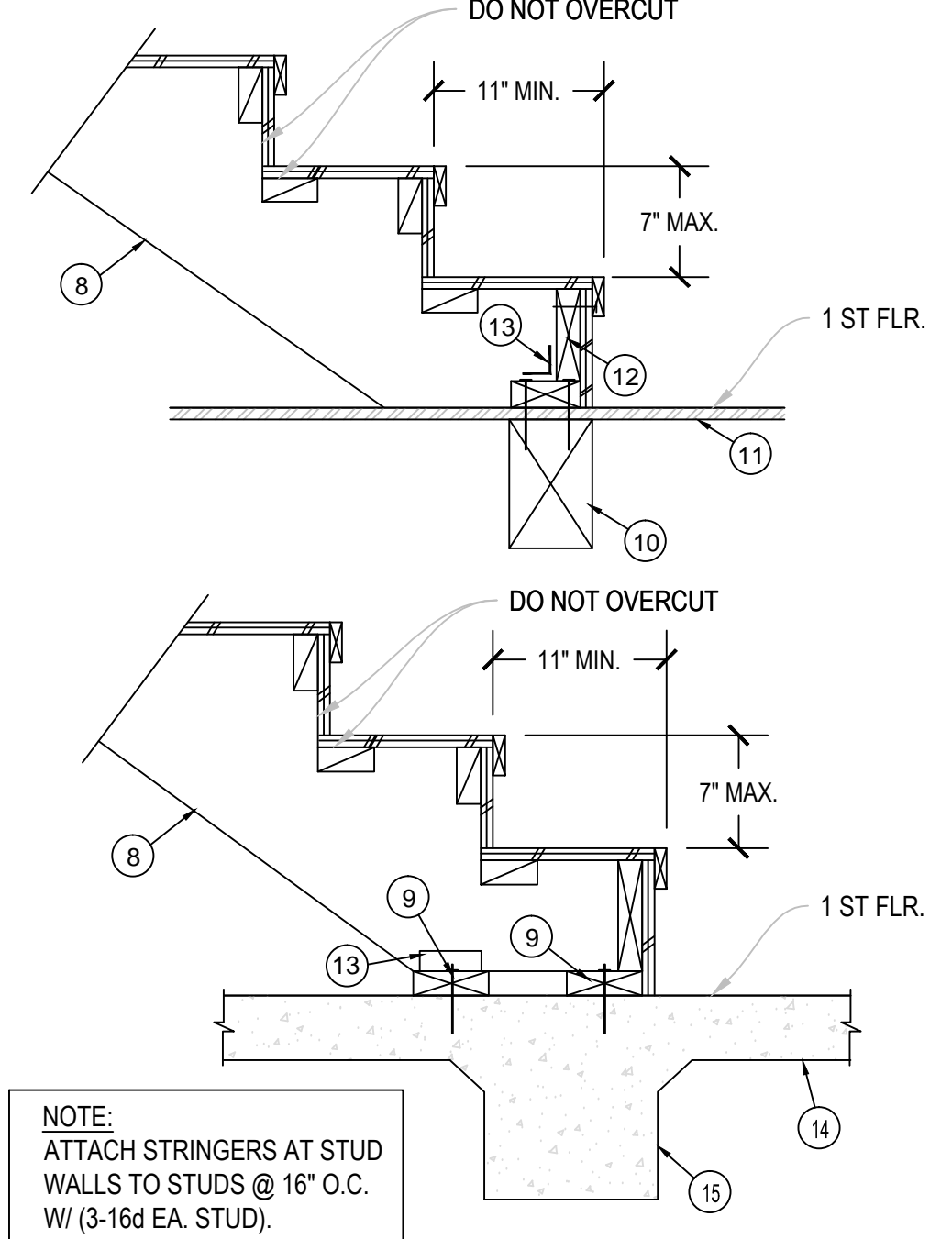
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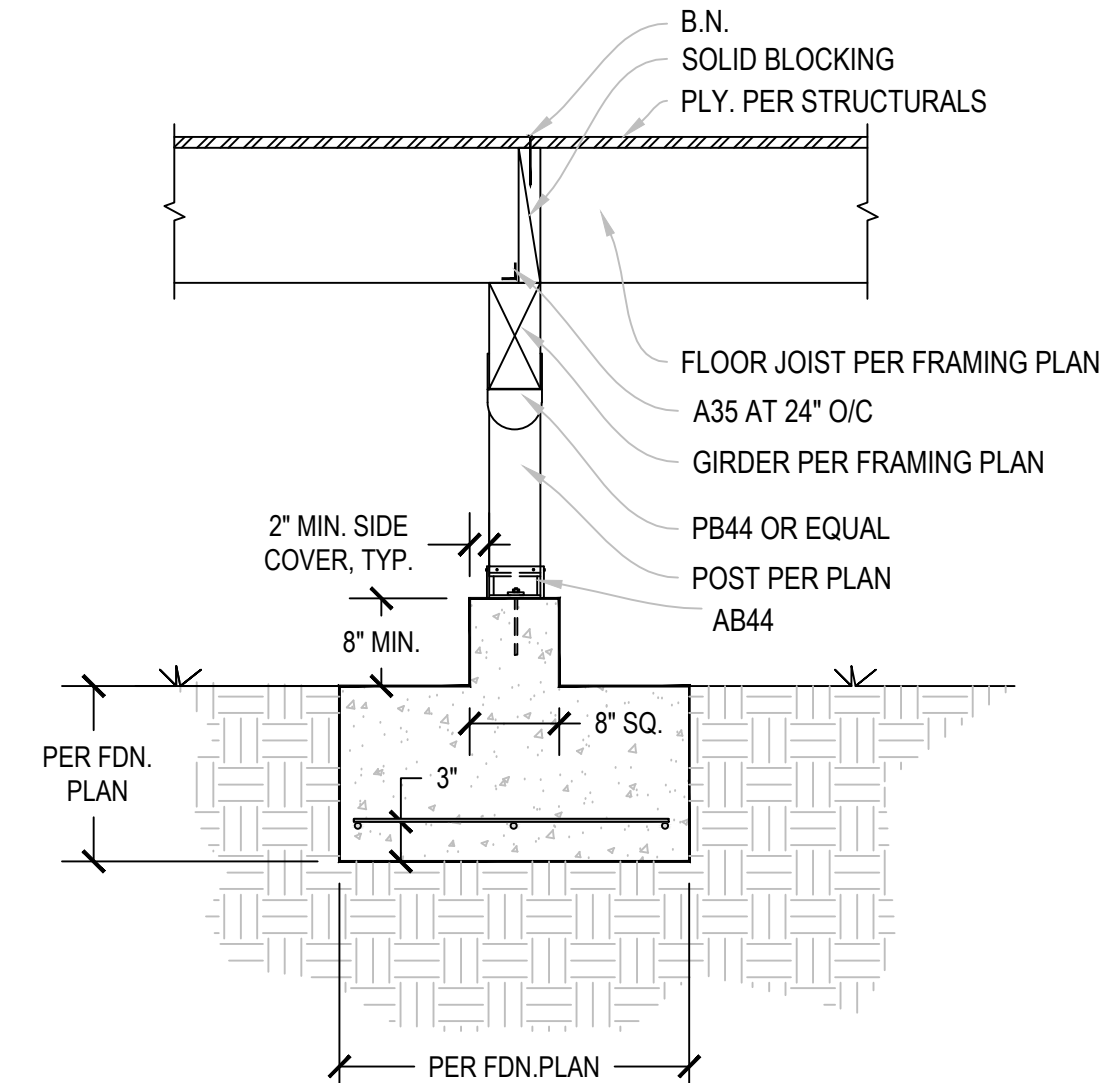
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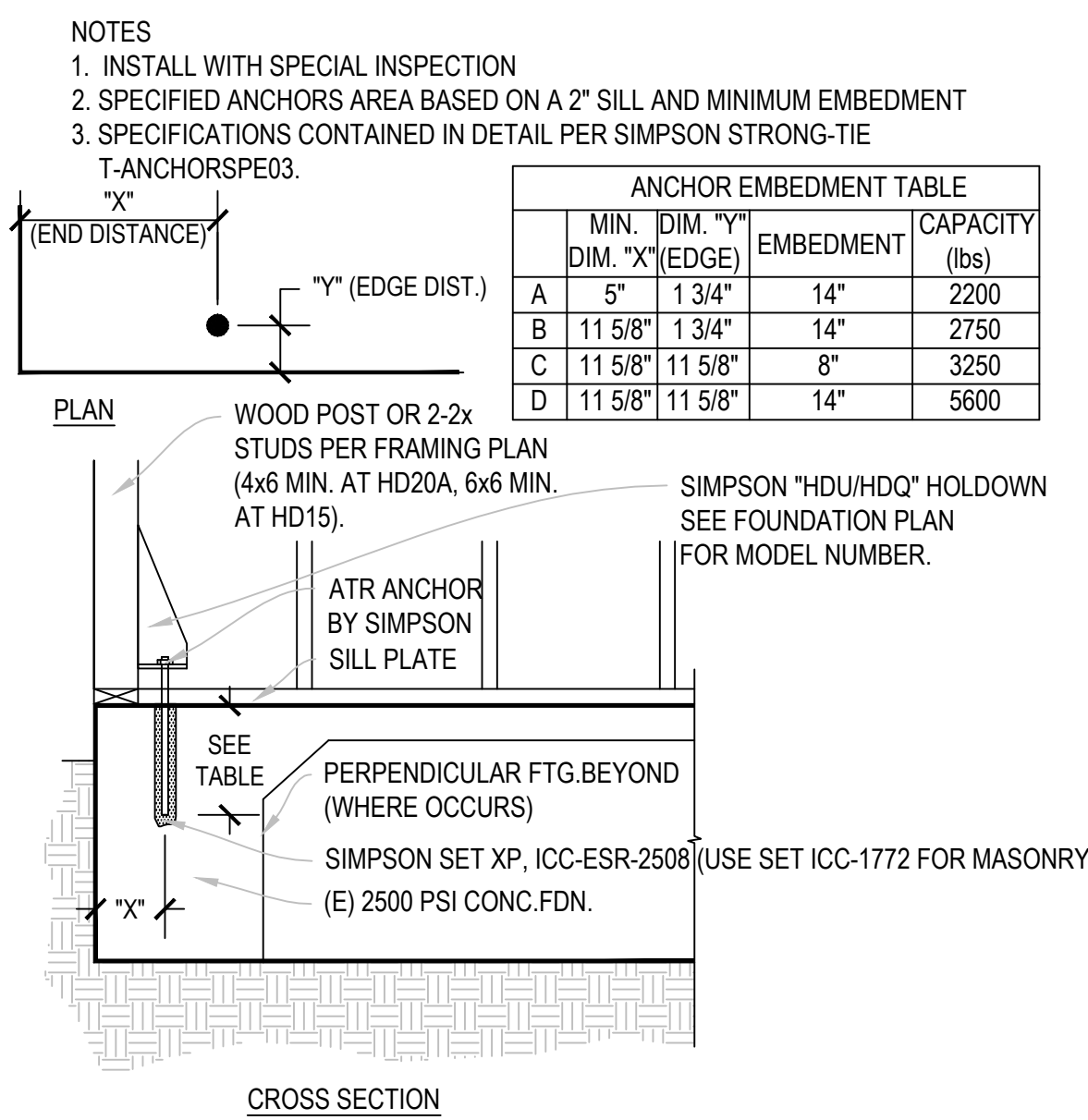
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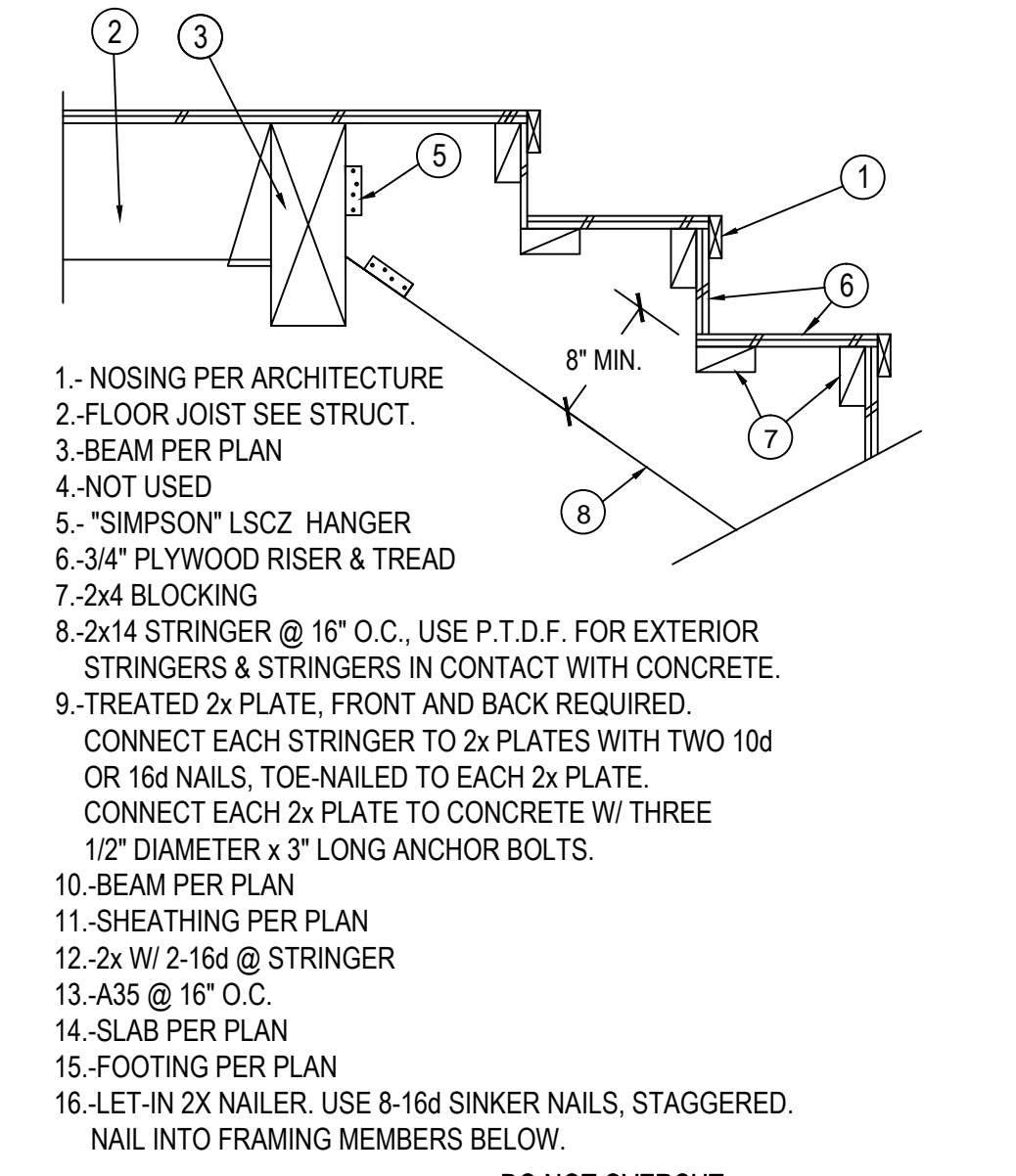
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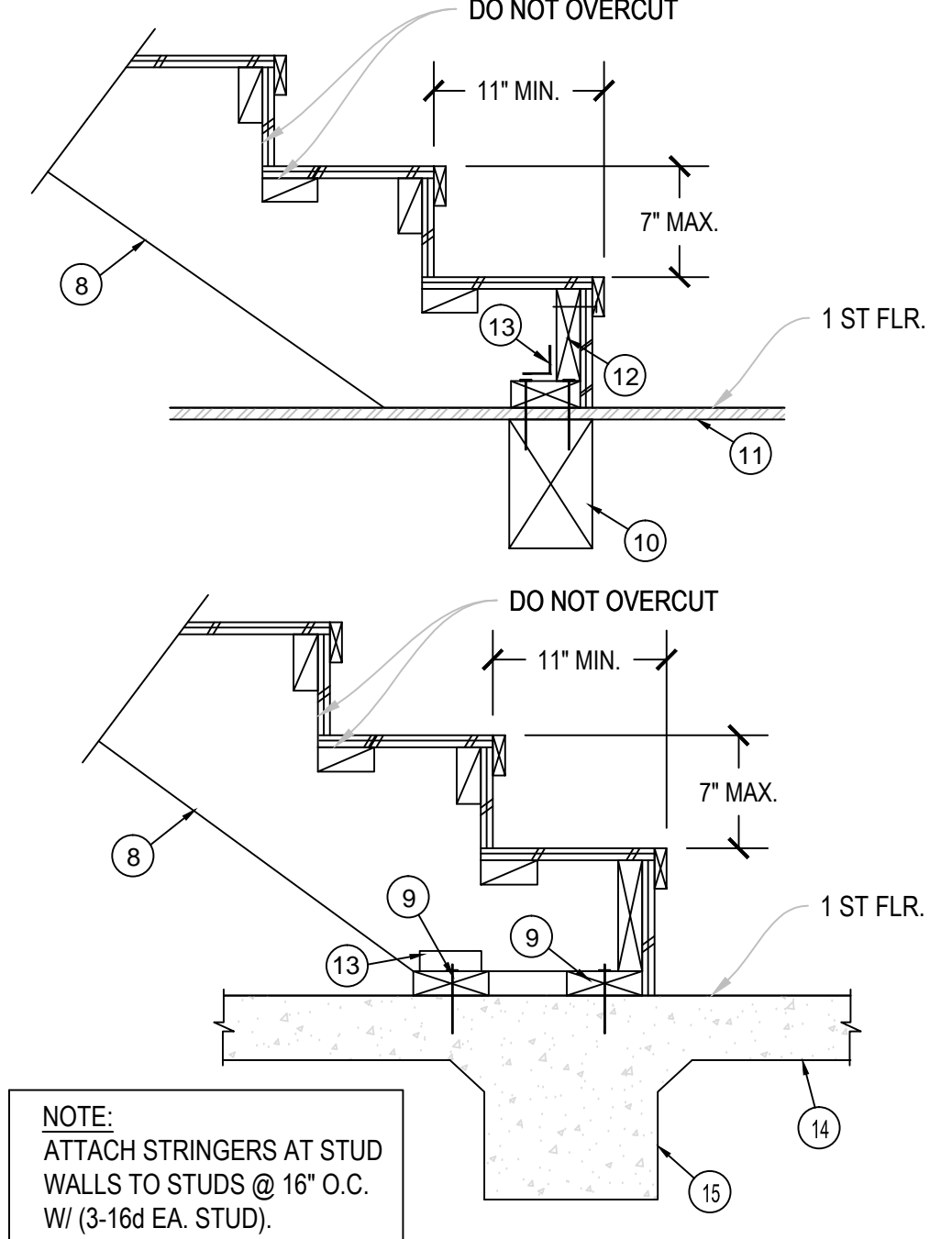
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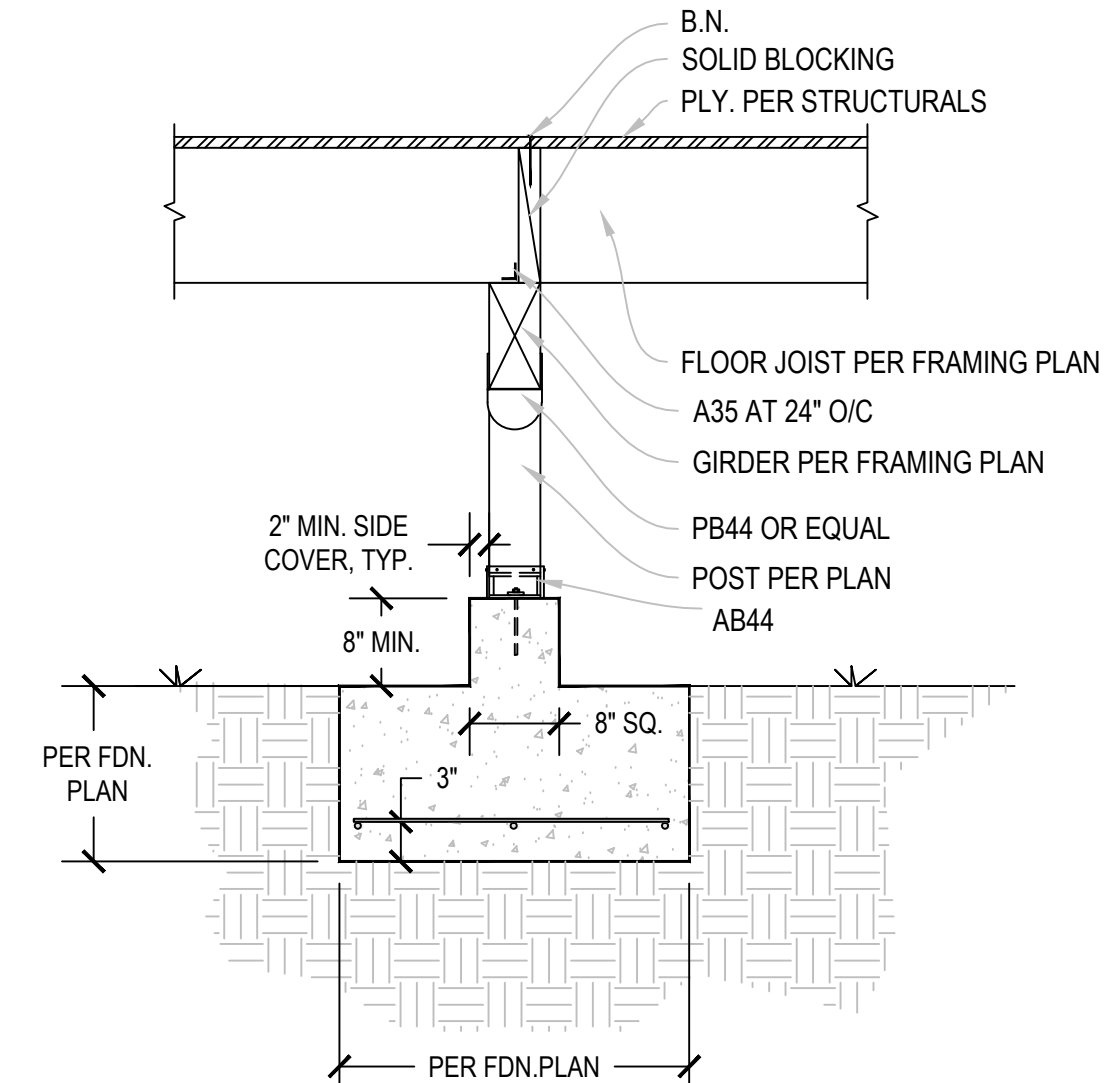
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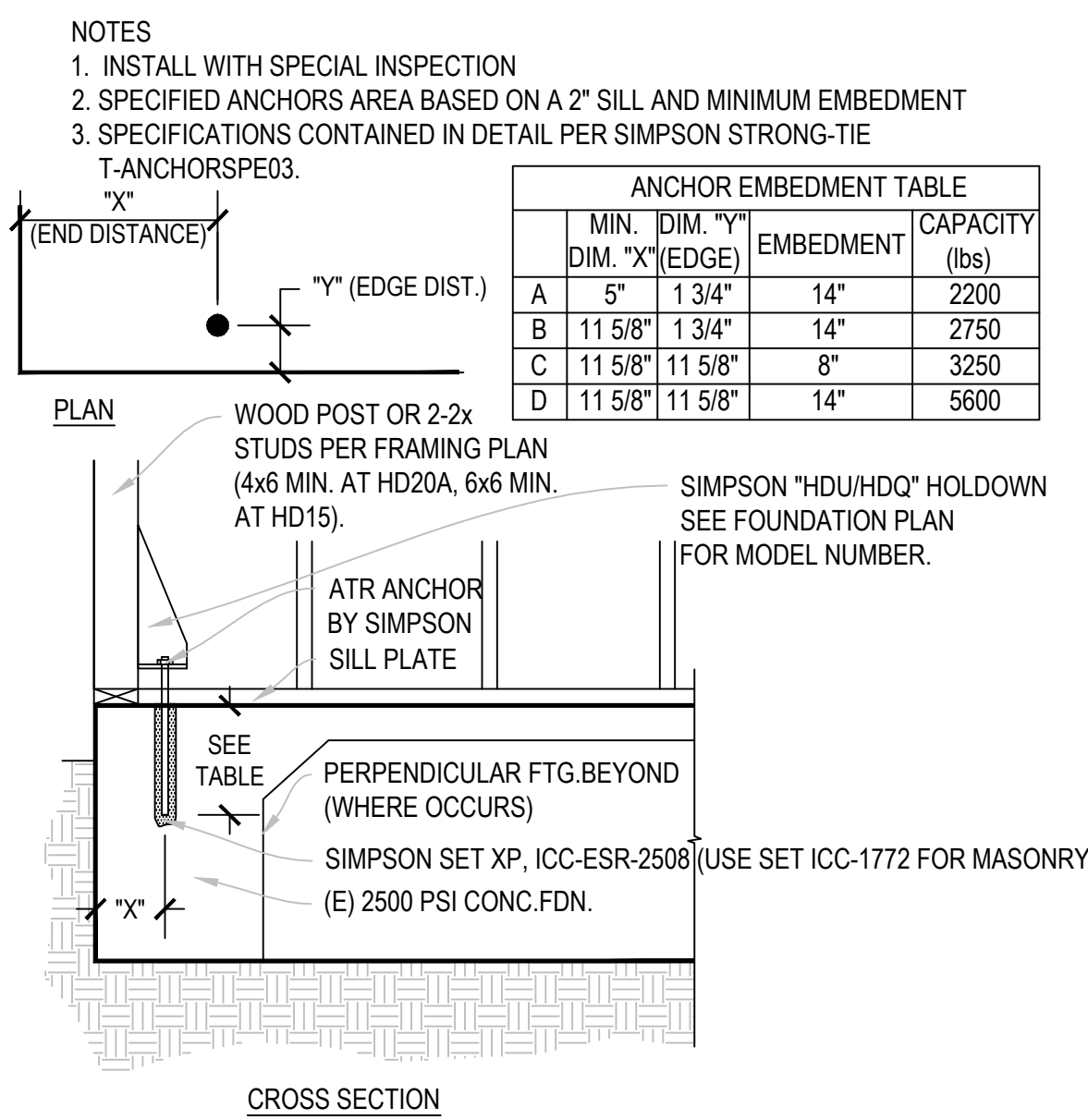
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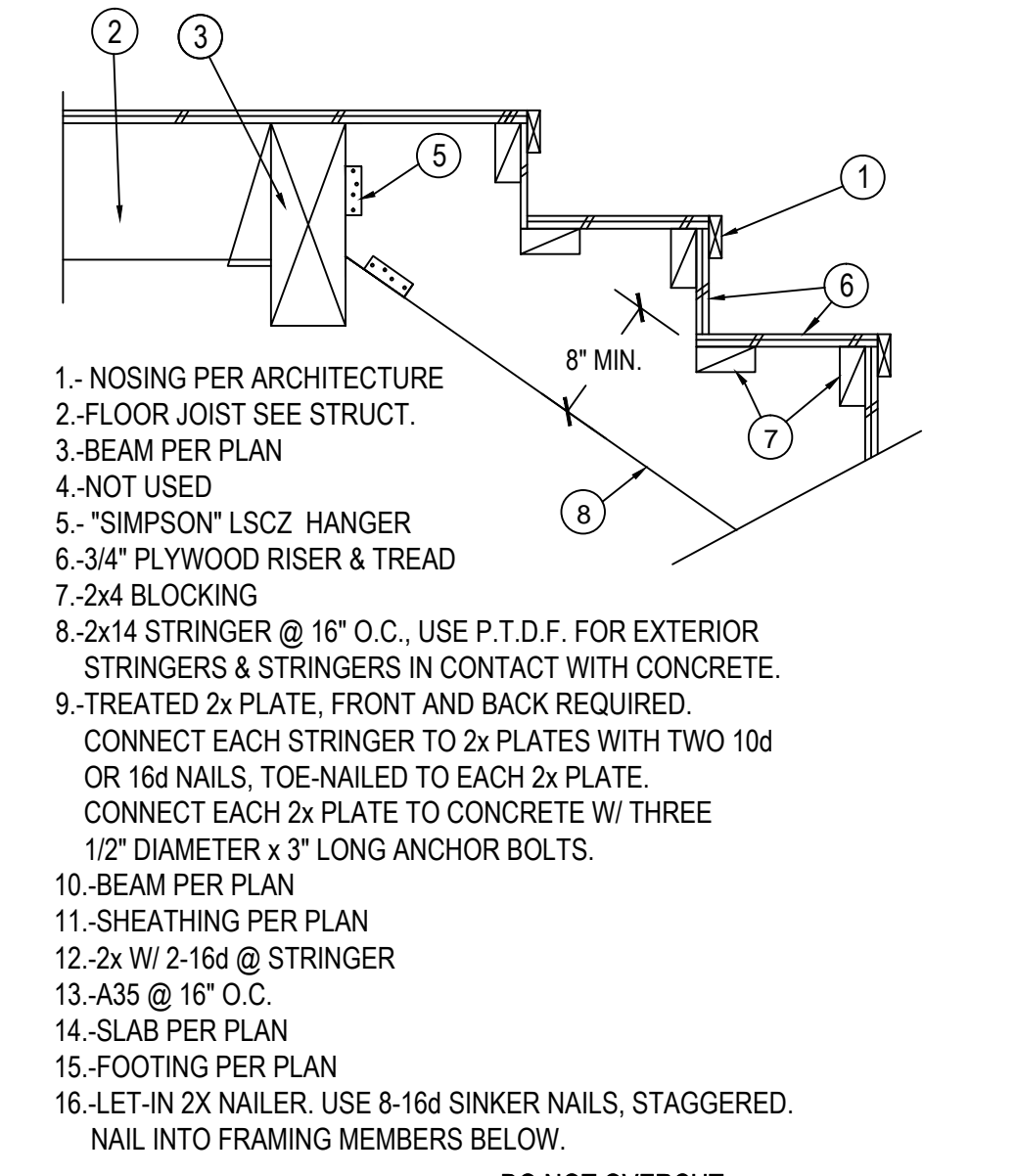
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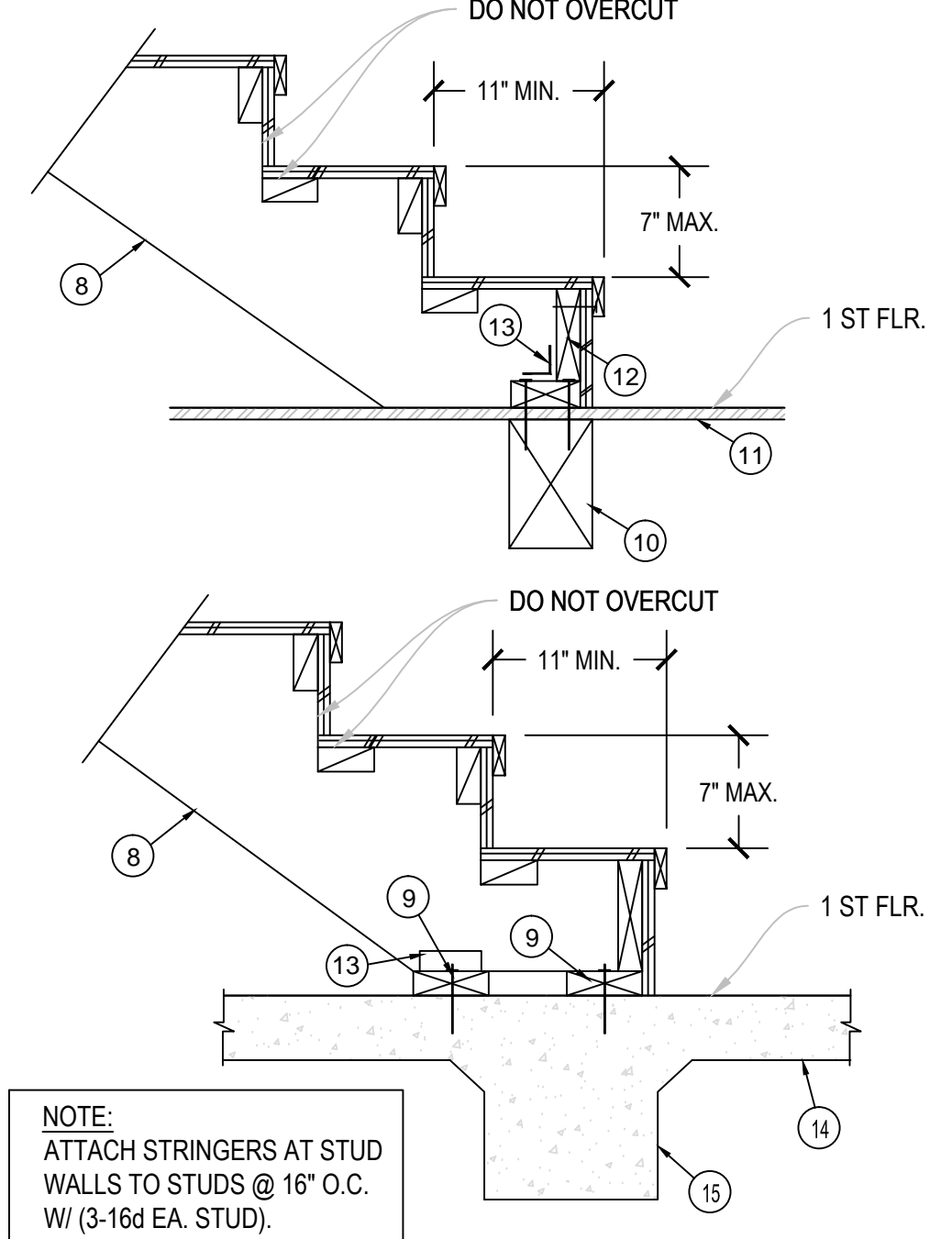
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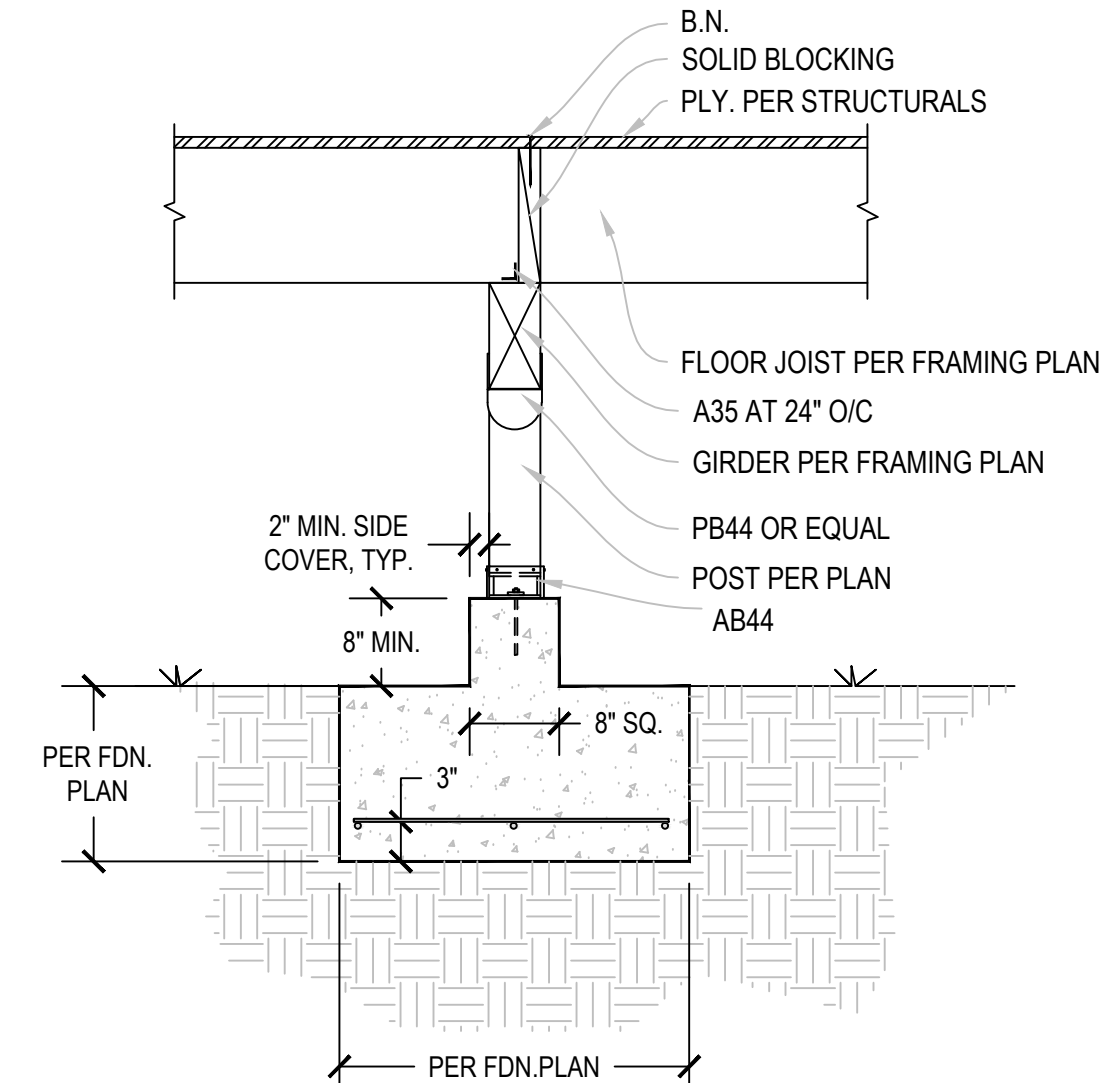
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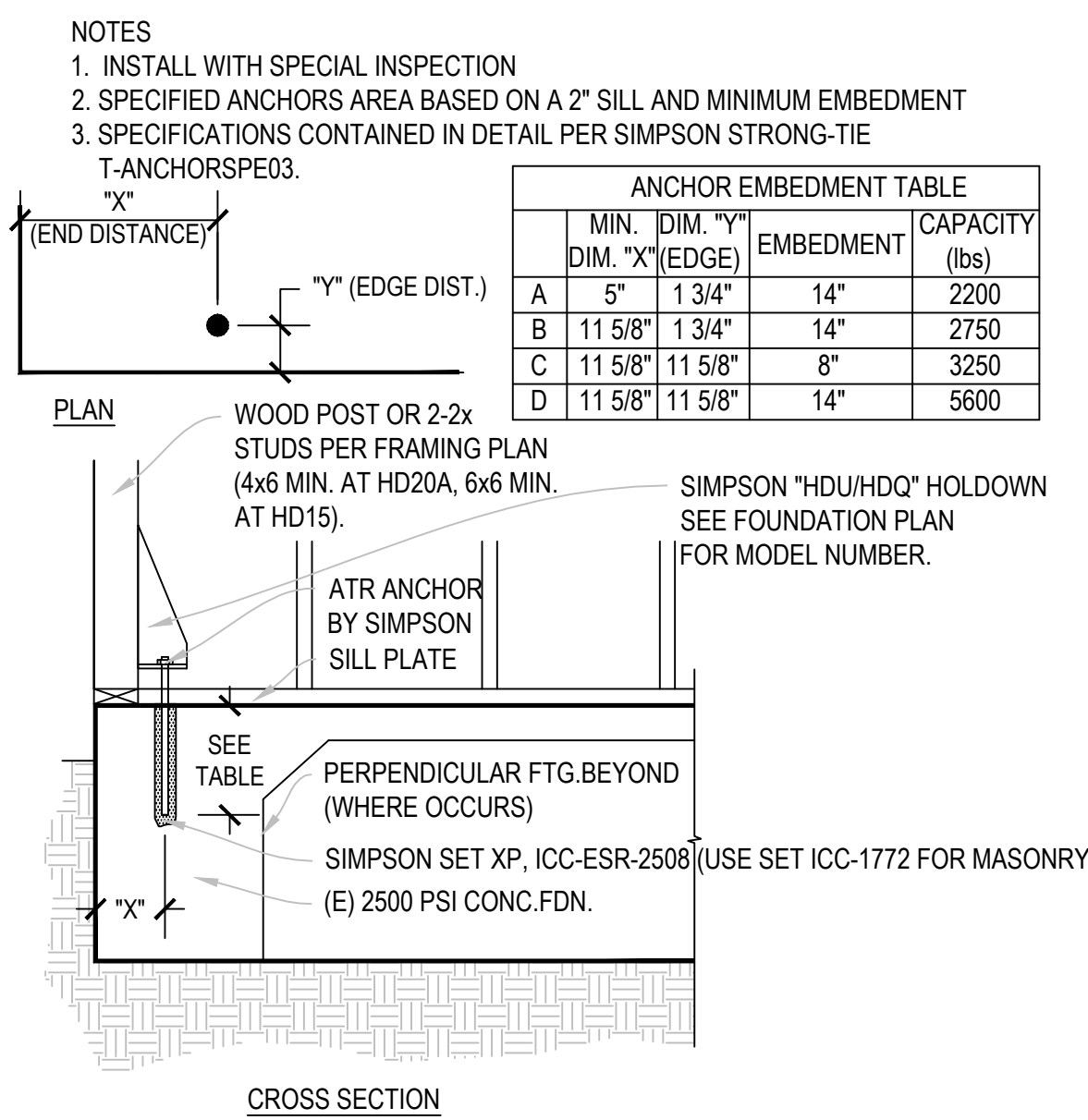
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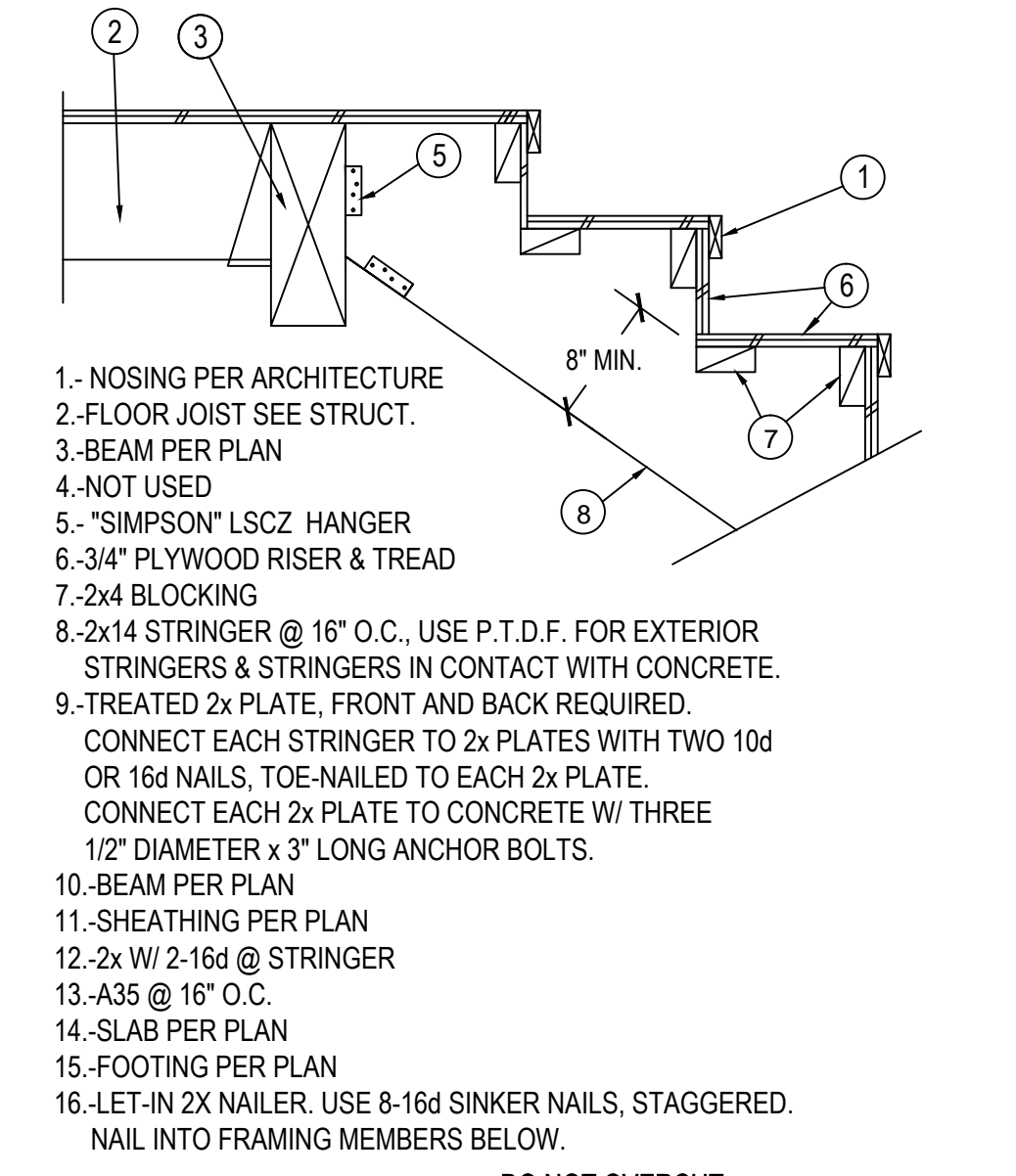
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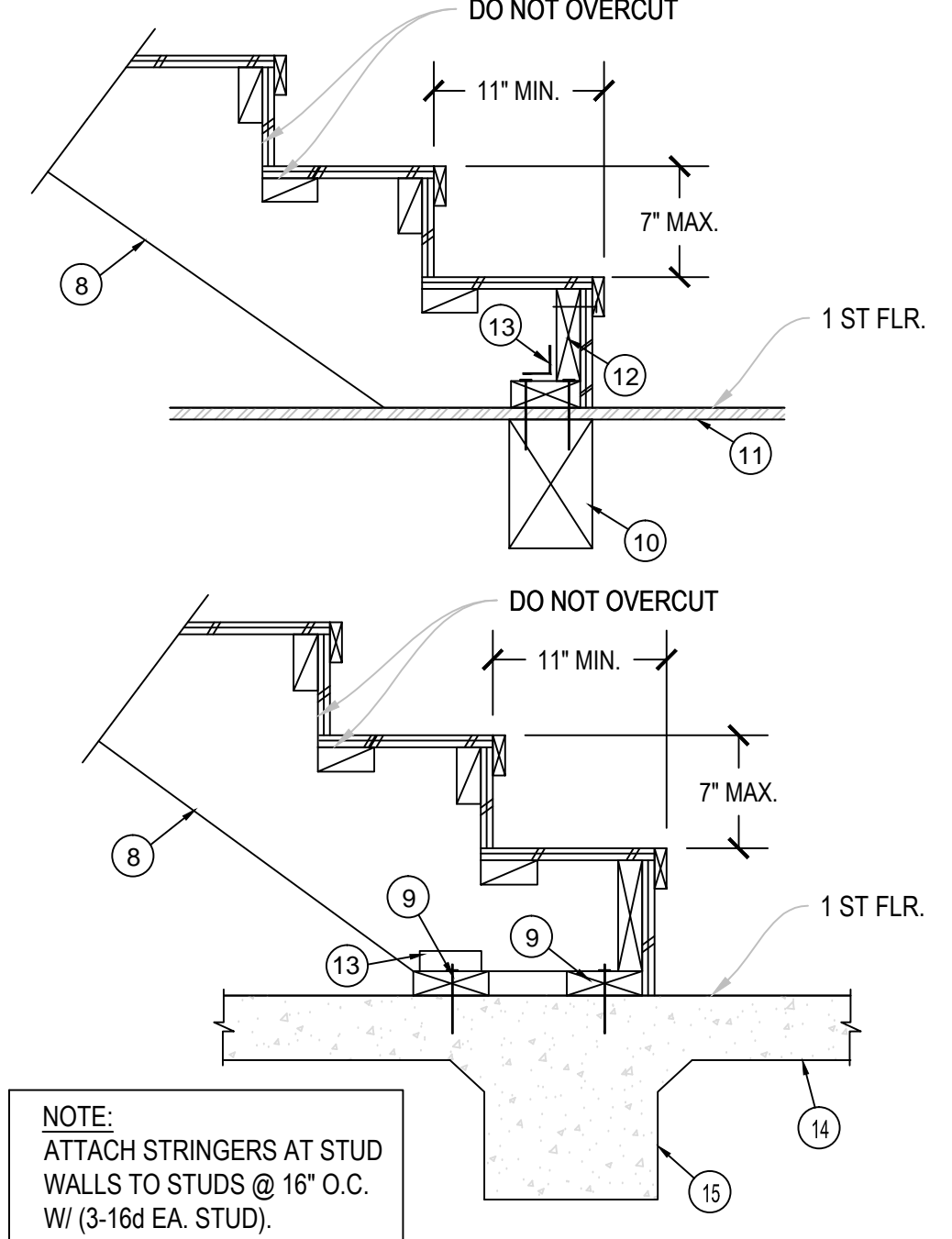
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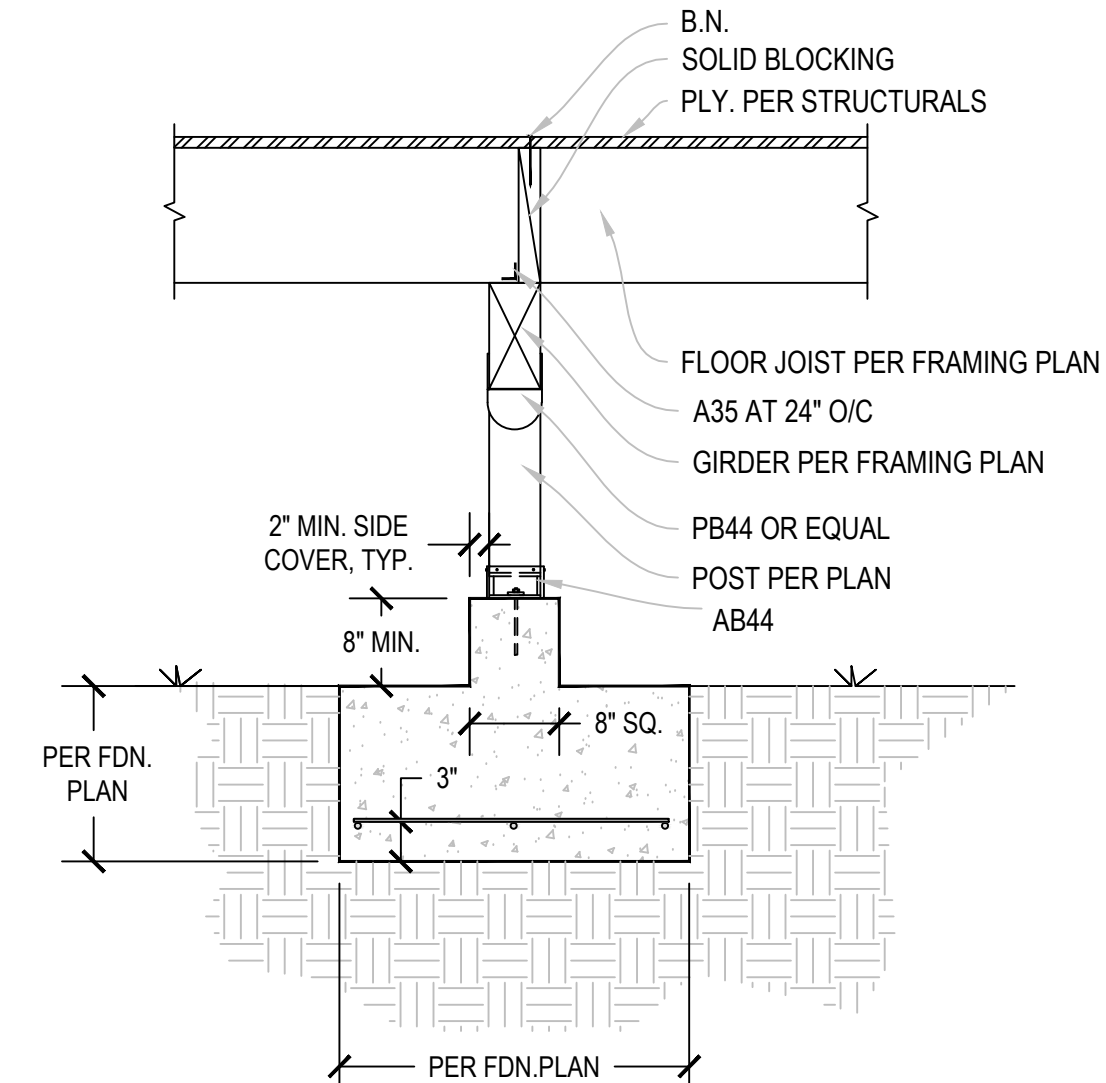
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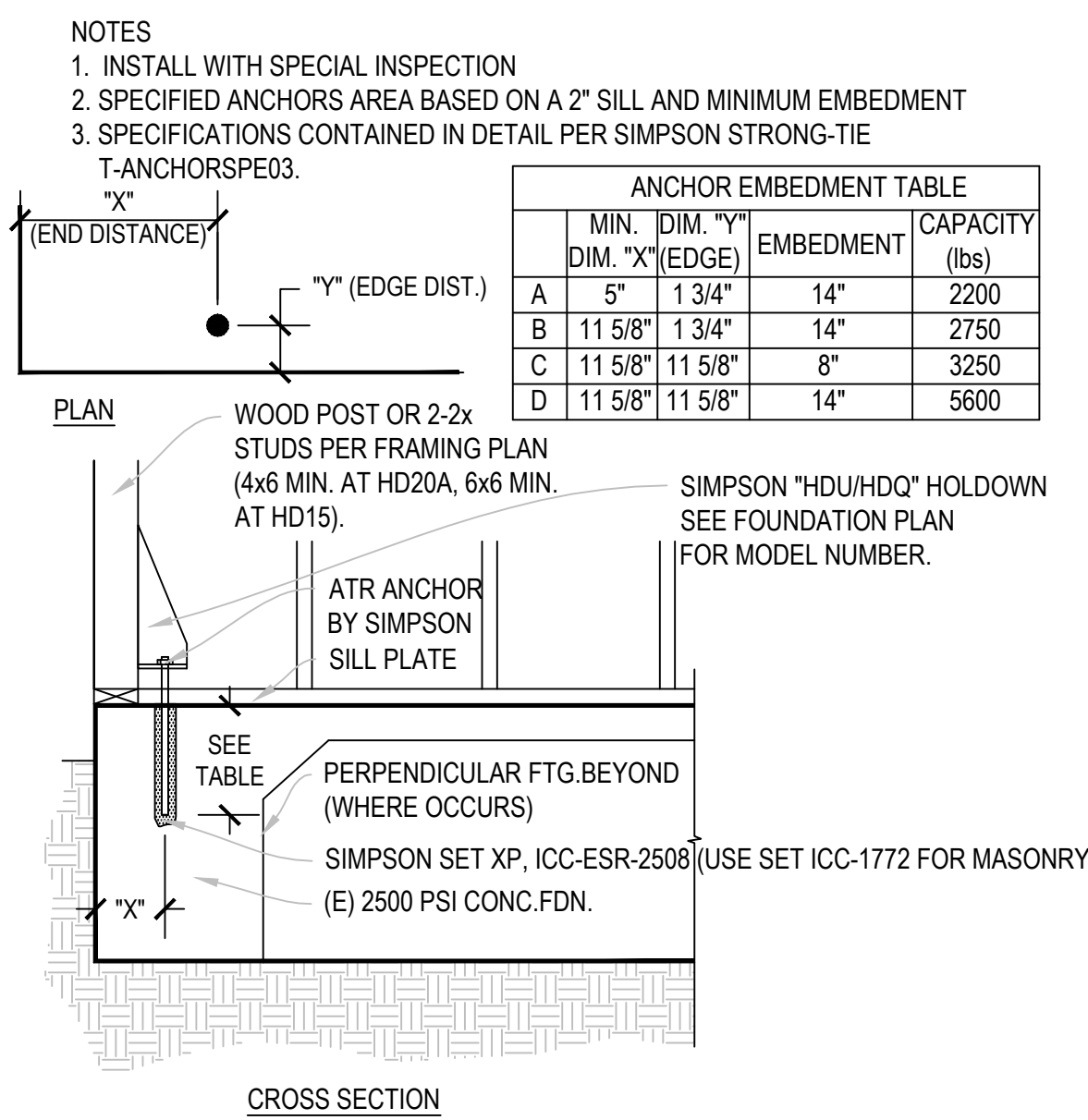
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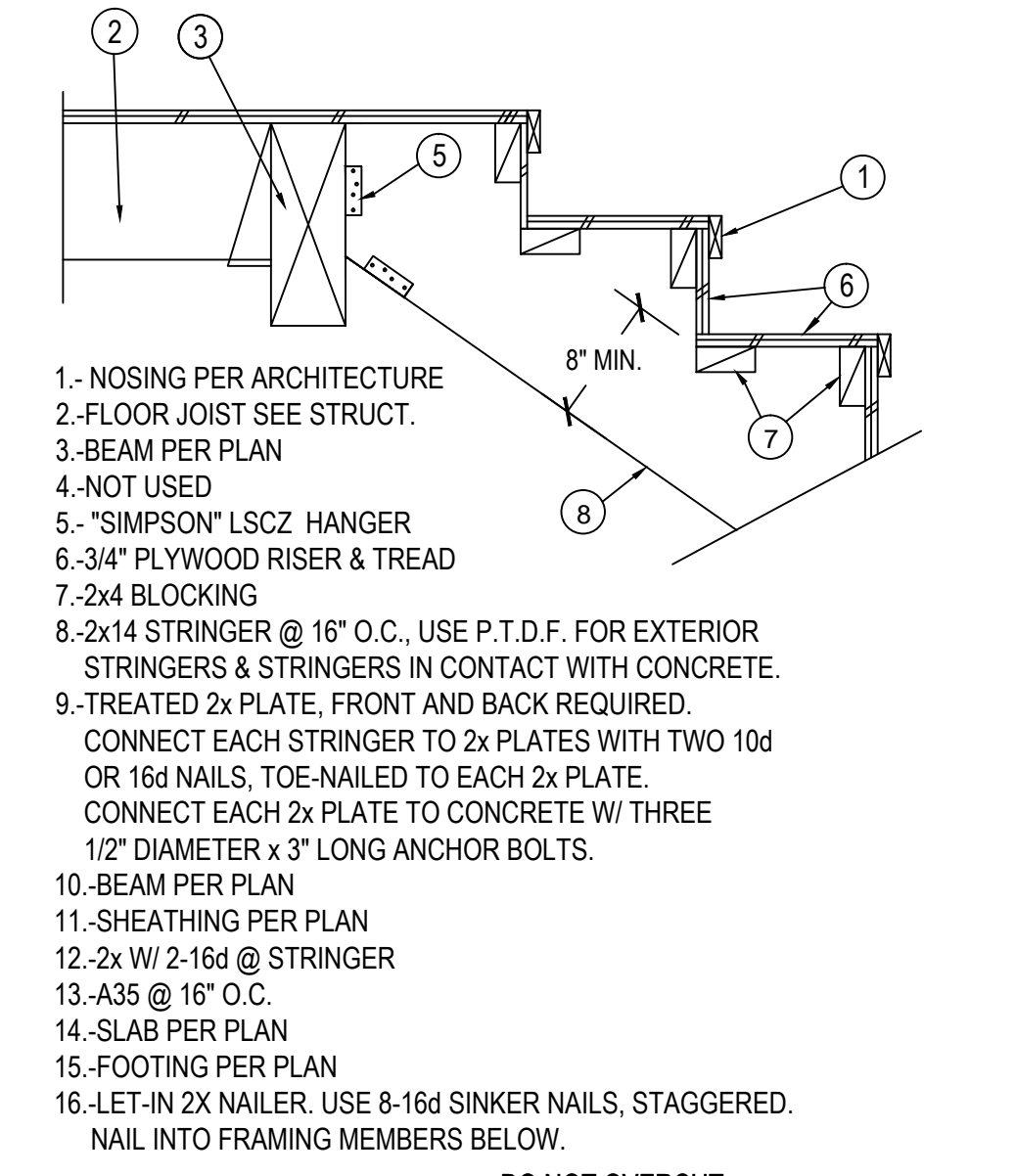
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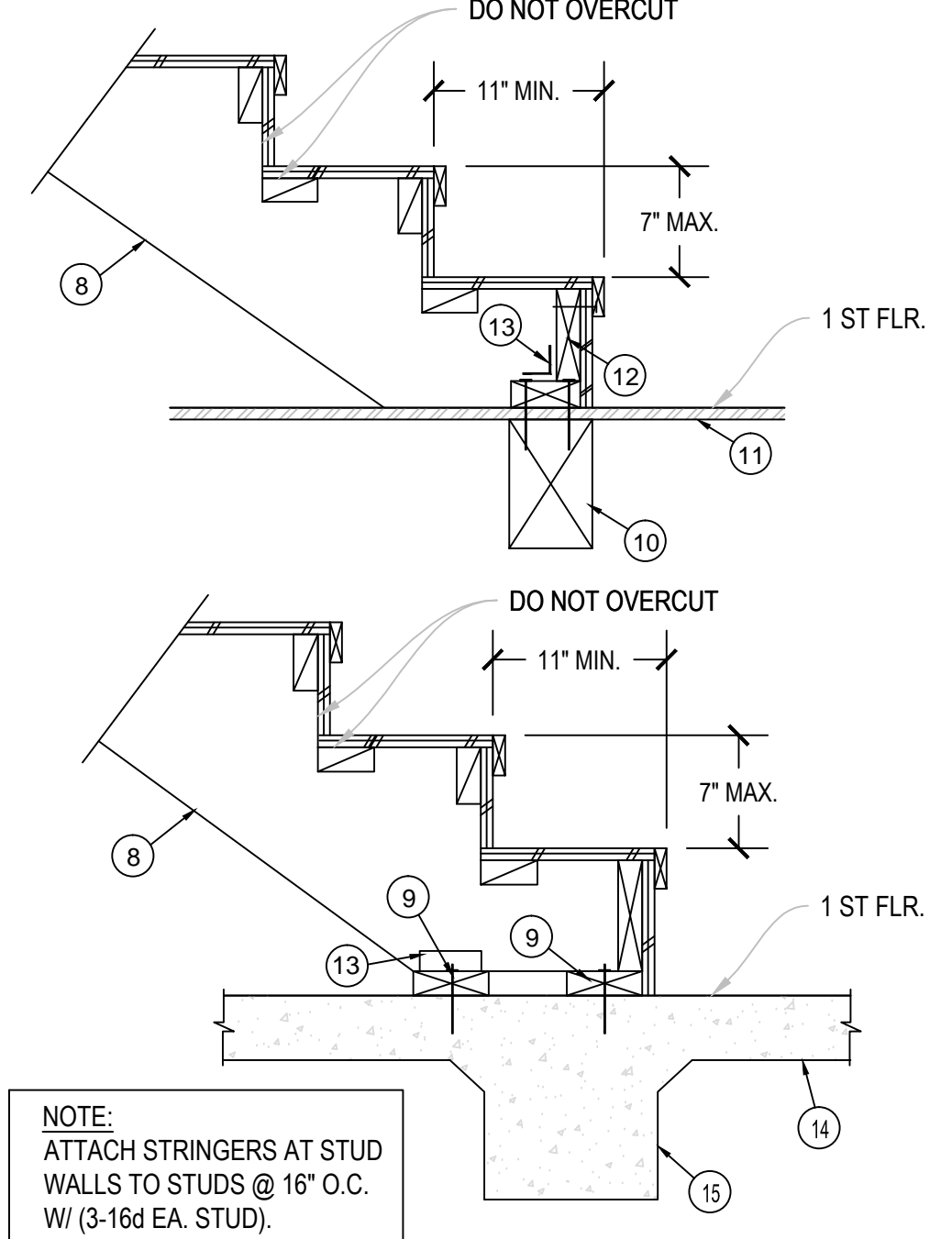
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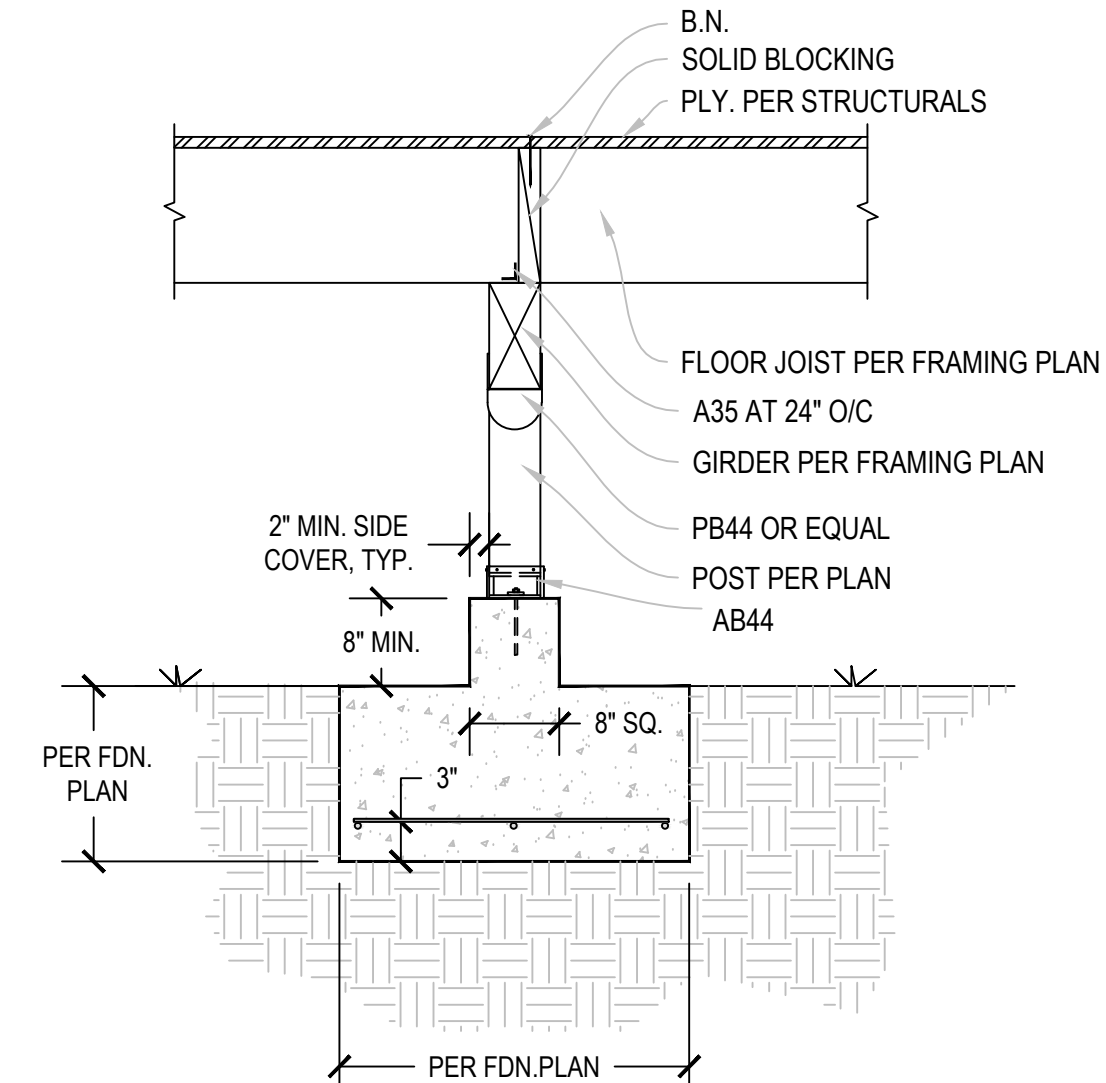
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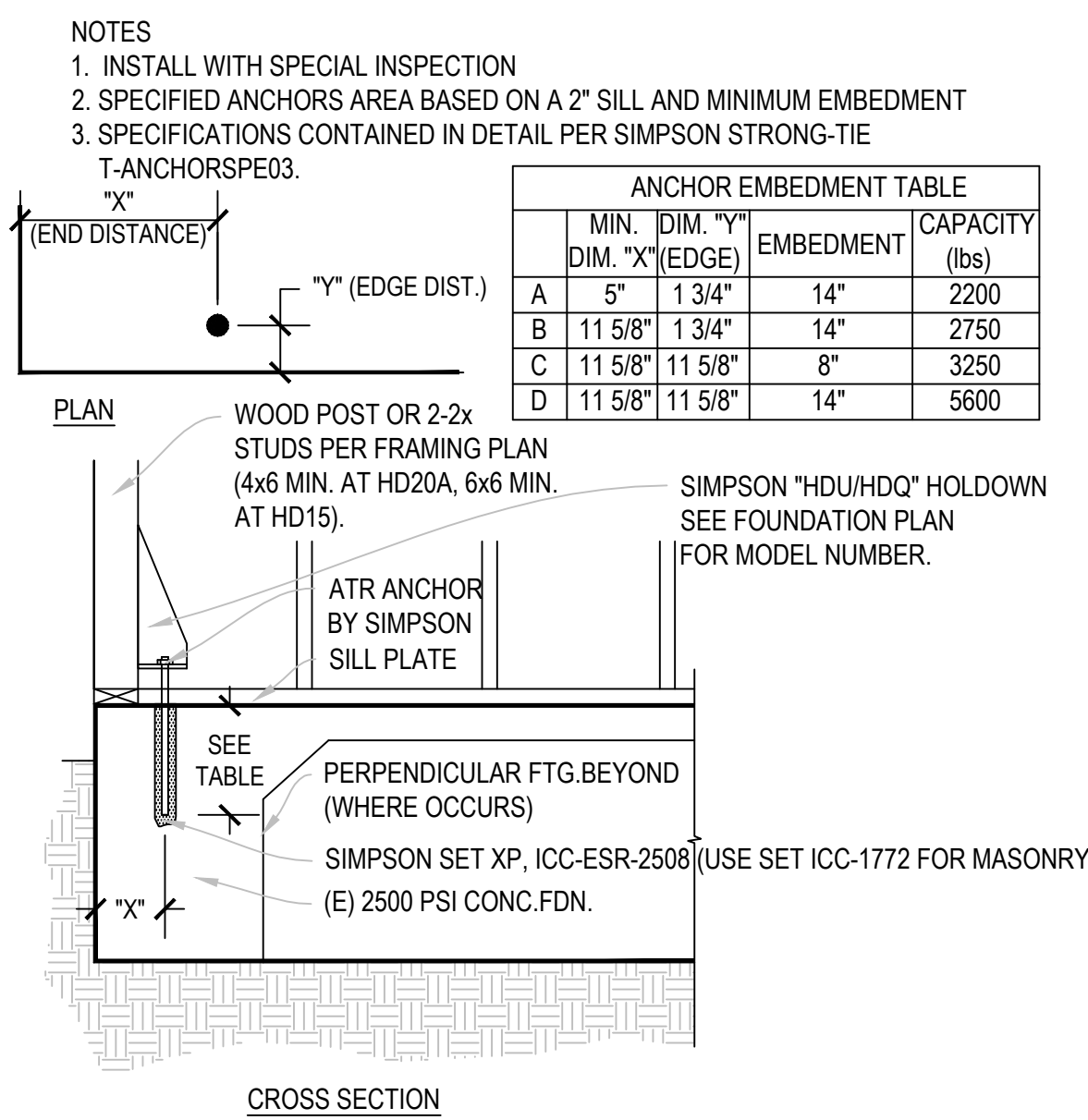
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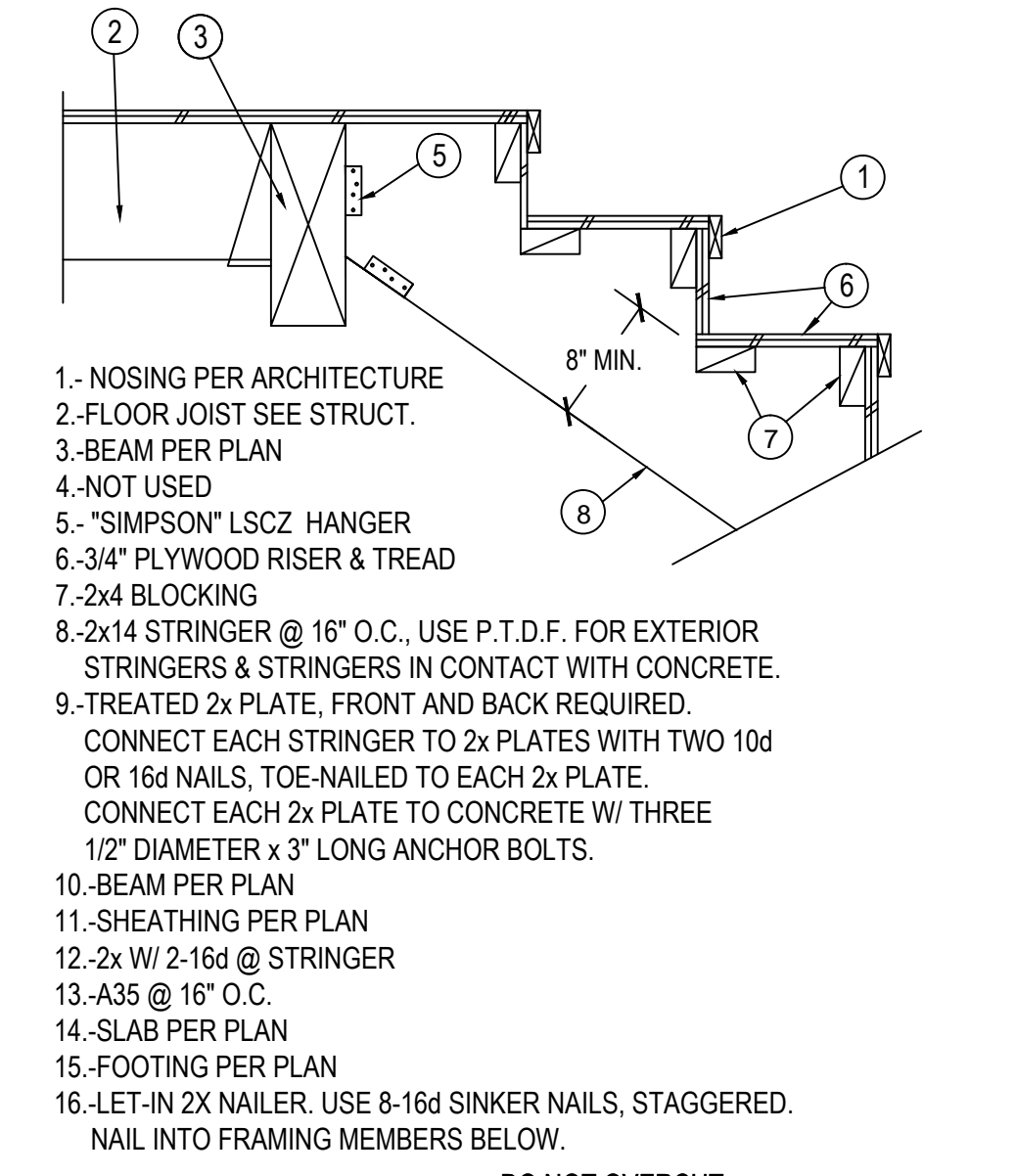
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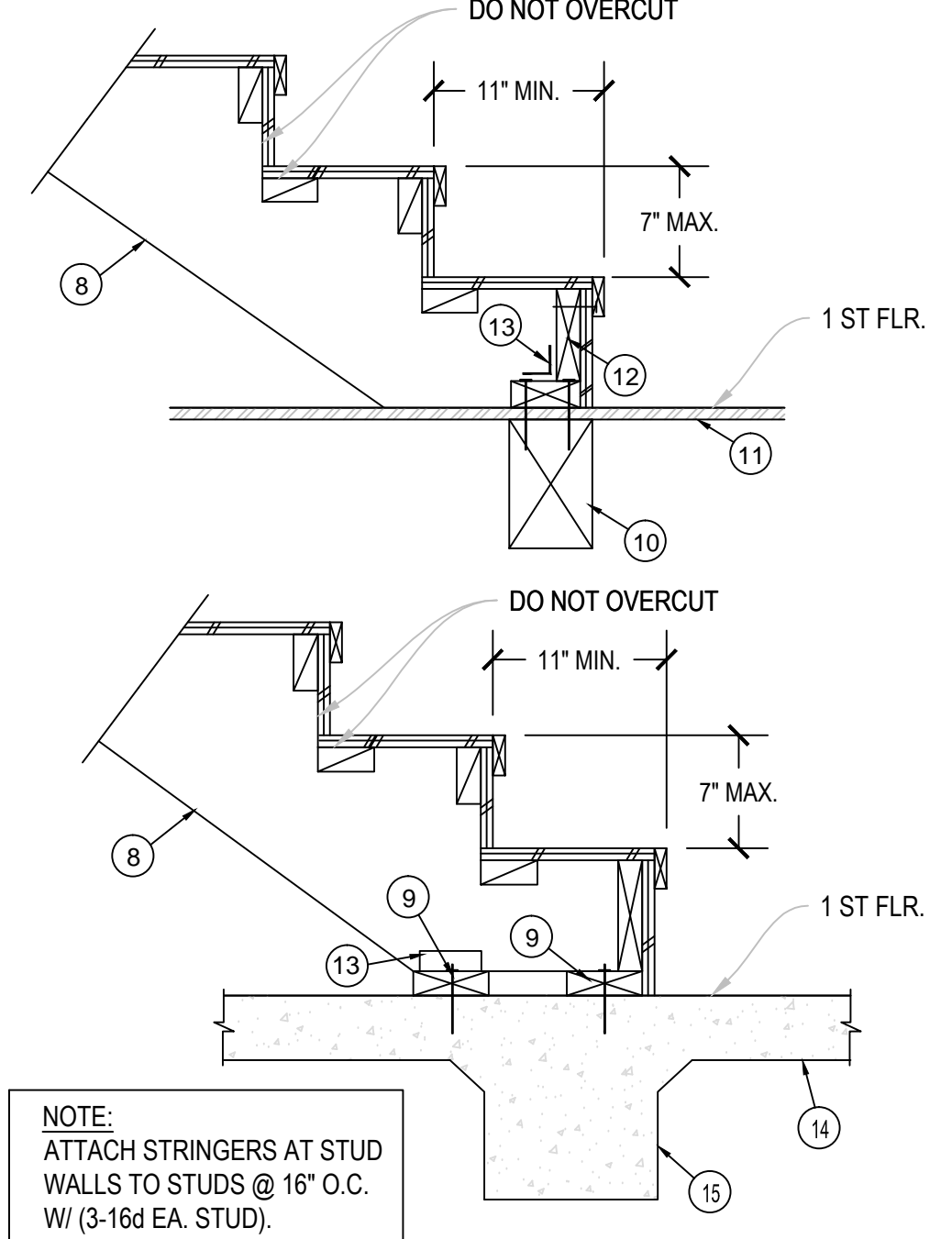
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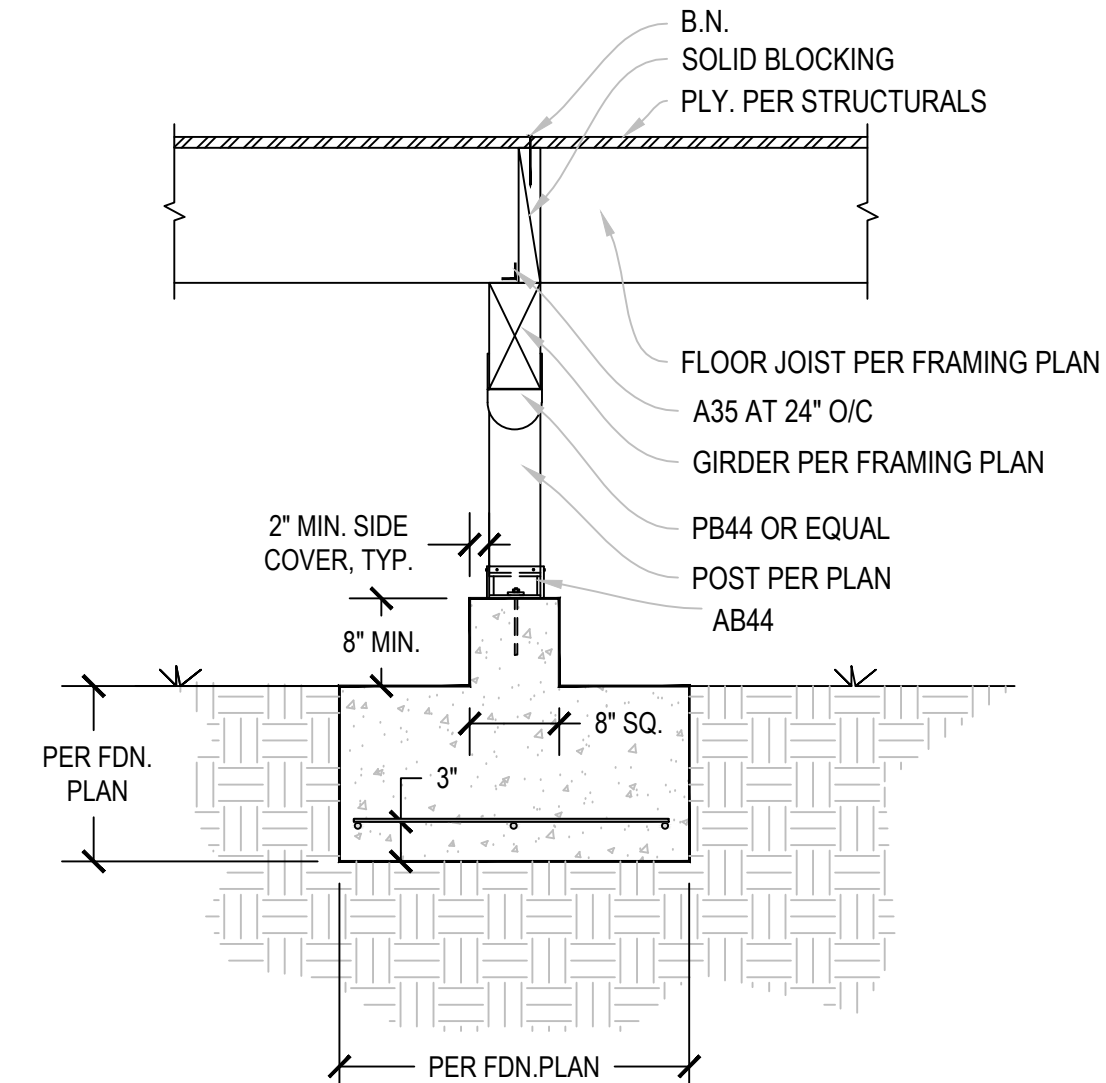
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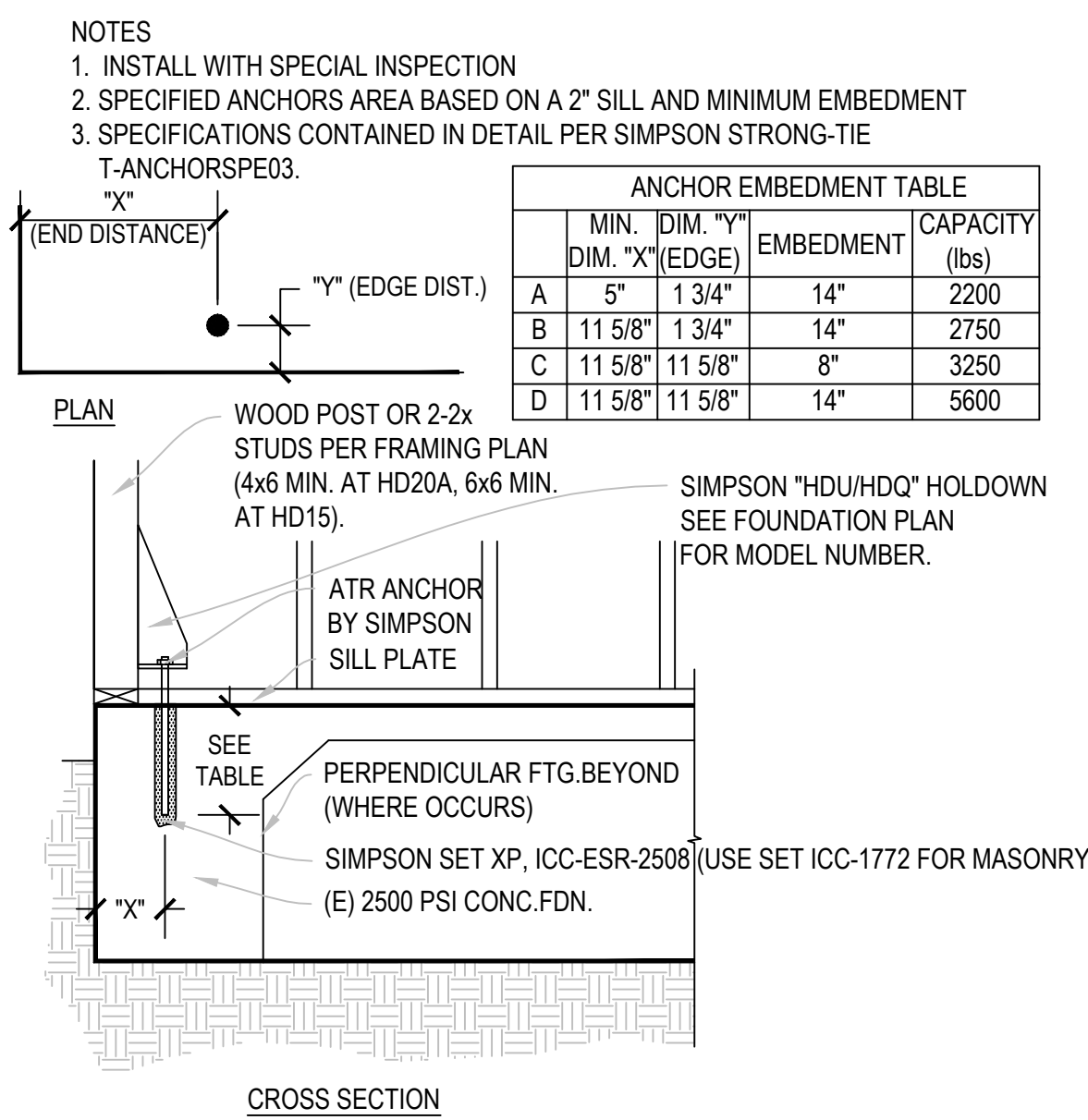
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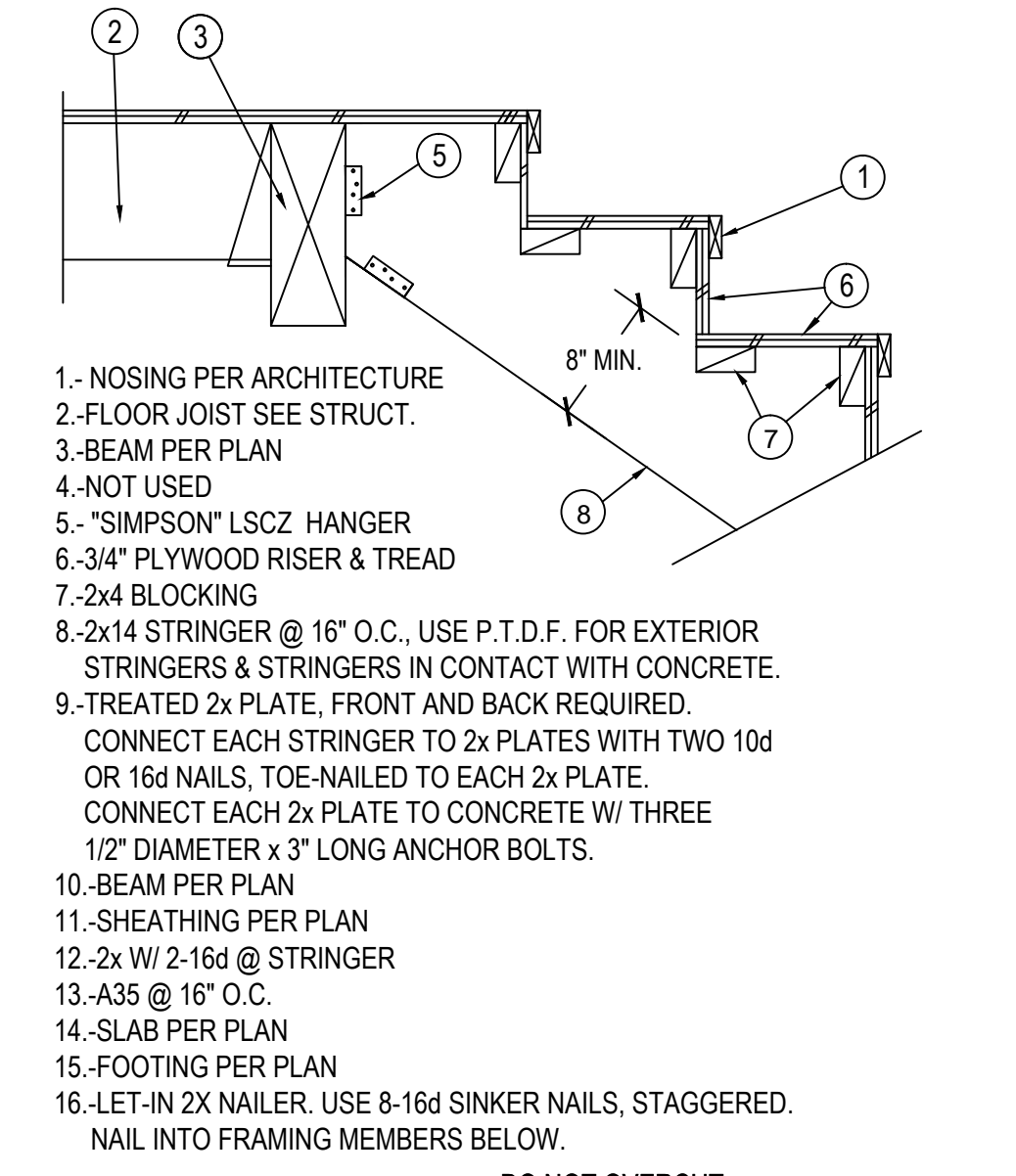
FOOTING INTERIOR PIER

11



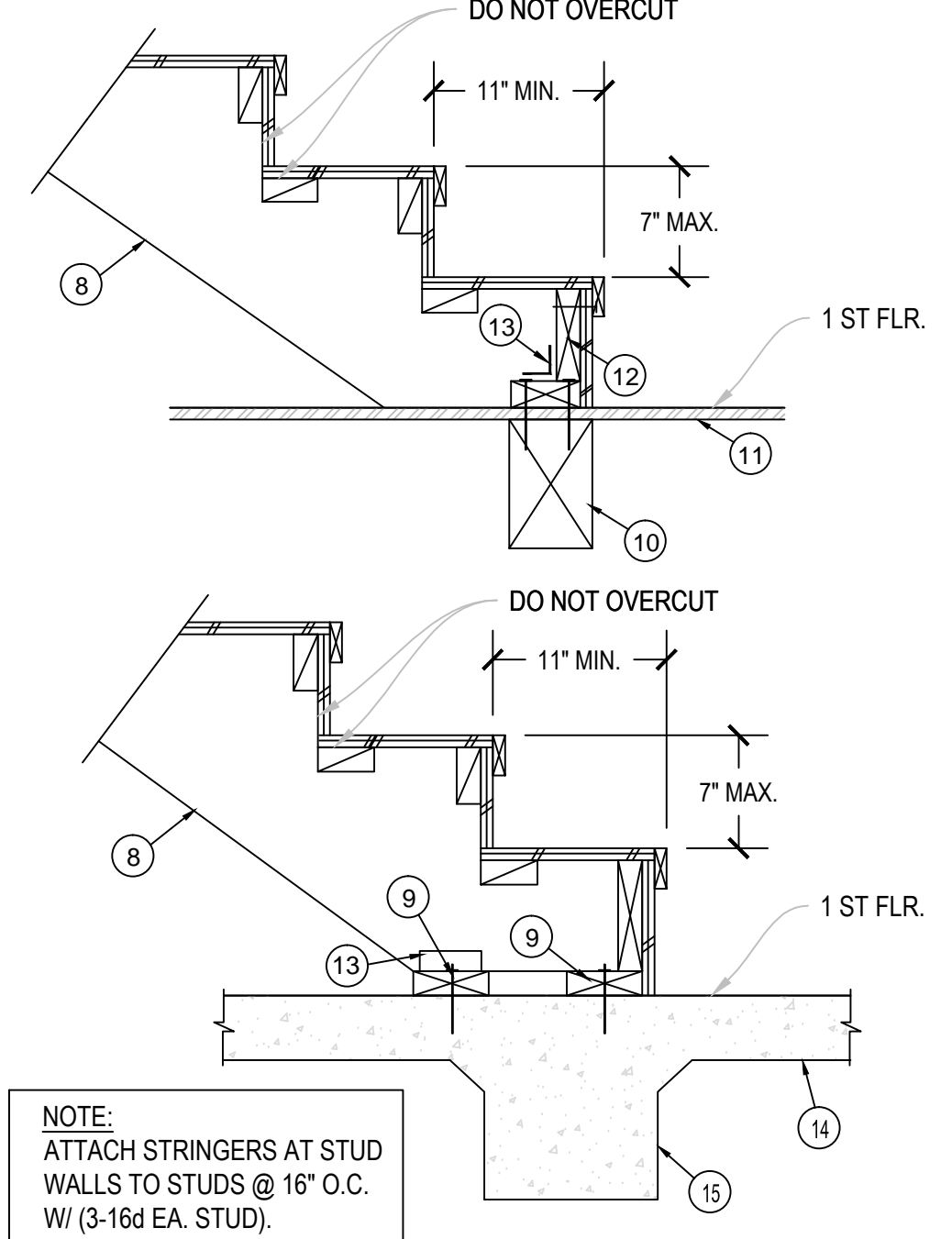
HOLDOWN RETROFIT

12



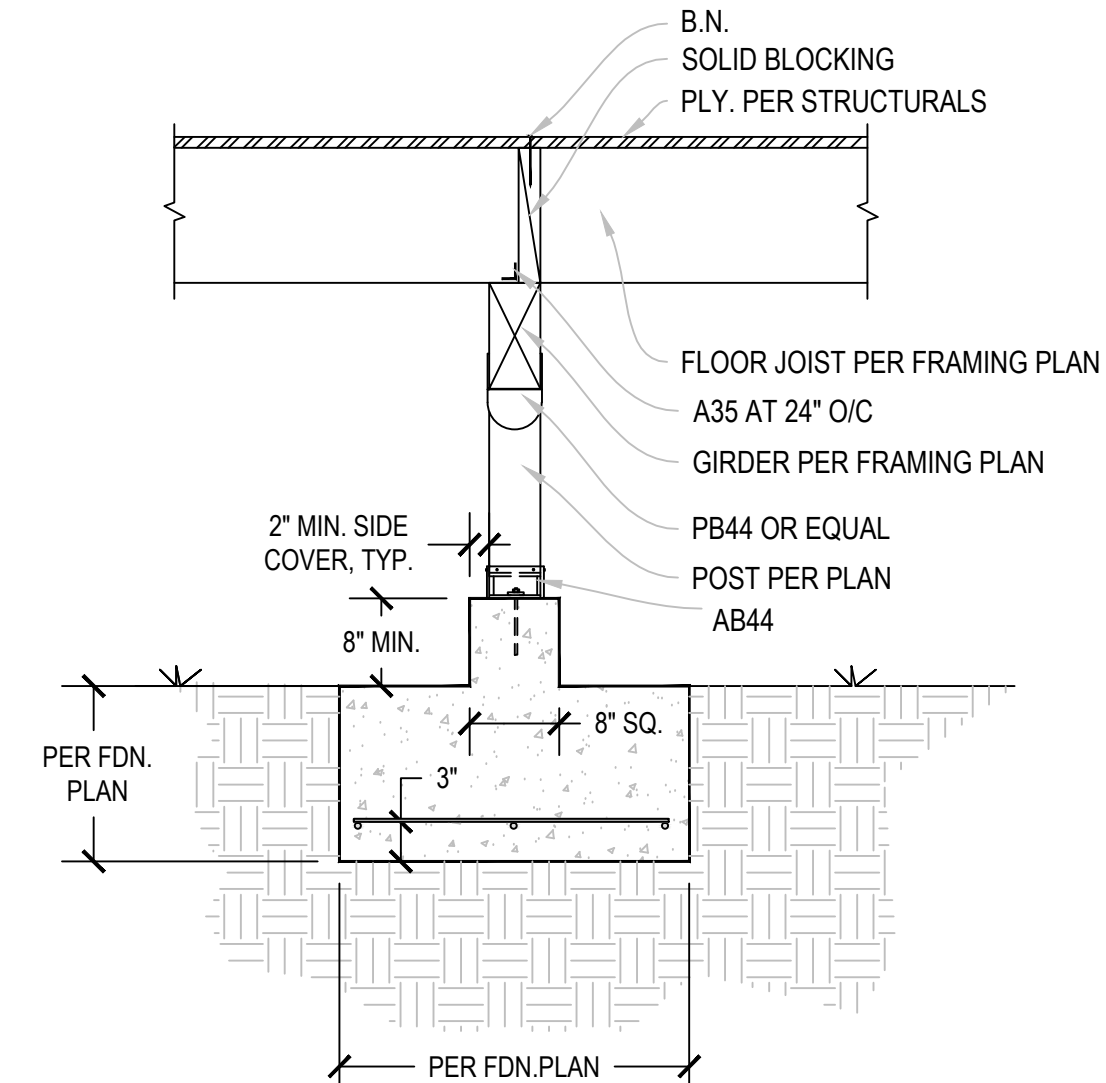
STAIR DETAIL

8



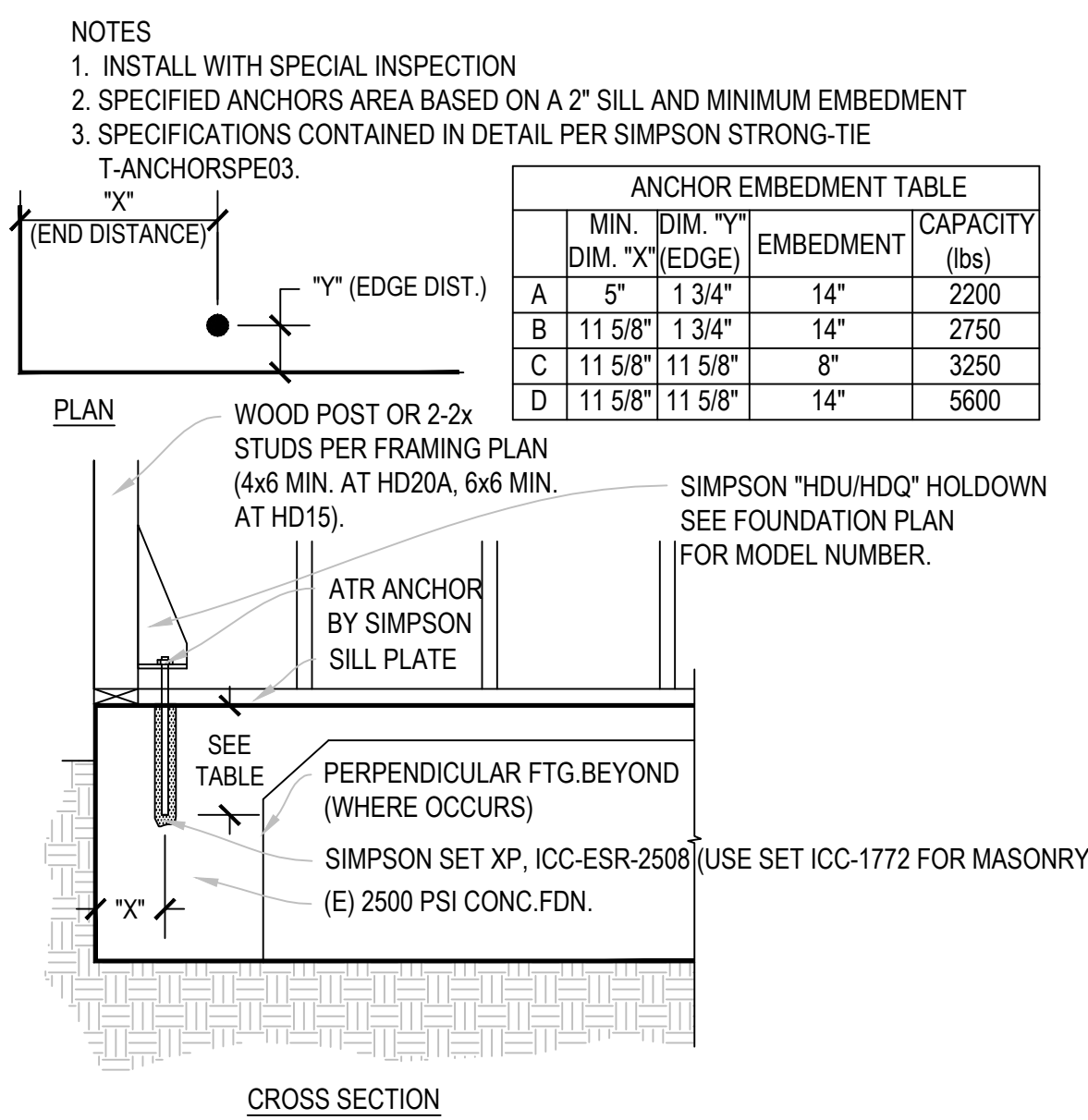
SLAB/RAISED FLOOR TRANSITION

10



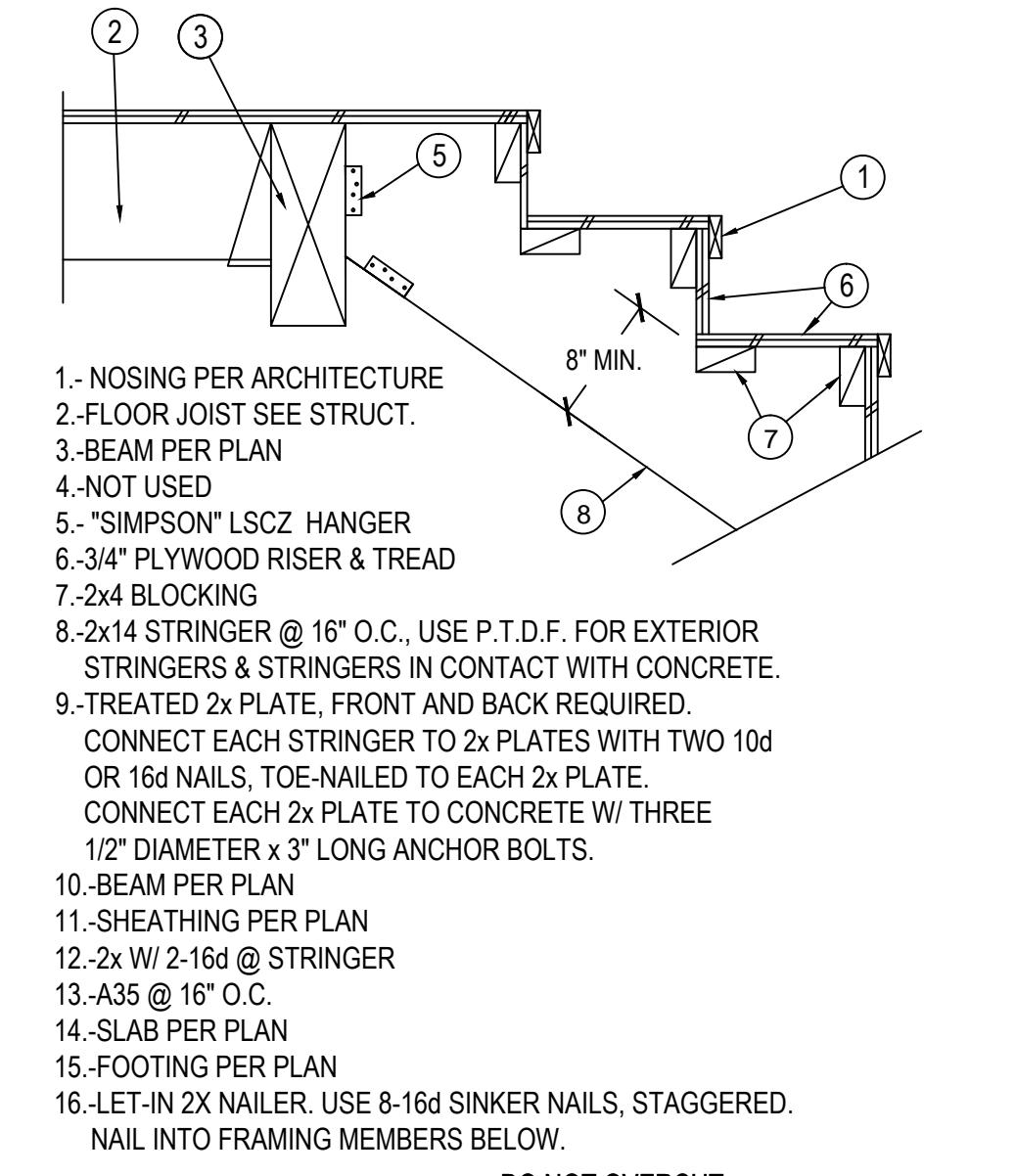
FOOTING INTERIOR PIER

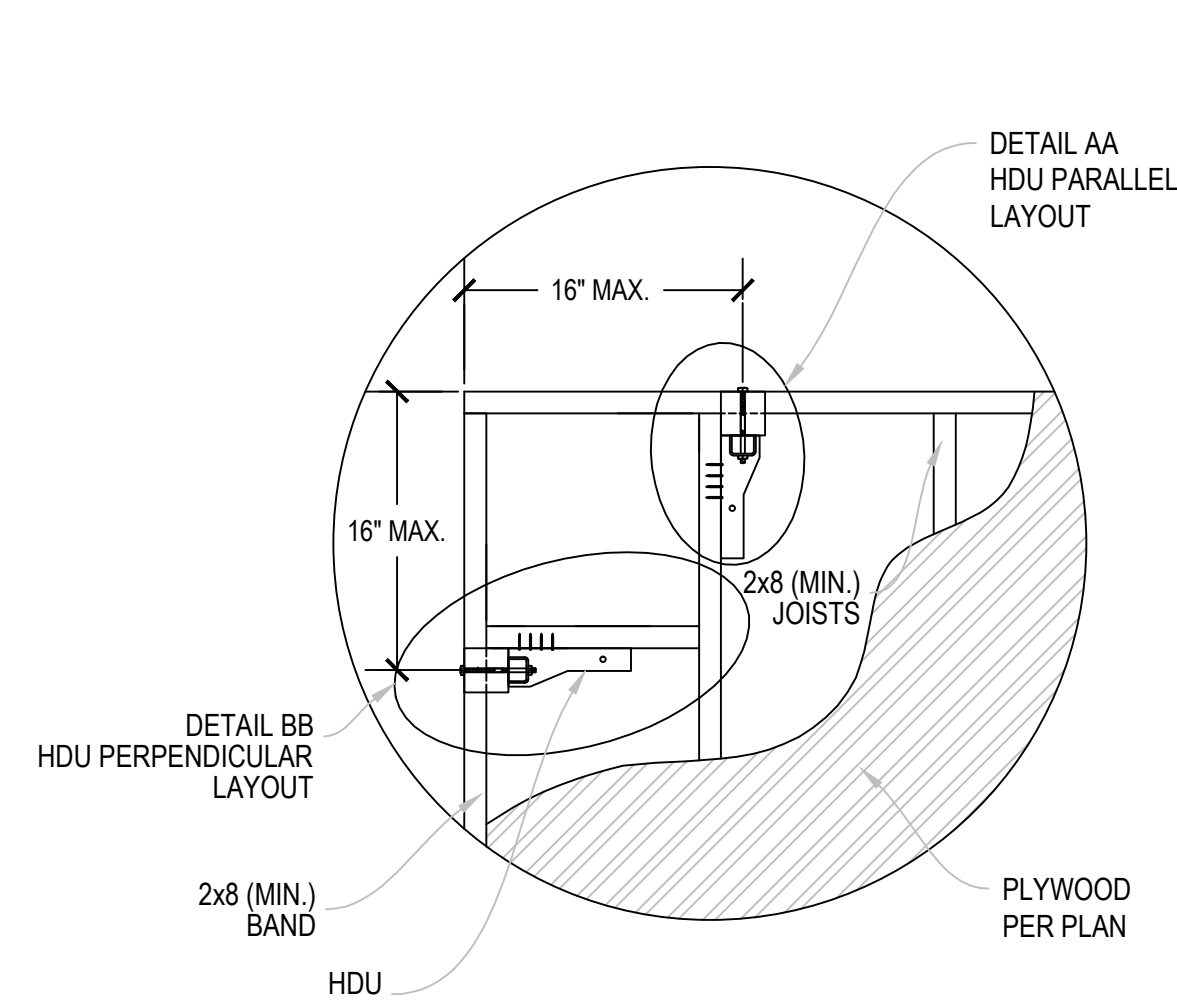
11



HOLDOWN RETROFIT

12

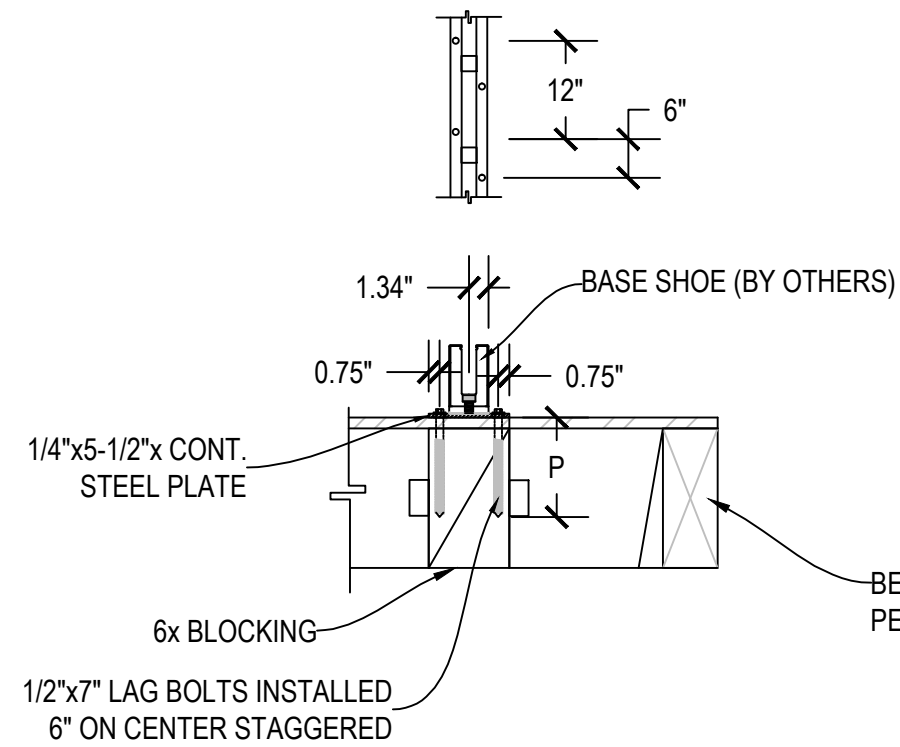




WOOD RAILING OPTION

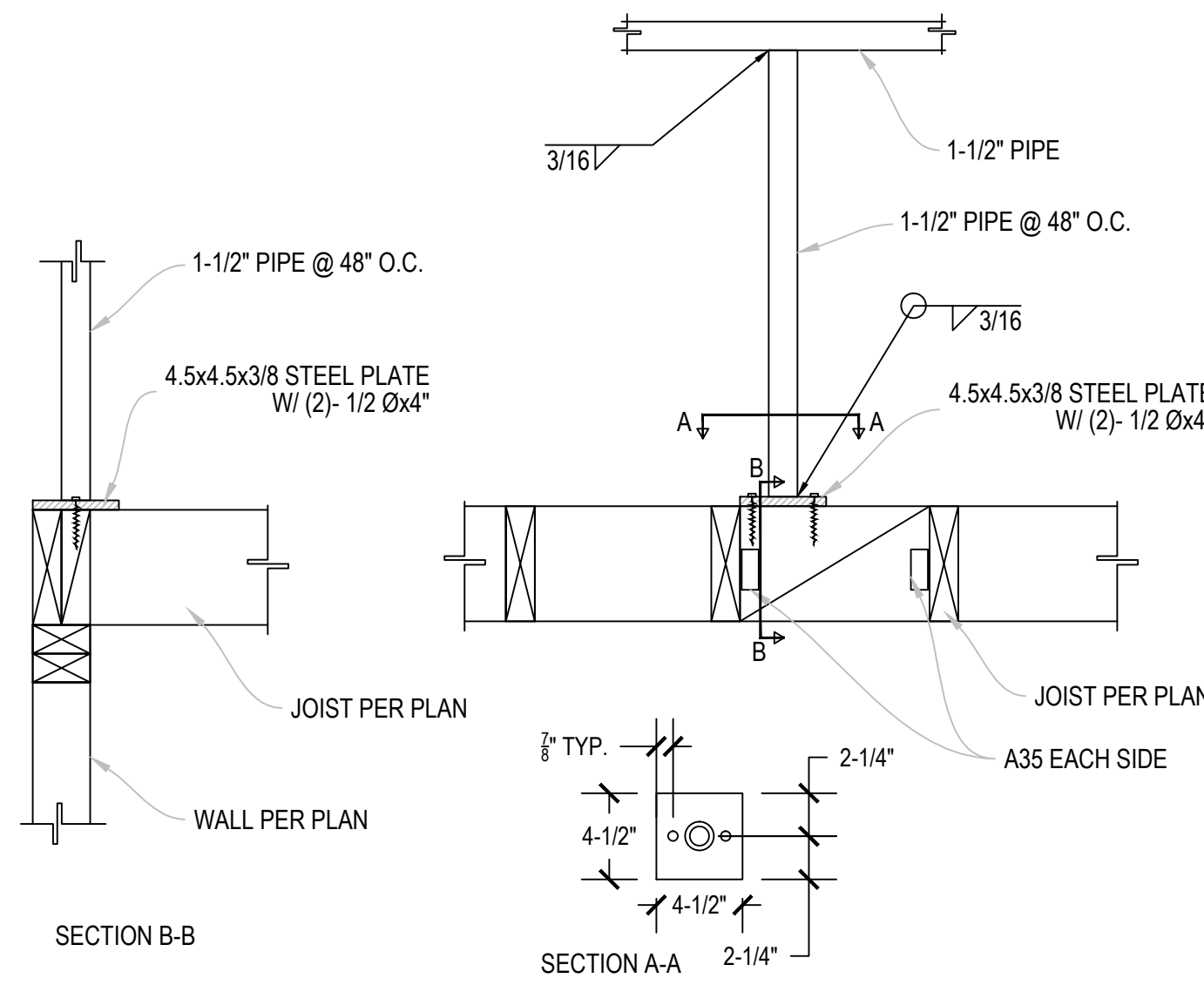
A

NOTE: ANY WELDING OF THE BASE TO BE DONE IN A LICENSED FABRICATOR SHOP APPROVED BY THE CITY OF SAN DIEGO.



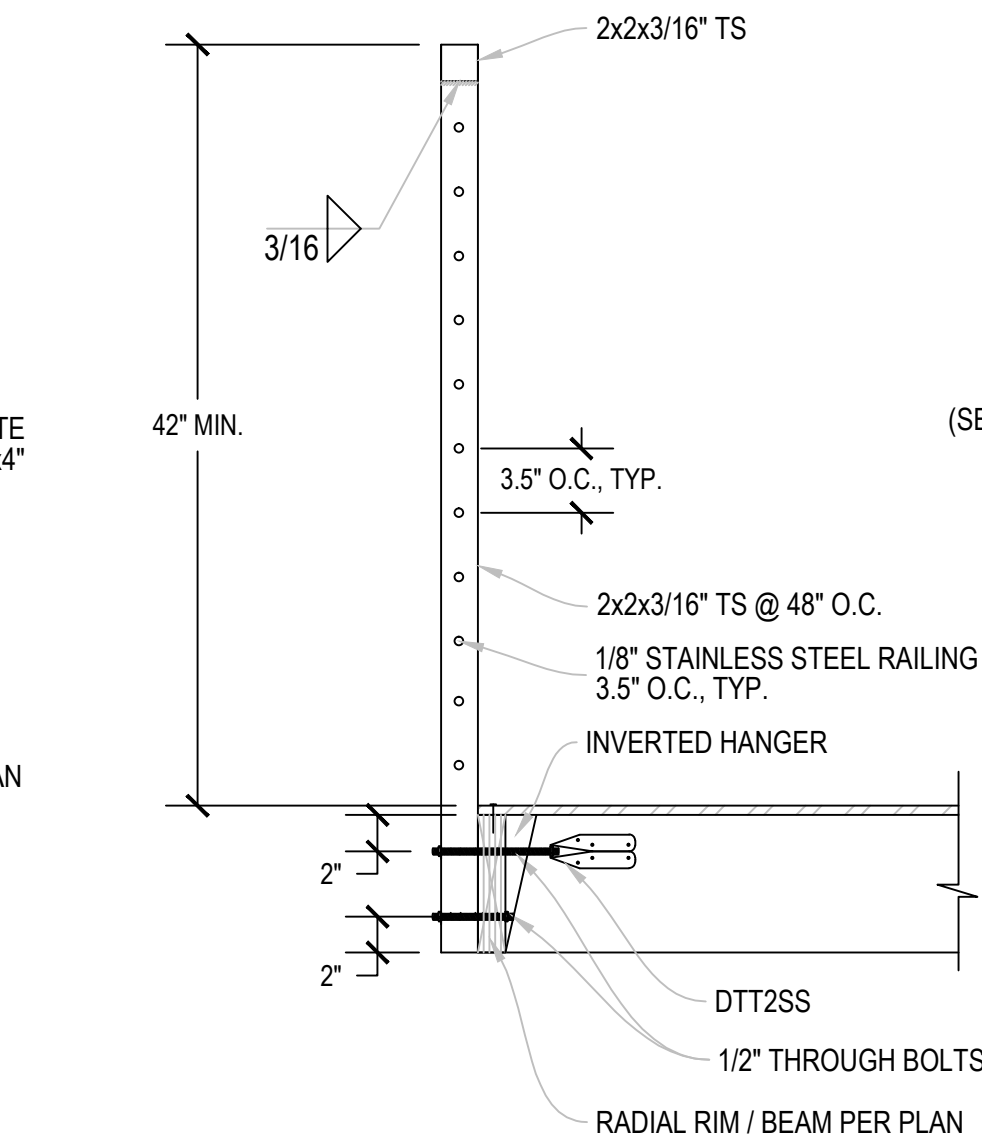
GLASS RAILING OPTION

B



STEEL RAILING OPTION

C

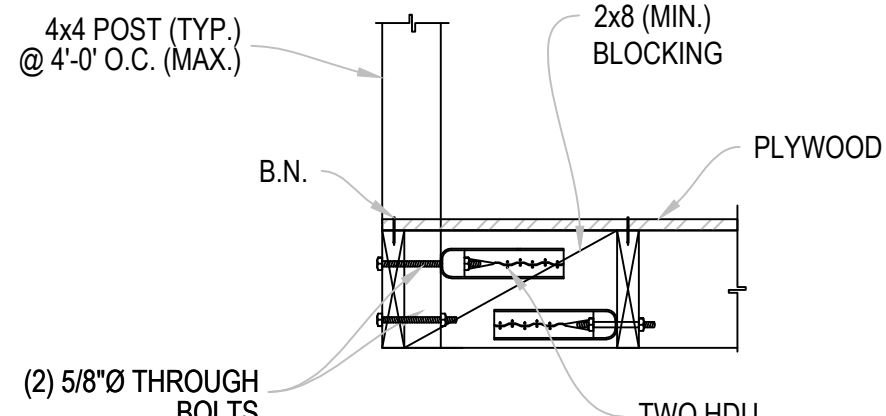


STEEL RAILING OPTION

D

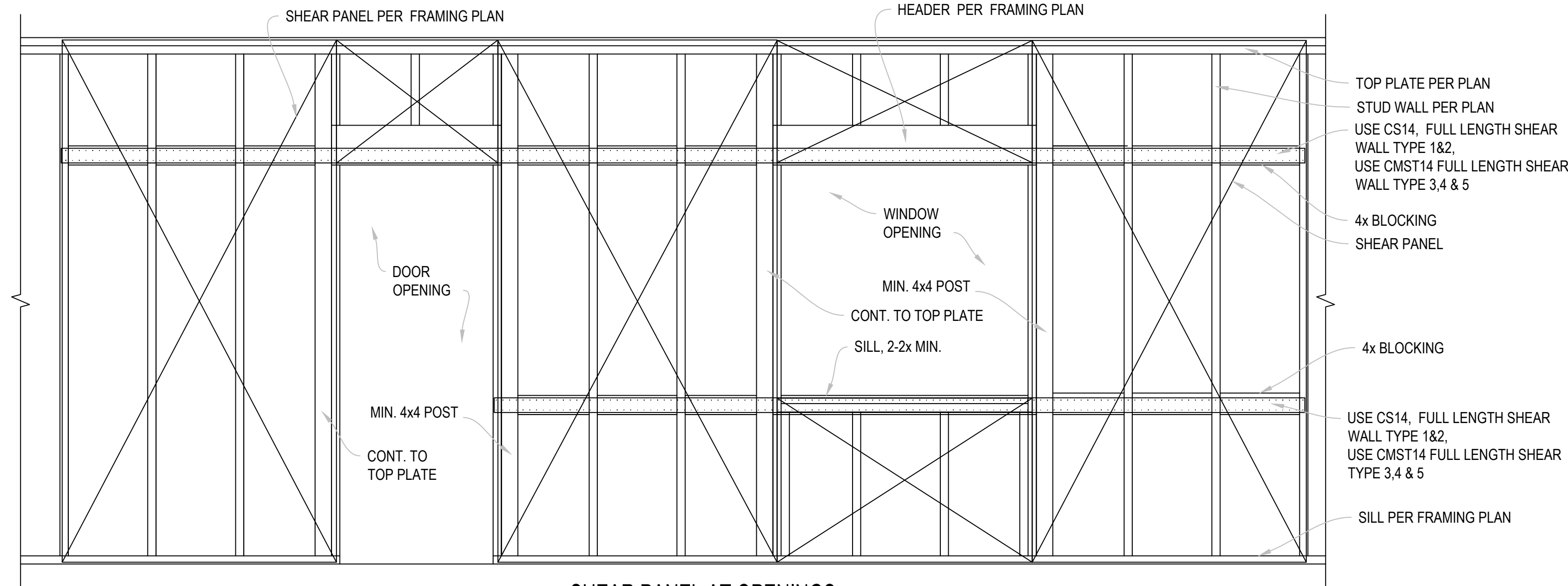
RAILING DETAILS

DETAIL AA



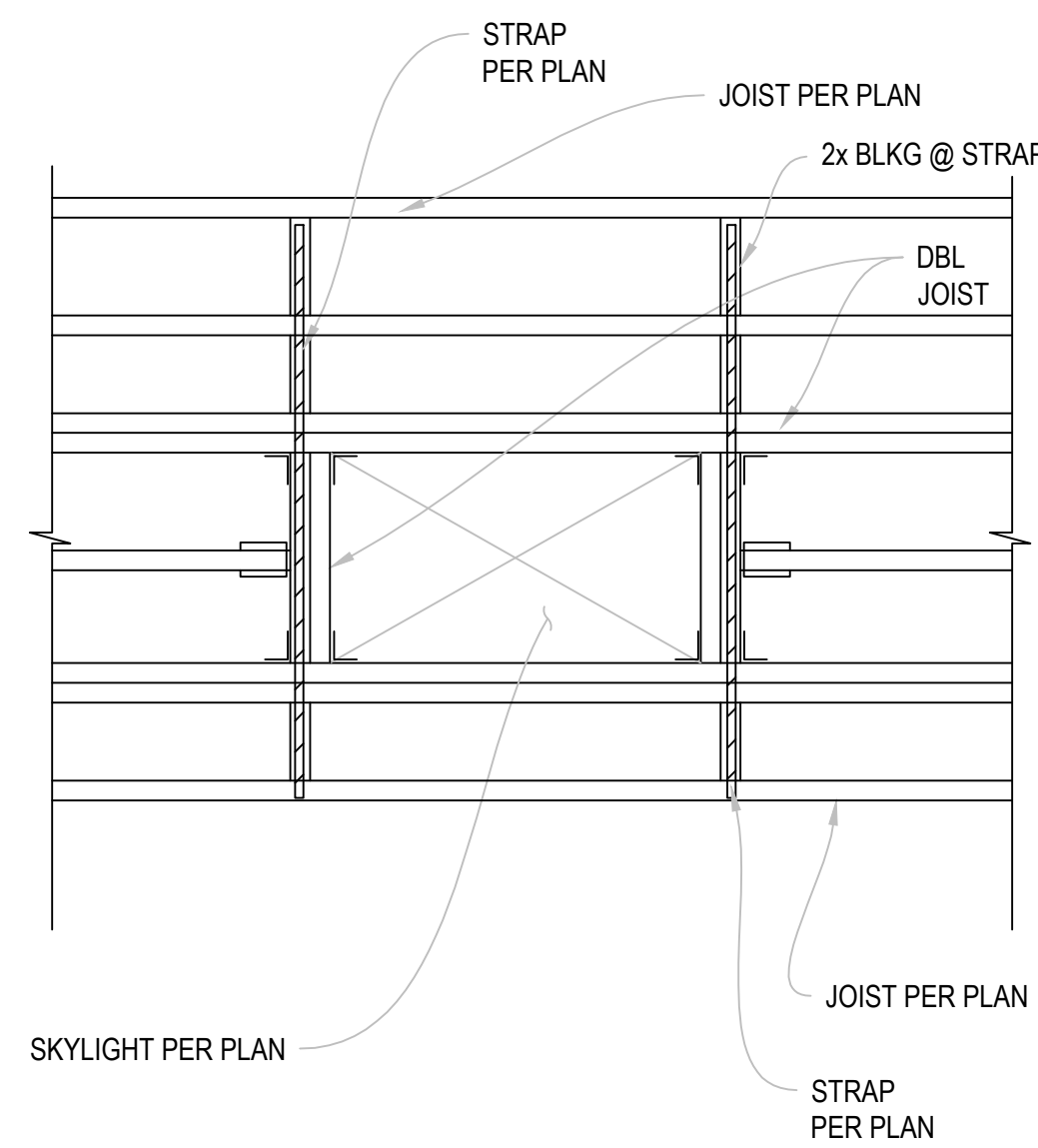
DETAIL BB

DETAIL AA



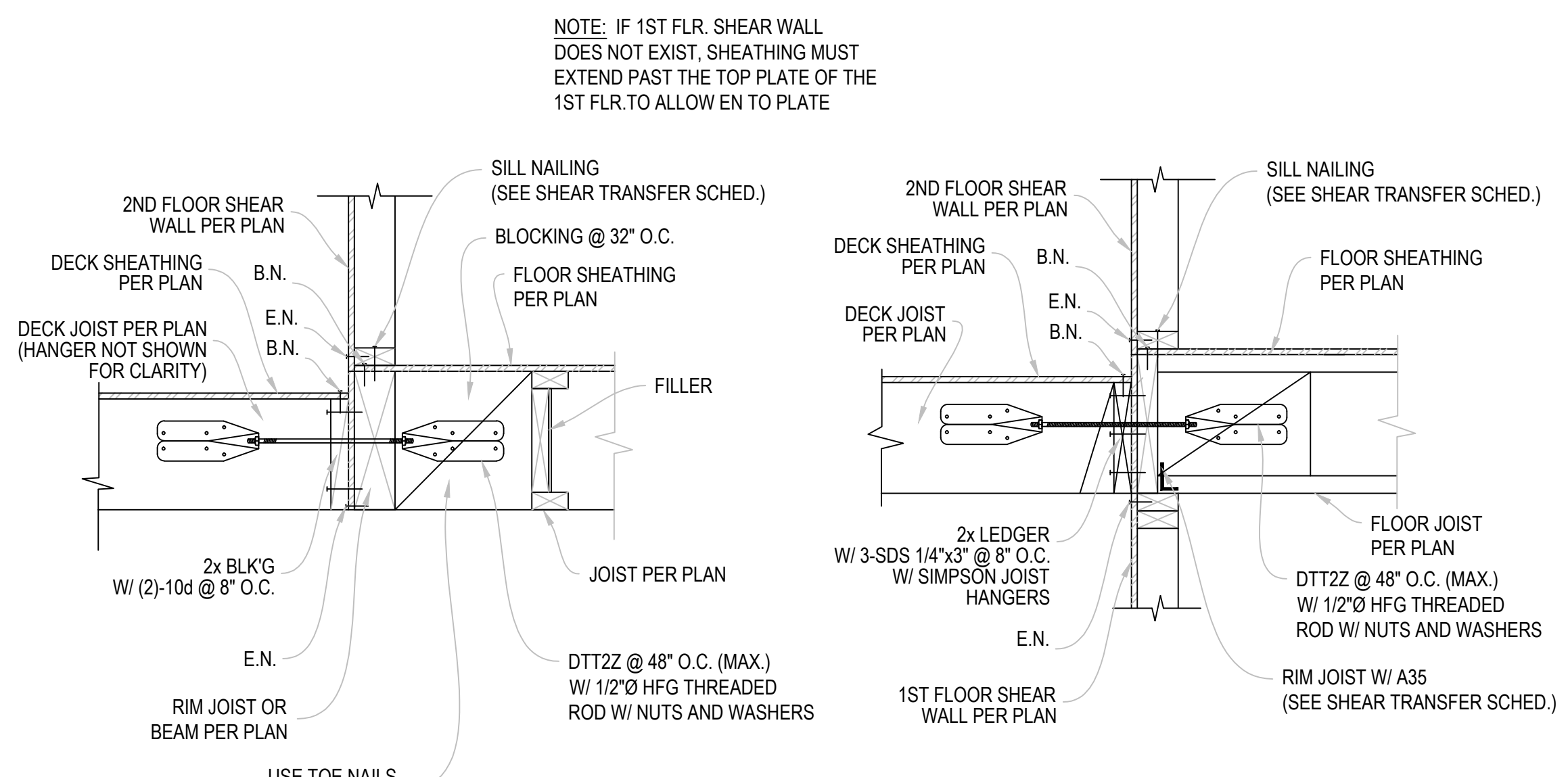
SHEAR PANEL AT OPENINGS

1



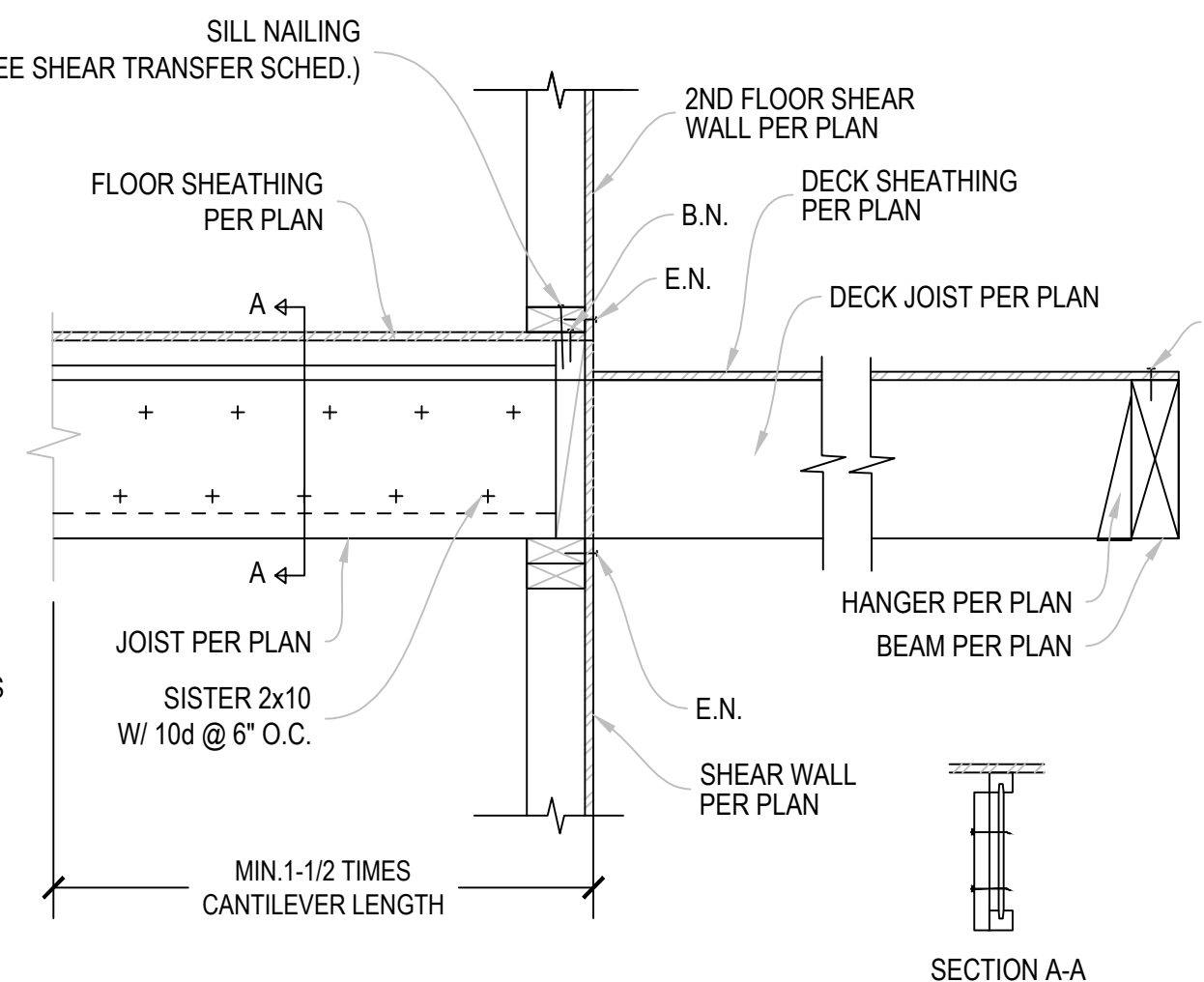
SKYLIGHT DETAIL

8



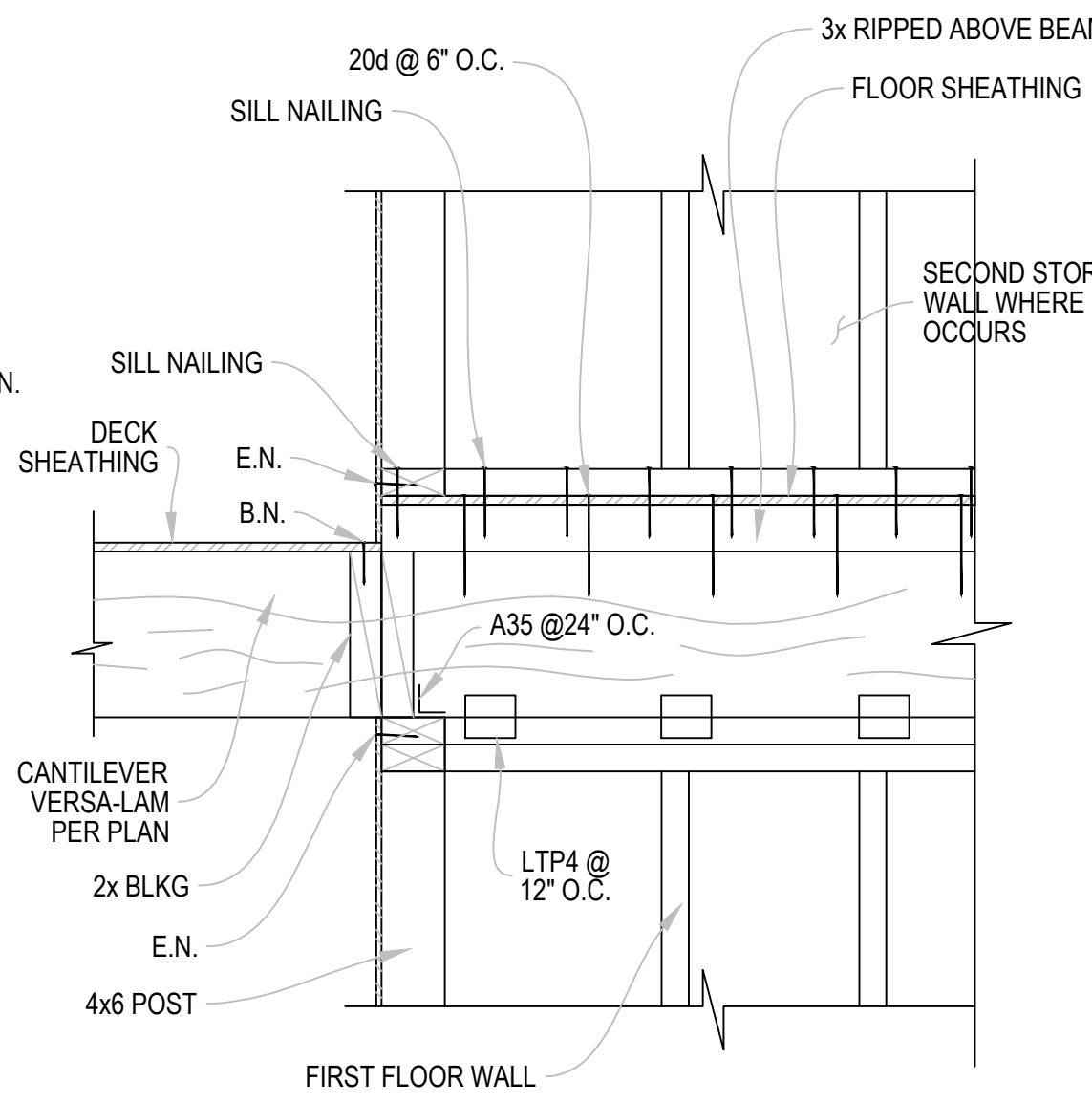
DECK / WALL CONNECTIONS

2



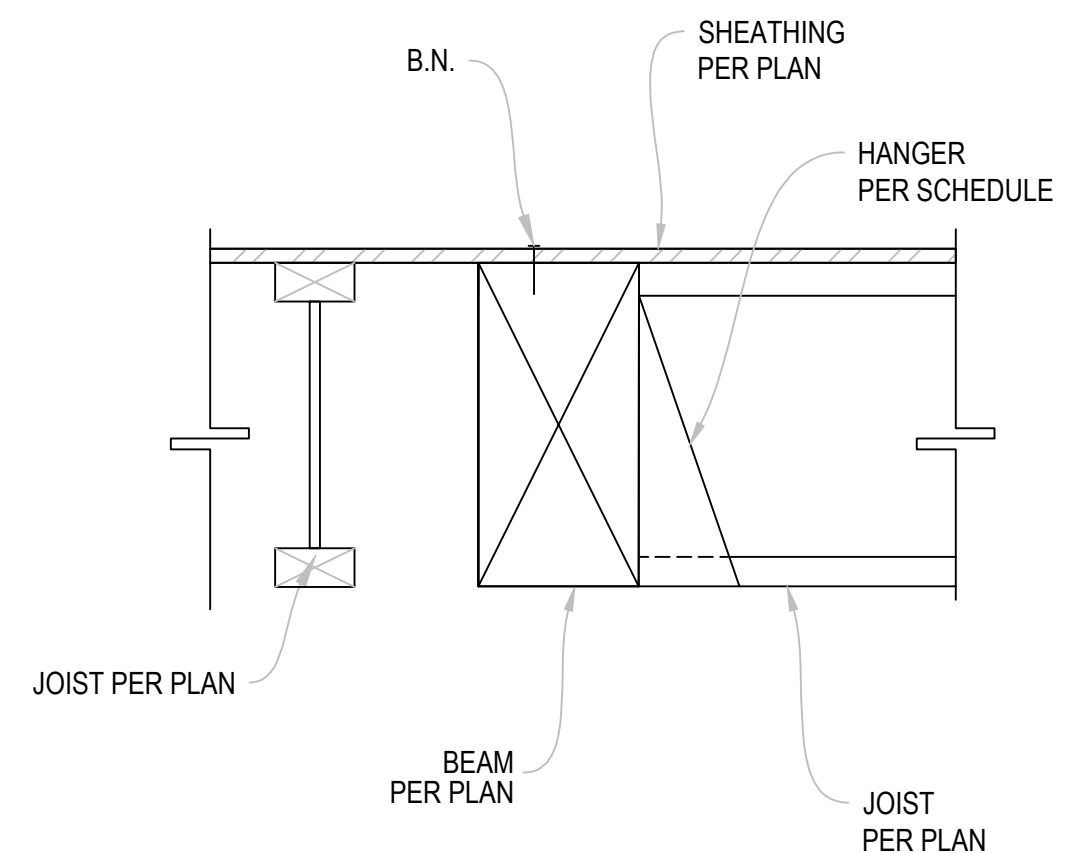
DECK / WALL CONNECTIONS

9



FRAMING DETAIL

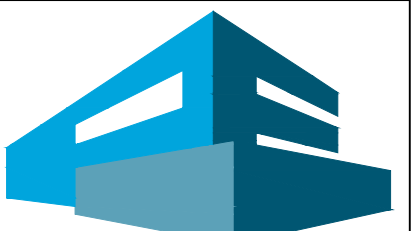
6



JOIST TO BEAM CONNECTIONS

3

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MEBUST RESIDENCE

5448 TAFT AVENUE
LA JOLLA, CA 92037
PROJECT #: 2013-0070

DESIGNER:

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REV	REVISION	DATE

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CONTENT

FRAMING DETAILS

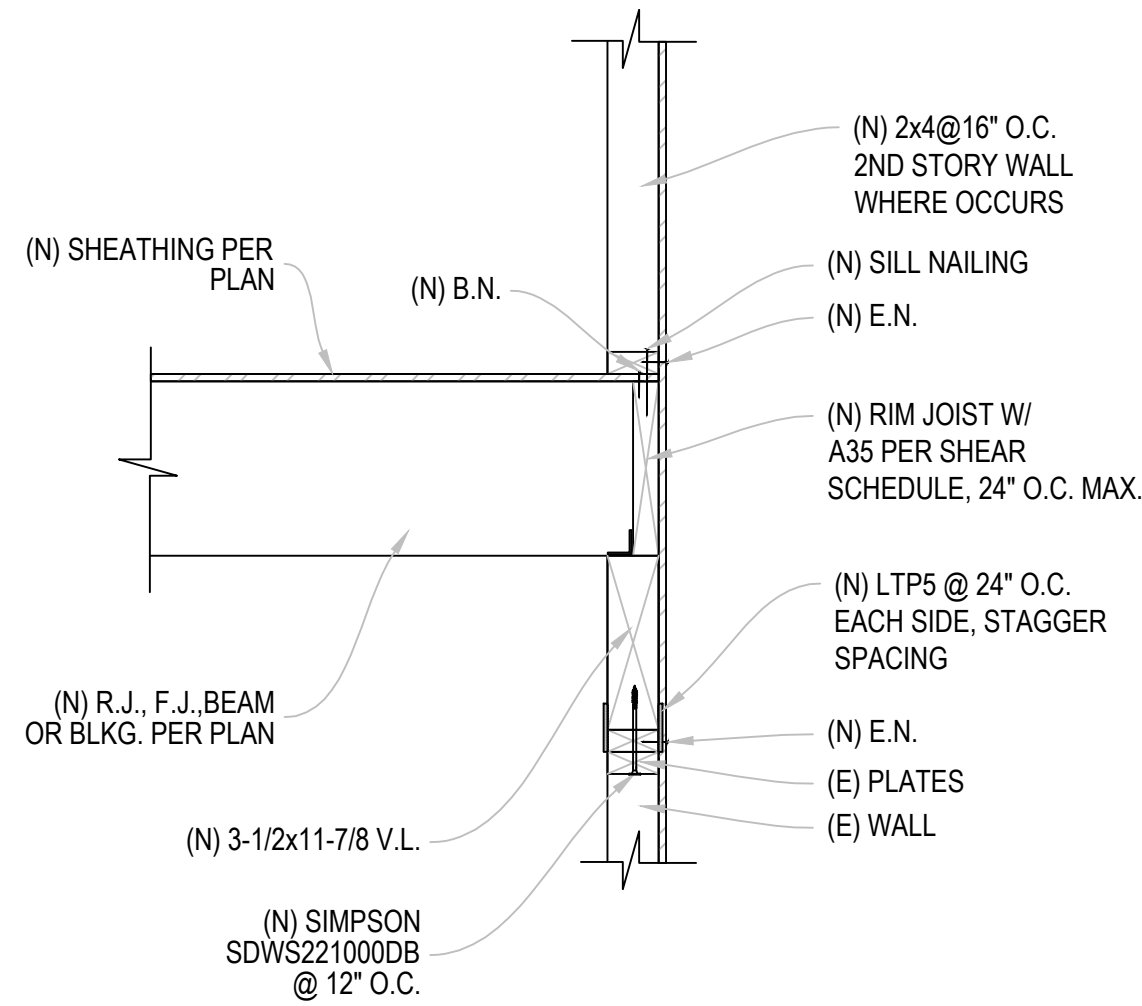
DRAWN BY: CR/CKP

CHECKED BY: CKP

DATE: SEPTEMBER 20, 2013

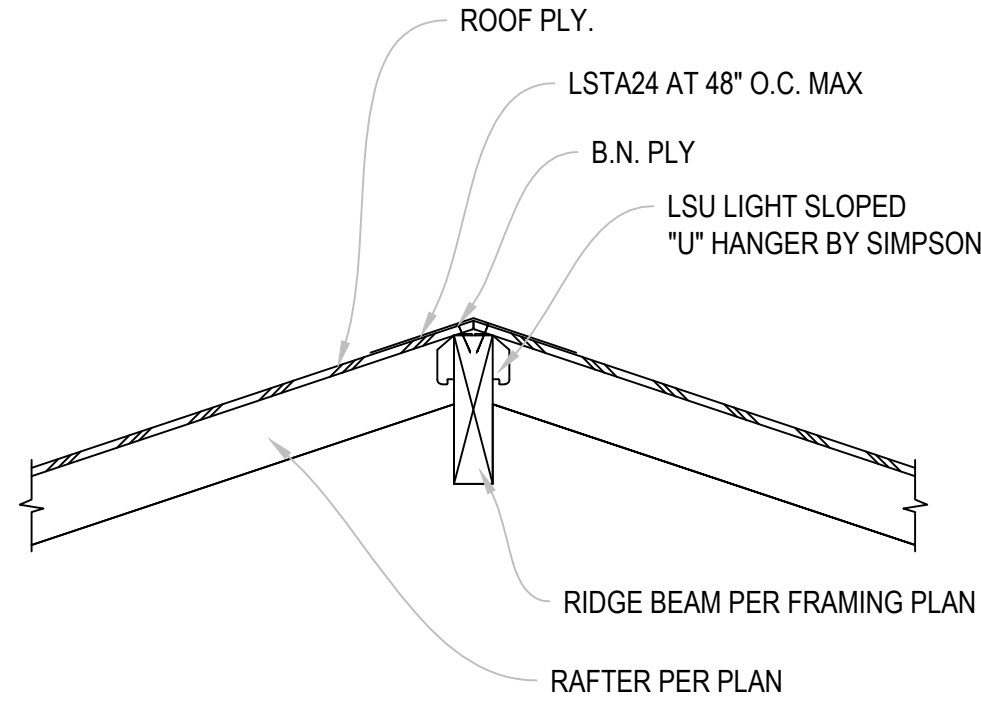
SHEET NO.

S4.1



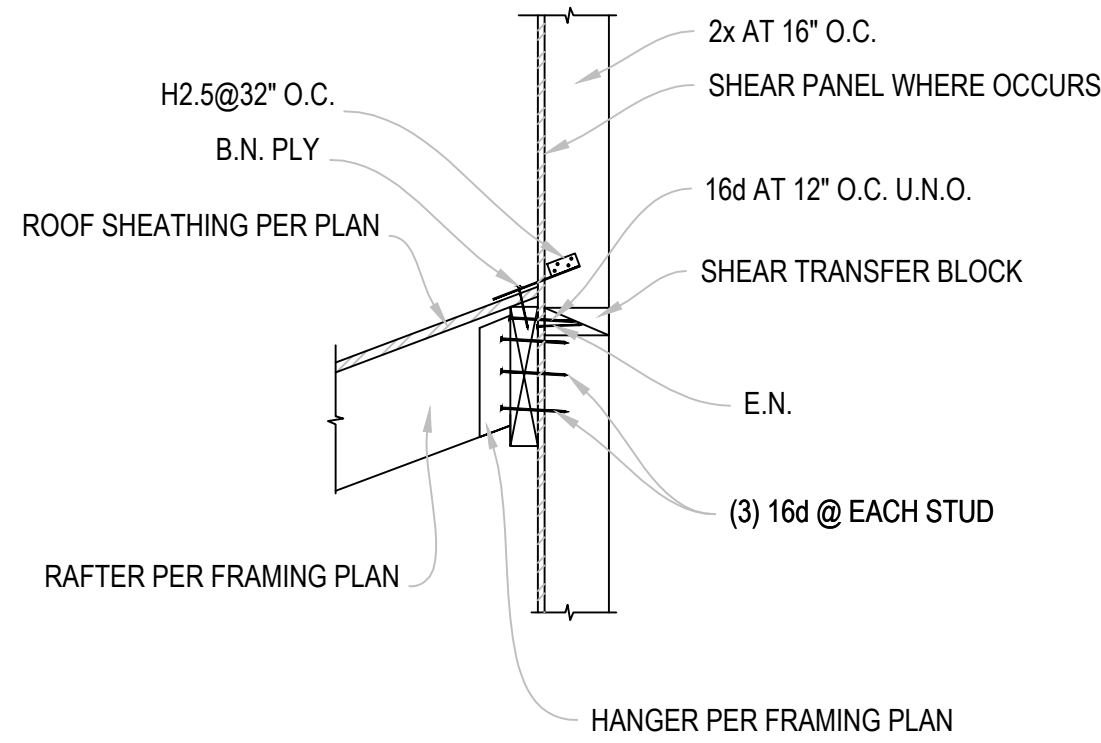
TYPICAL RAISED PLATE DETAIL

13



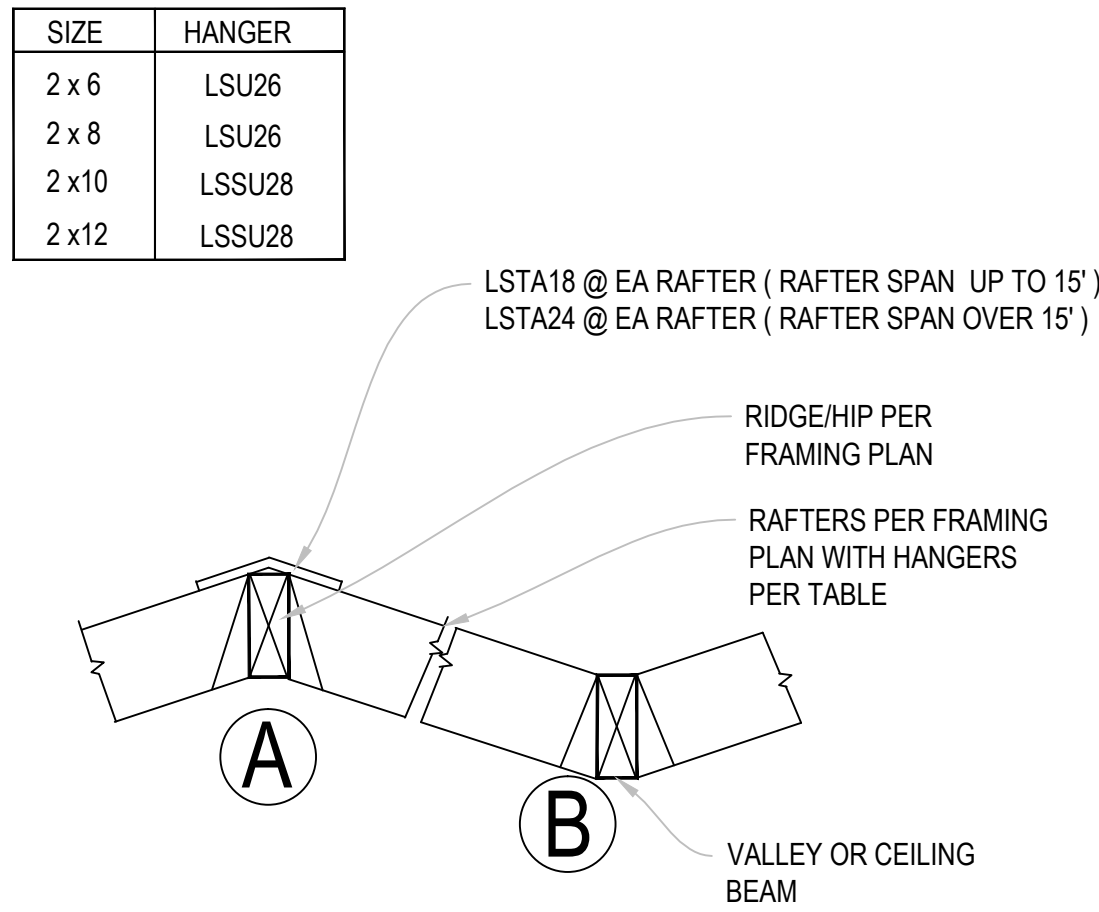
RIDGE BEAM TO RAFTER CONNECTION

10



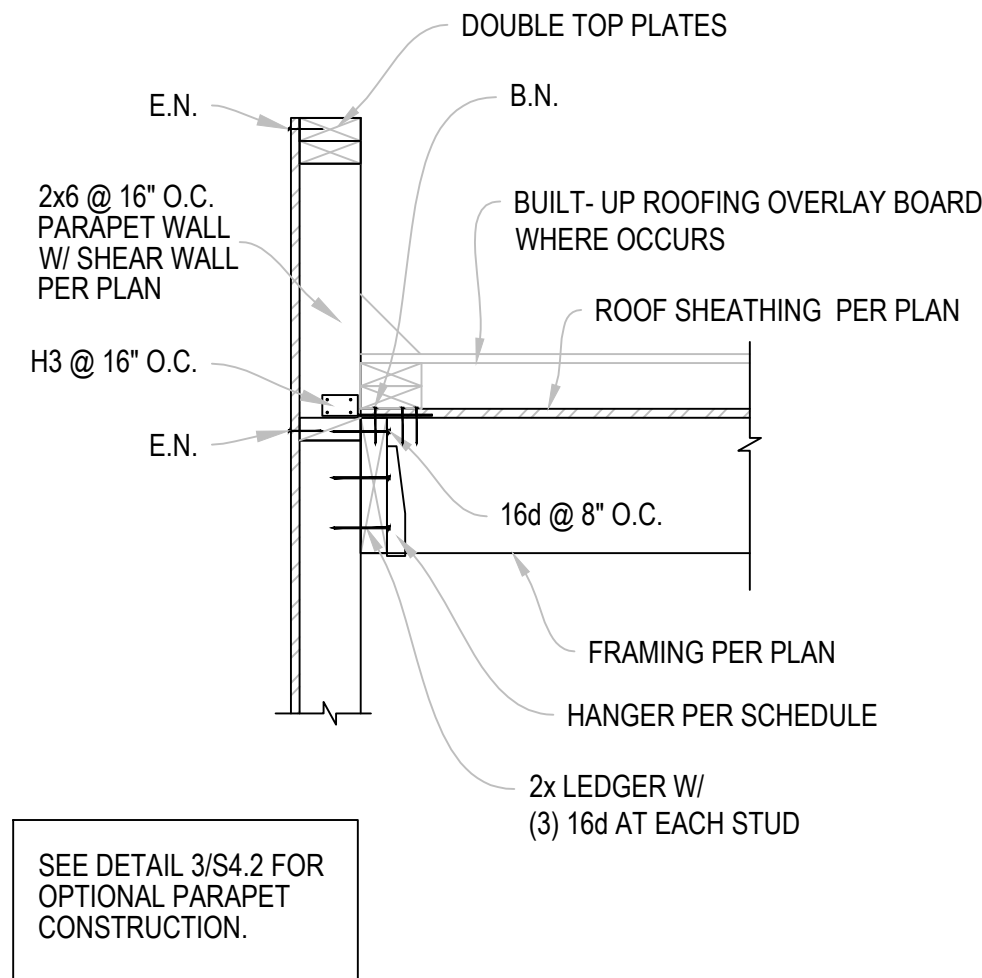
ROOF SHEAR TRANSFER DETAIL

7



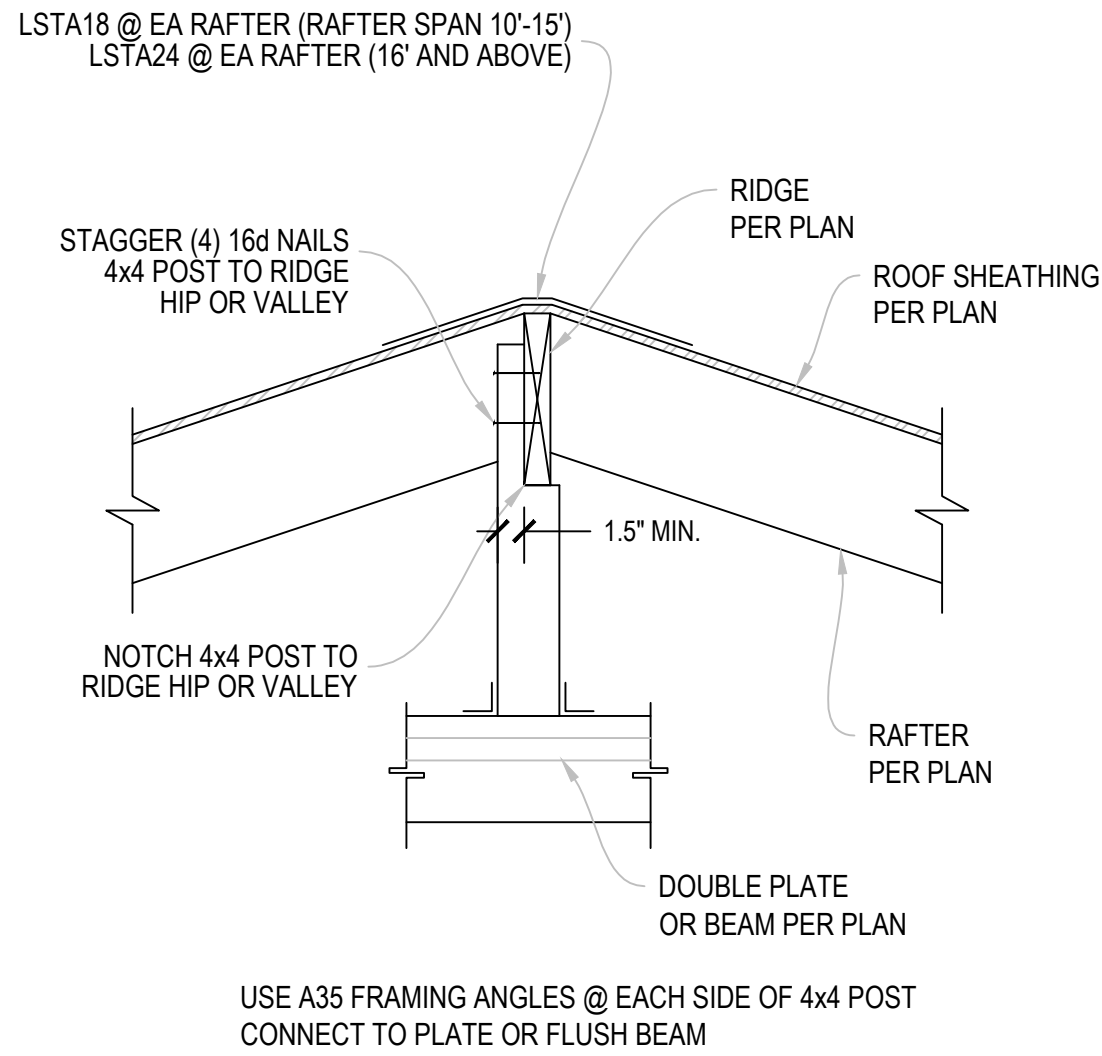
RAFTER CONNECTION DETAIL

4



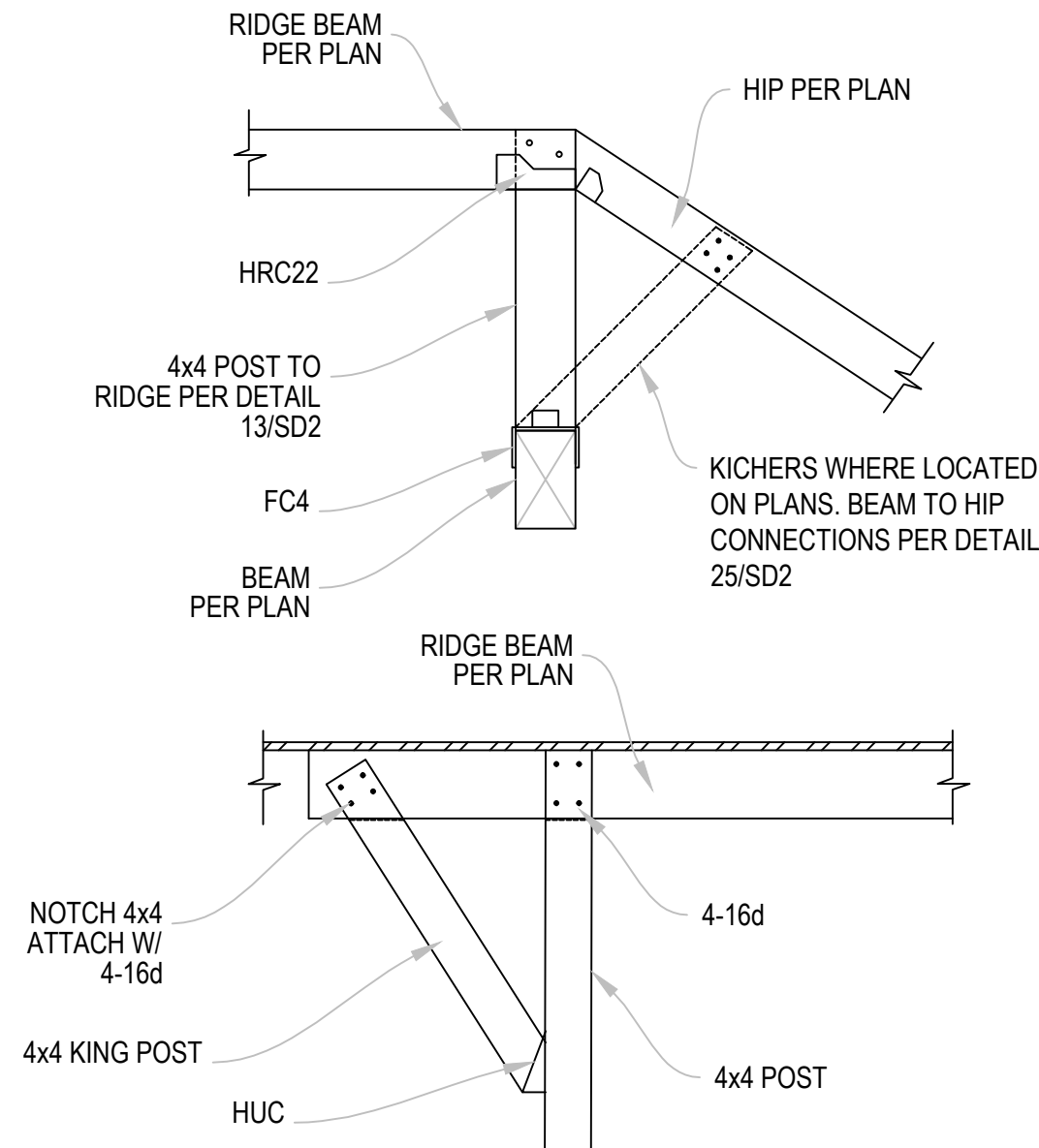
PARAPET DETAIL

1



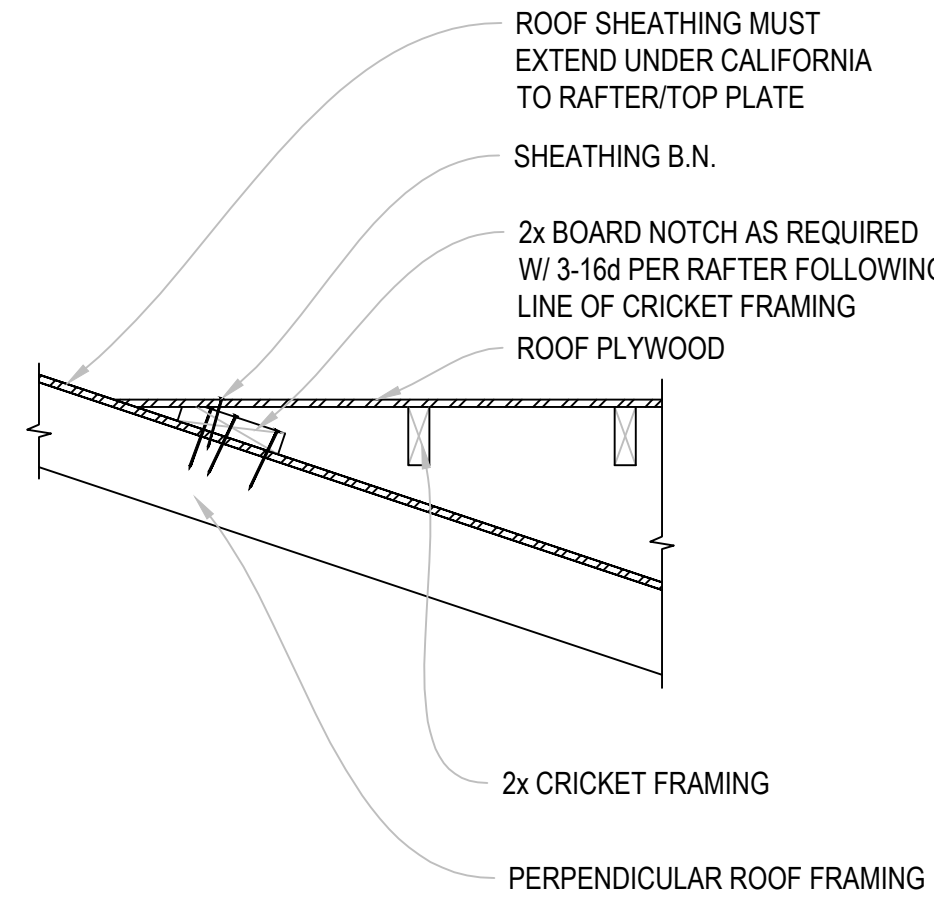
RIDGE CONNECTION DETAIL

11



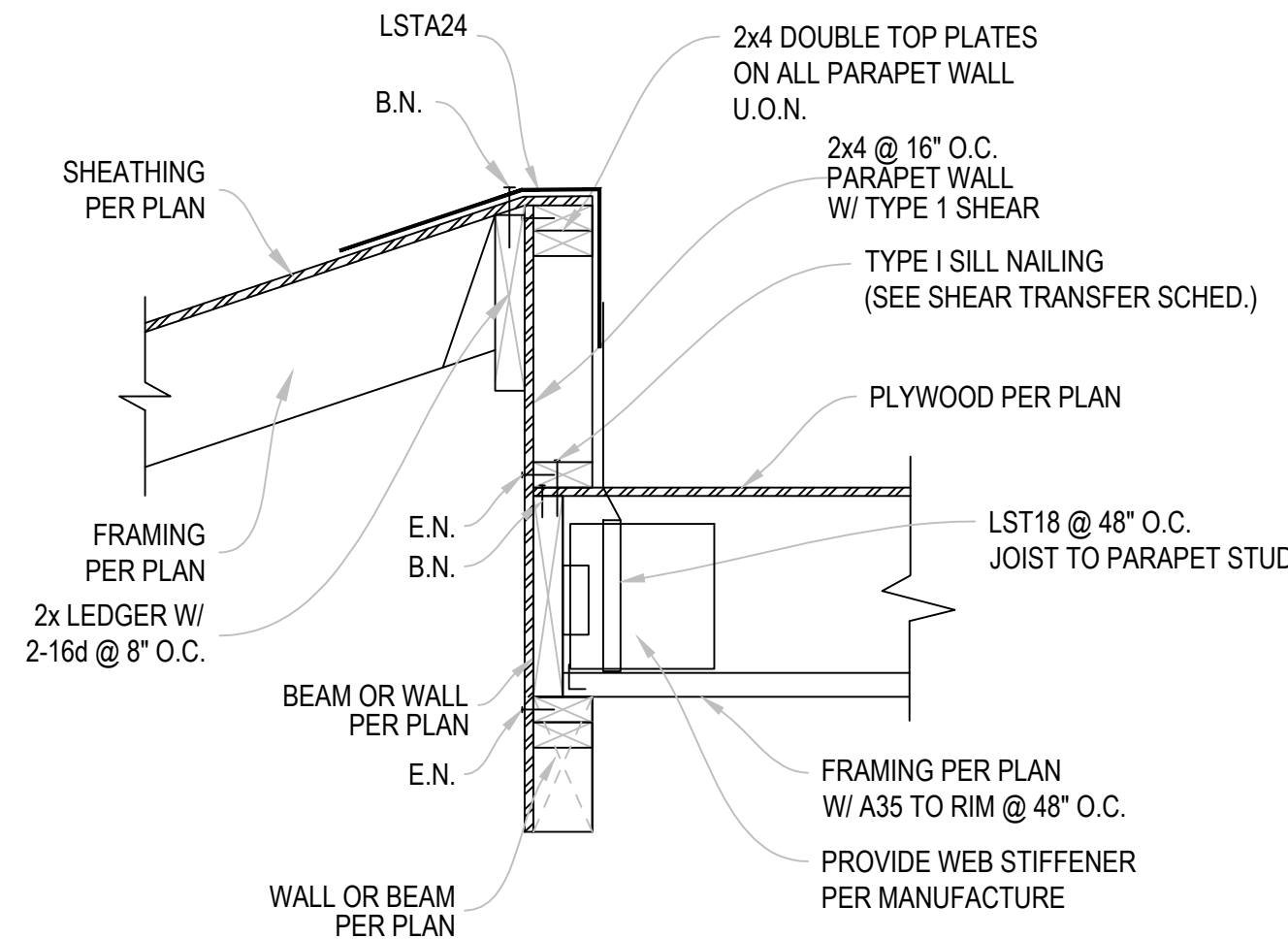
RIDGE TO HIP CONNECTION DETAILS

8



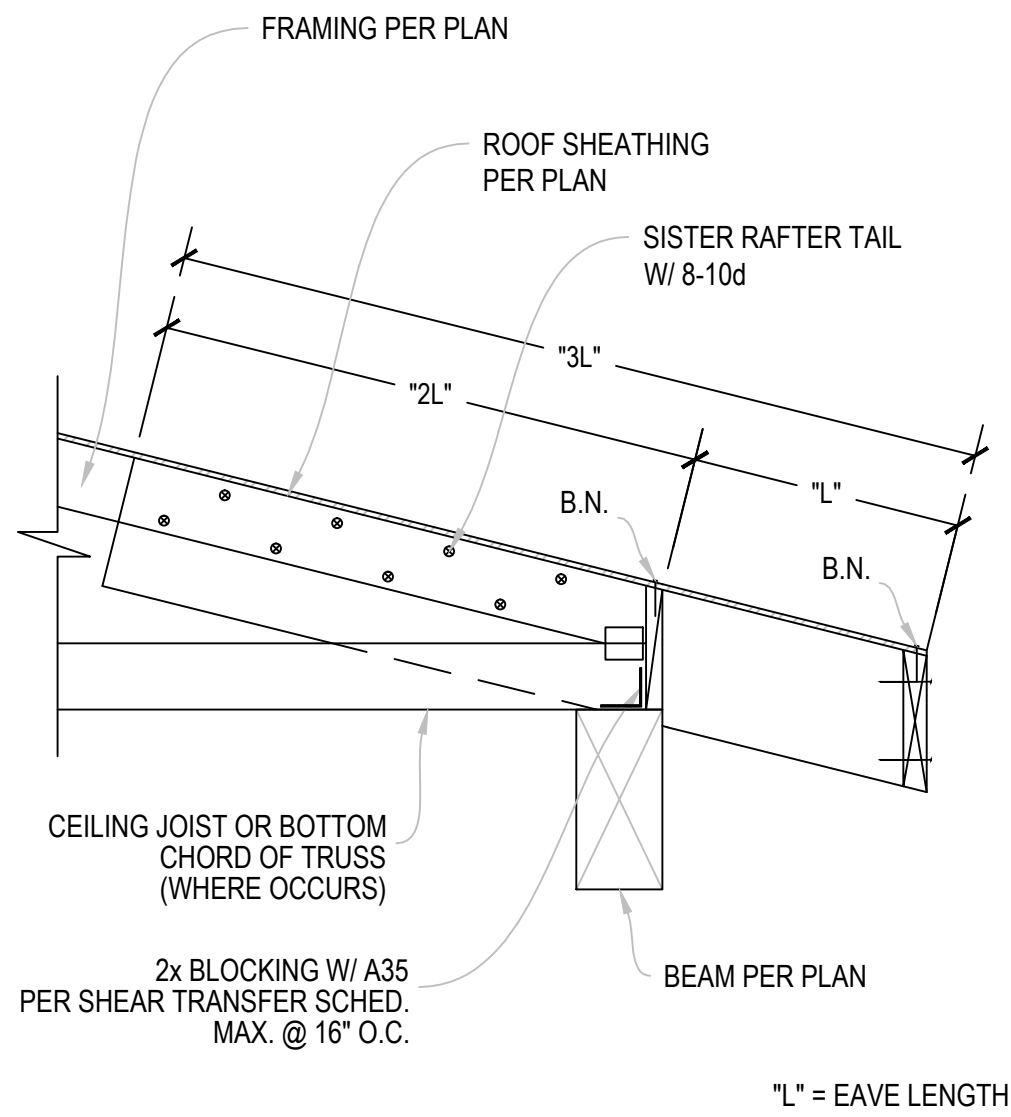
CAL-FILL SHEAR TRANSFER

5



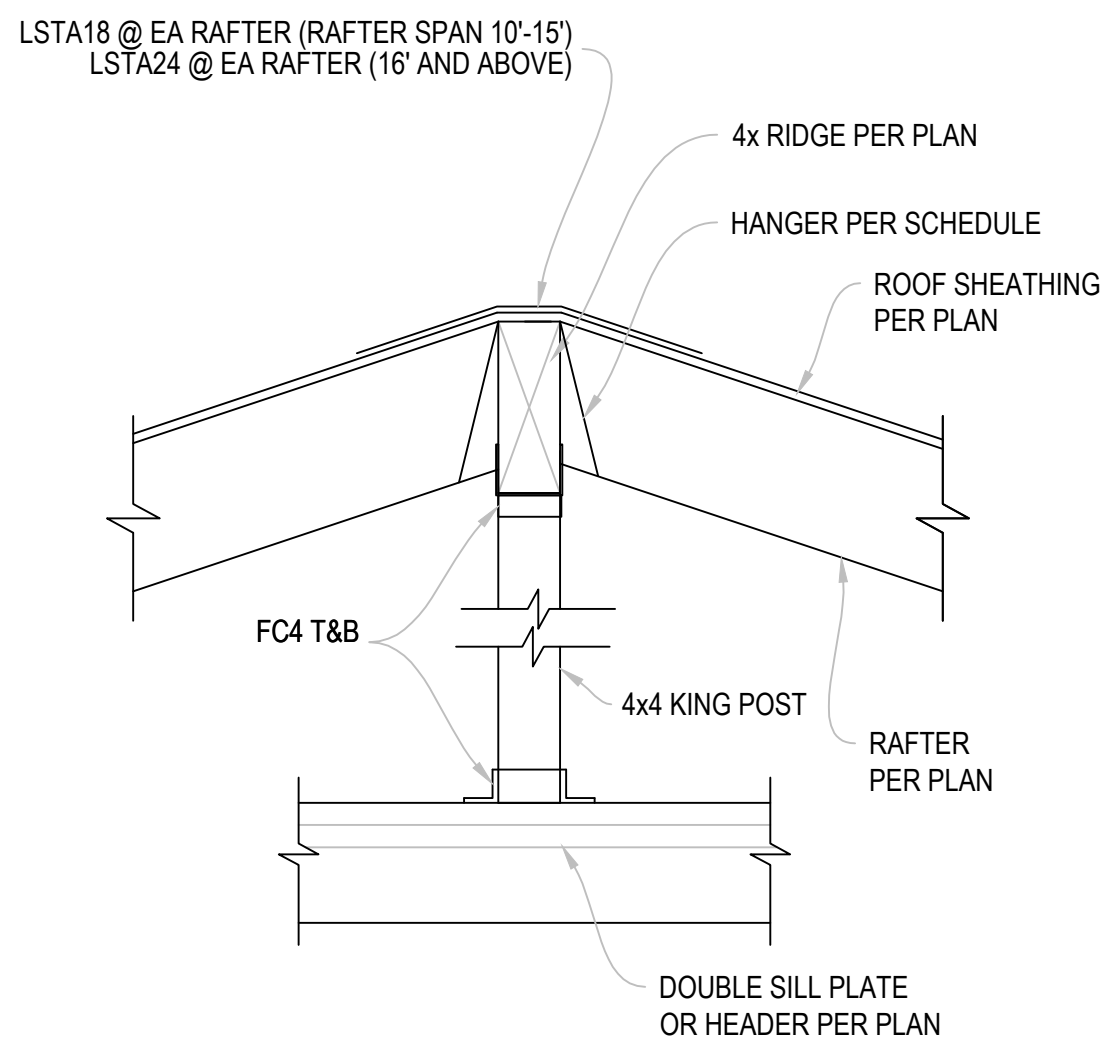
ROOF SHEAR TRANSFER

12



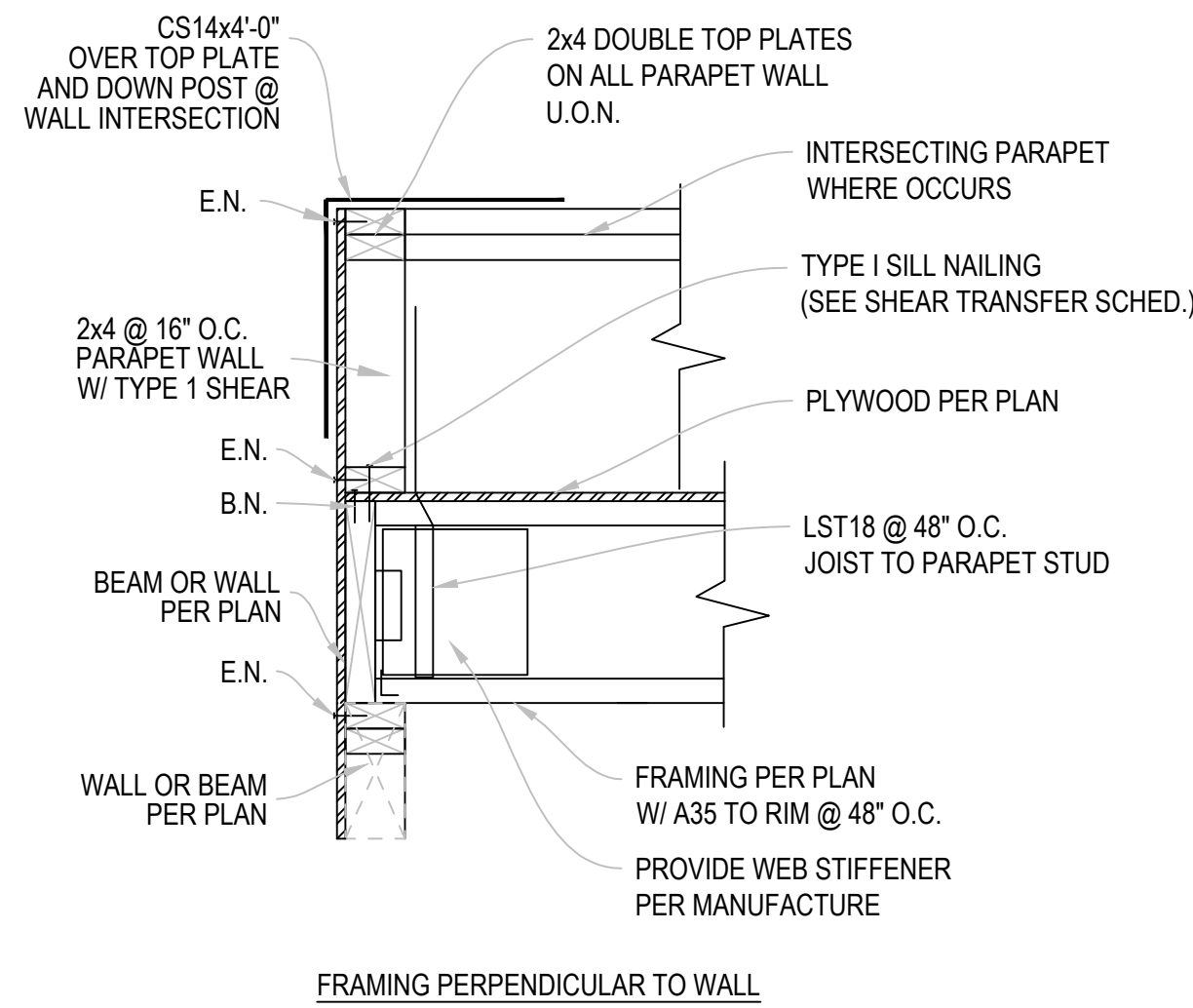
ROOF SHEAR TRANSFER

9



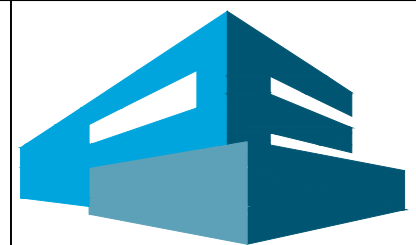
RIDGE CONNECTION DETAIL

6



PARAPET DETAILS

3



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CONTENT

ROOF
FRAMING
DETAILS

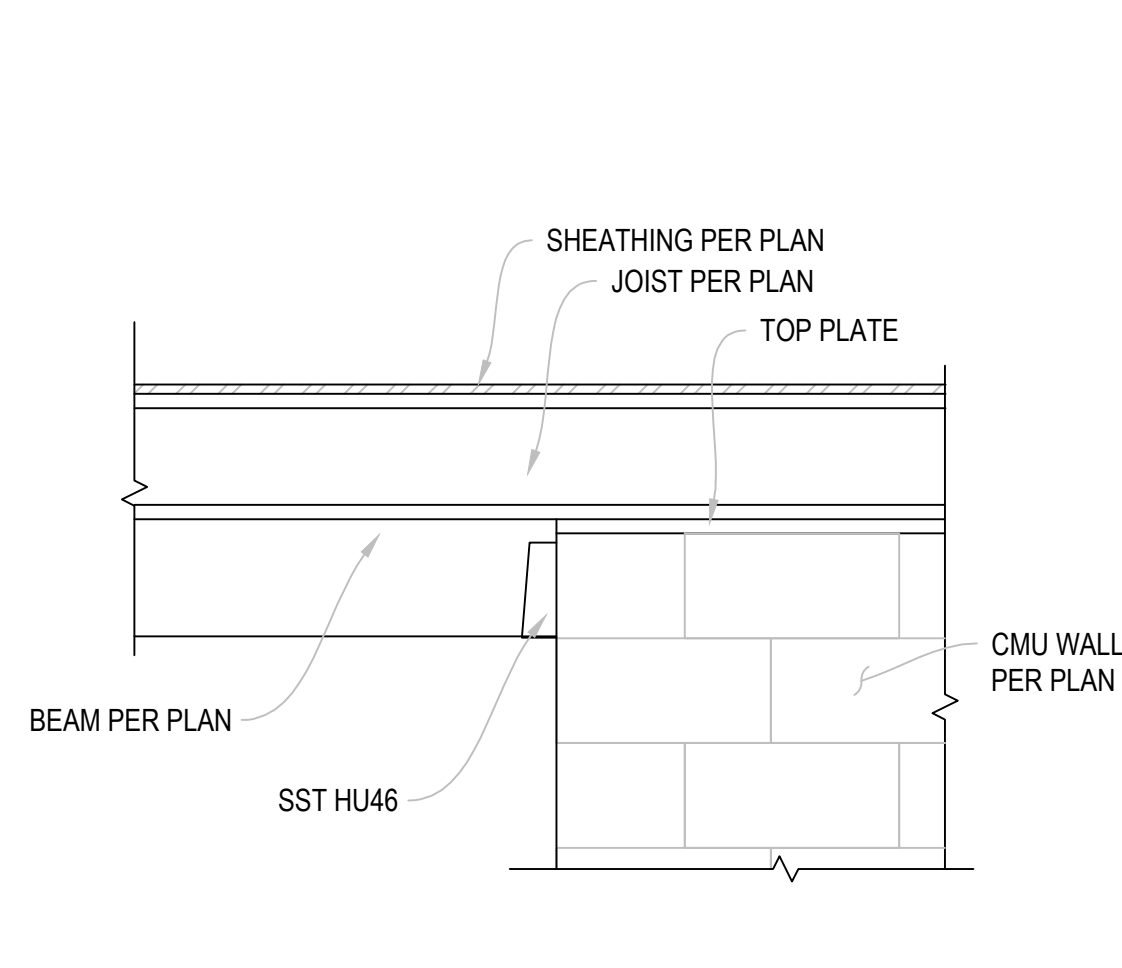
DRAWN BY: CR/CKP

CHECKED BY: CKP

DATE: SEPTEMBER 20, 2013

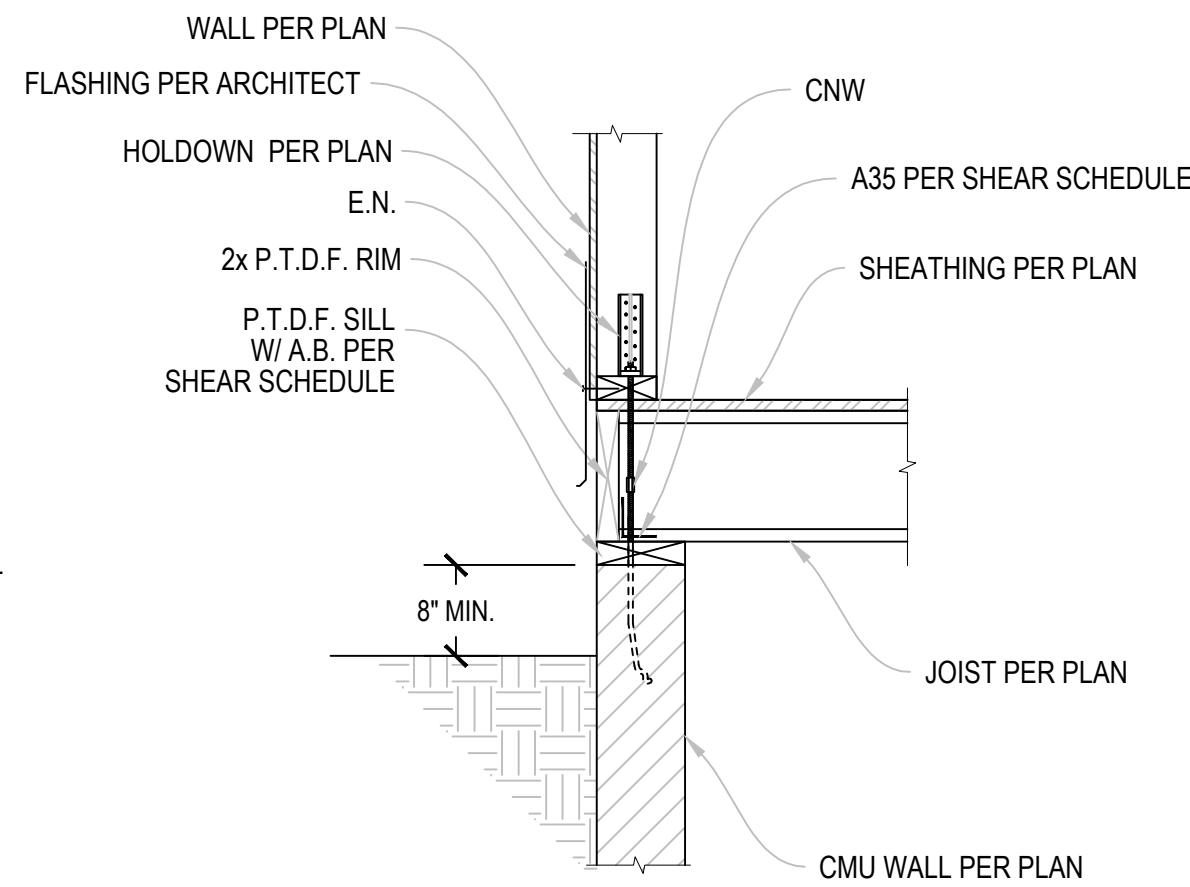
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S4.2



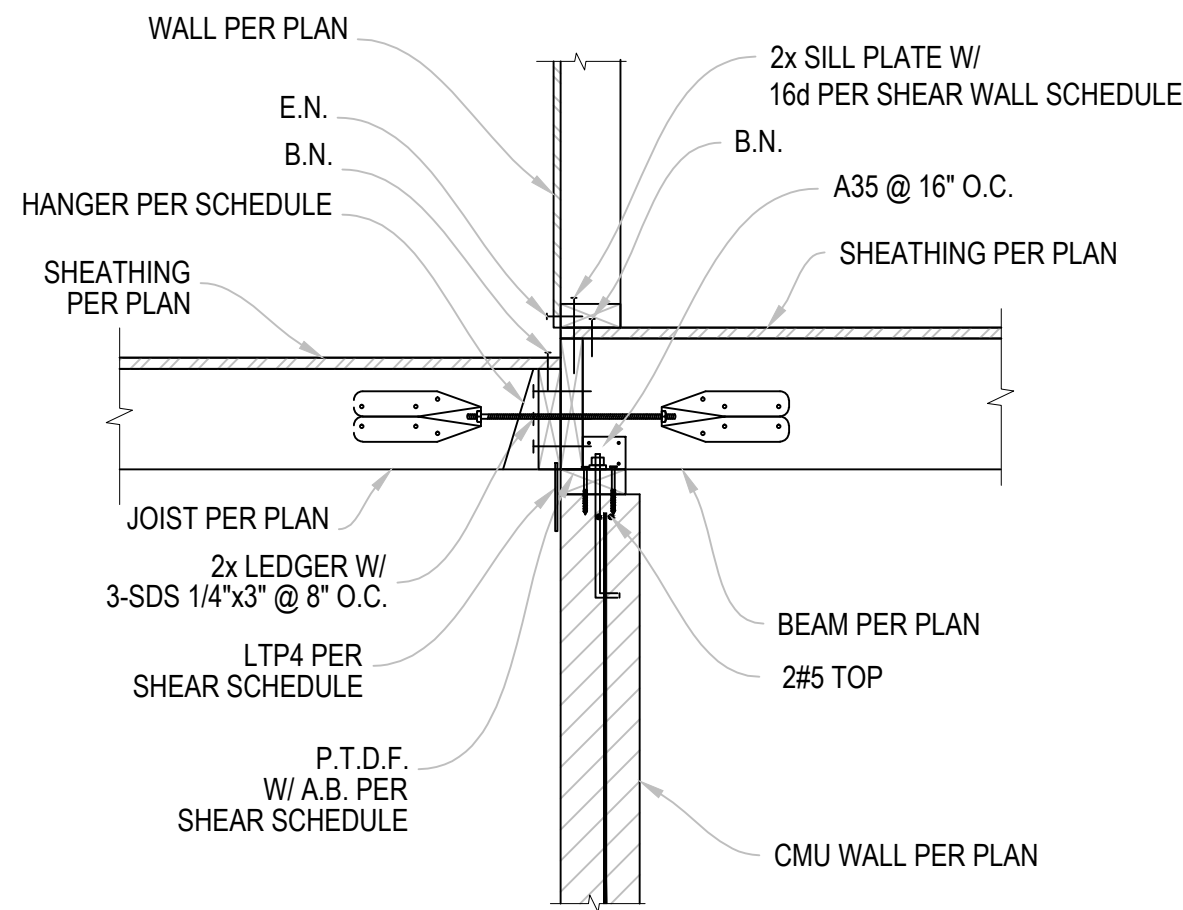
(N) BEAM TO MASONRY / FOOTING

13



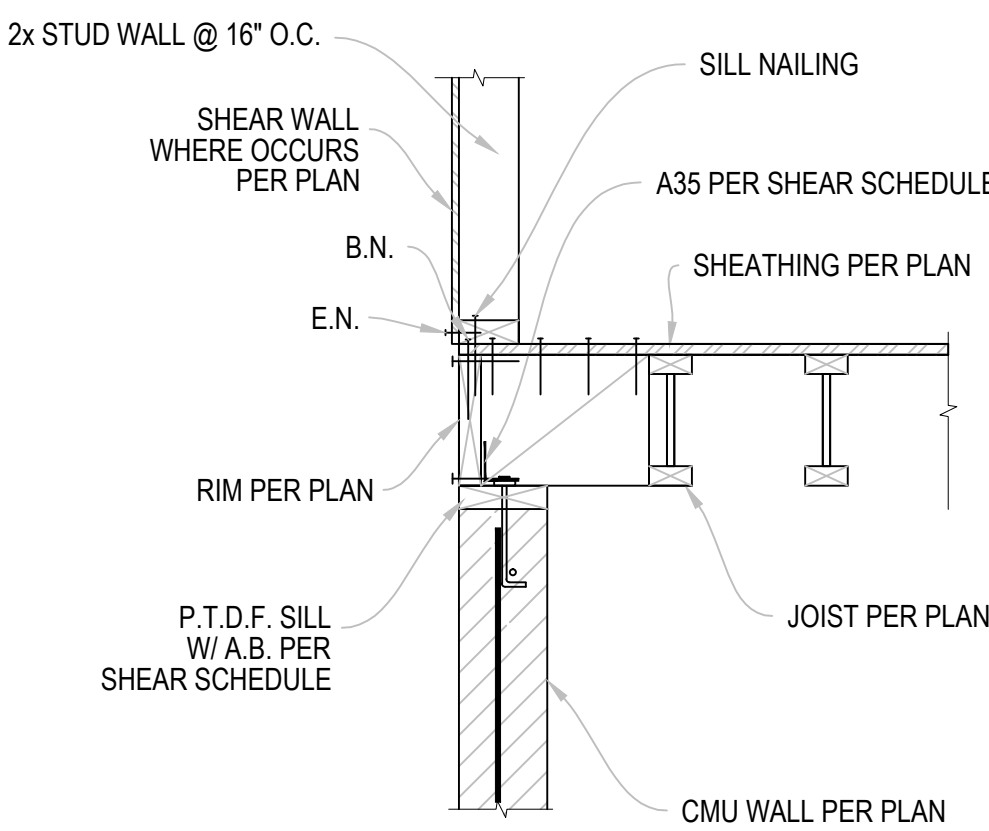
JOIST TO CMU CONNECTION

10



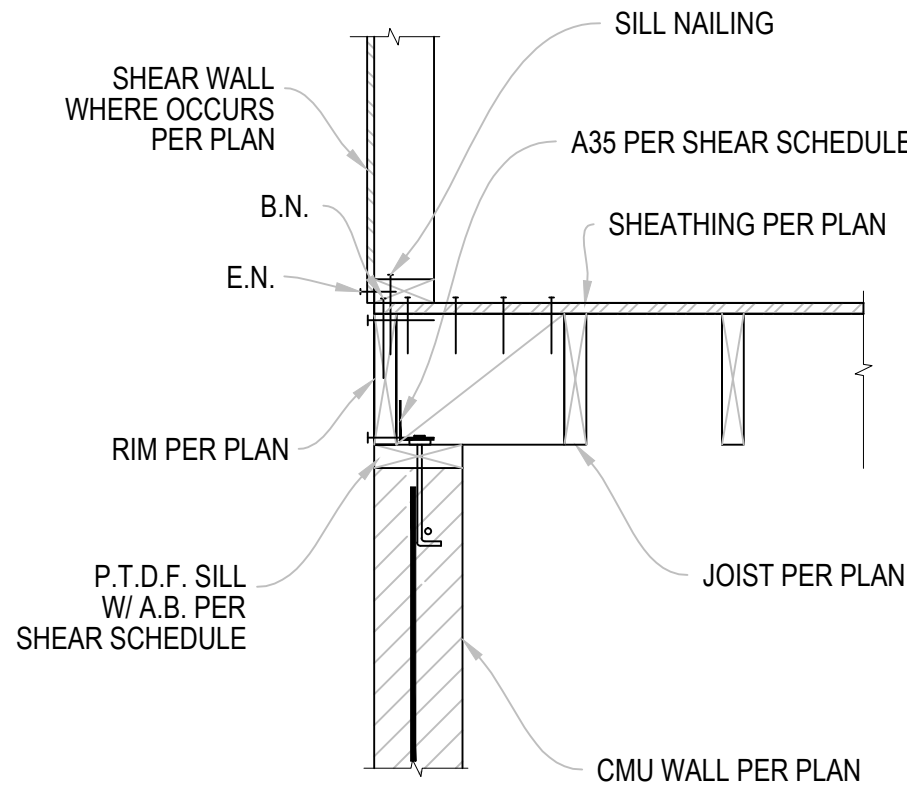
JOIST TO CMU WALL CONNECTION

14



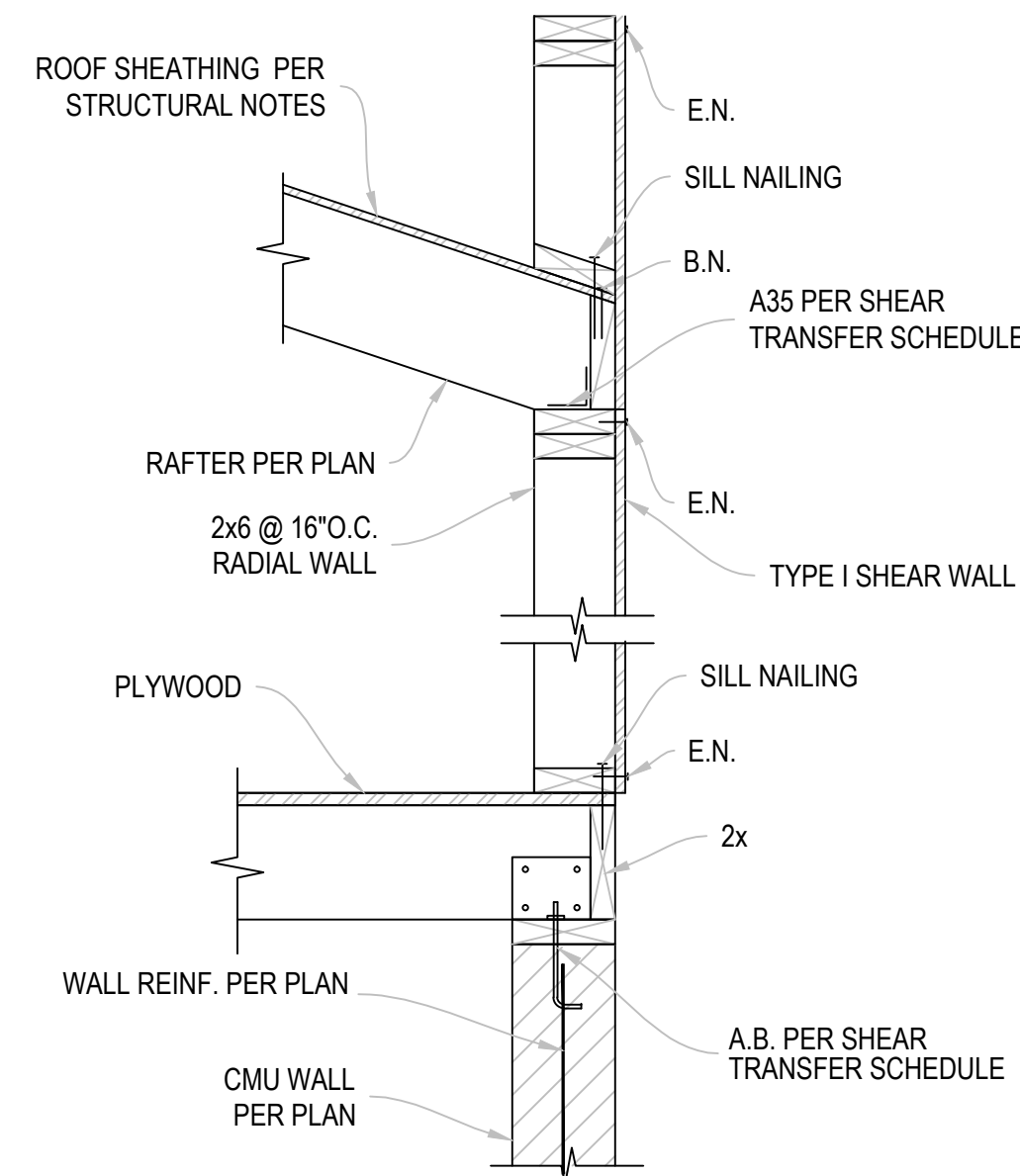
SHEAR TRANSFER DETAIL

11



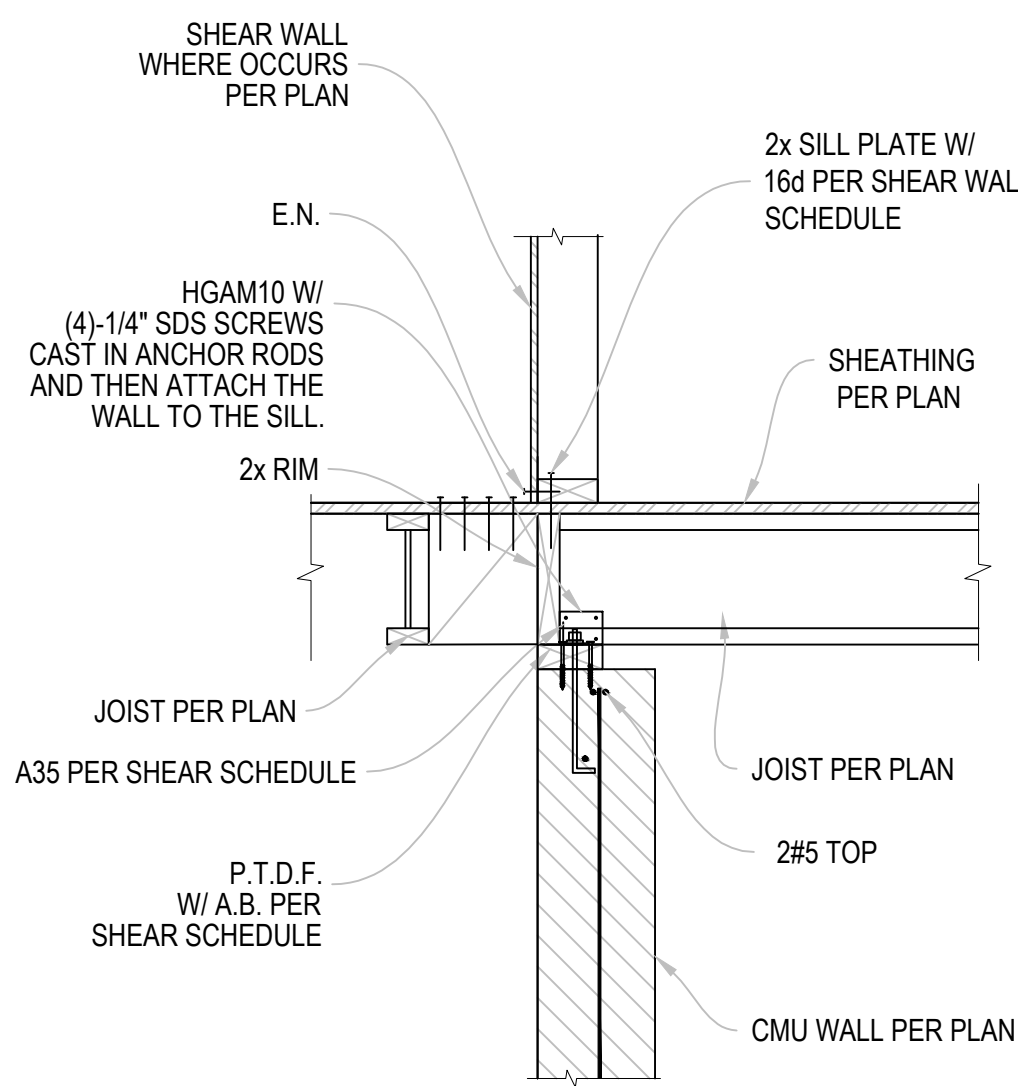
SHEAR TRANSFER DETAIL

8



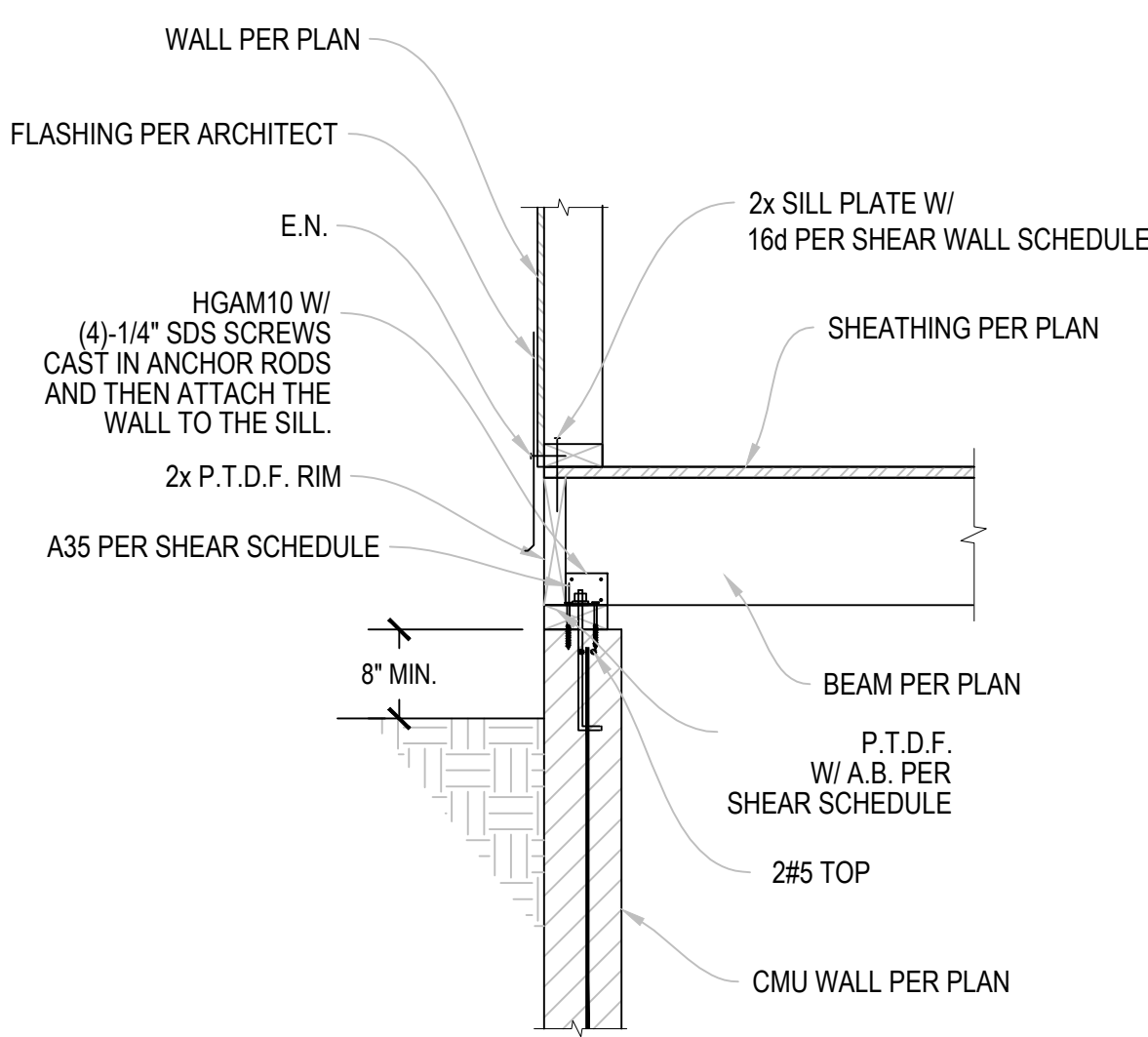
ROOF FRAMING CONNECTION

5



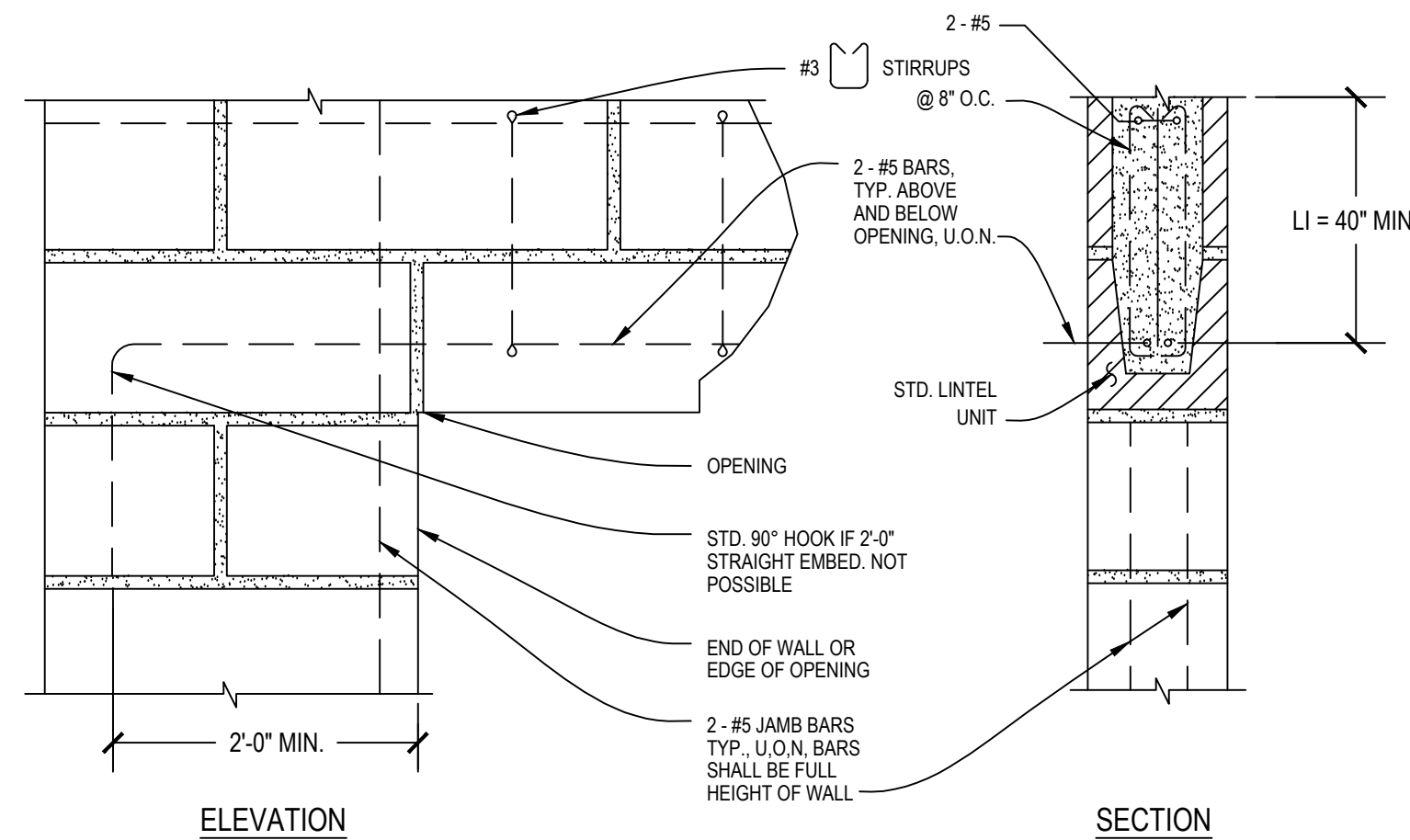
JOIST TO RETAINING WALL CONNECTION

15



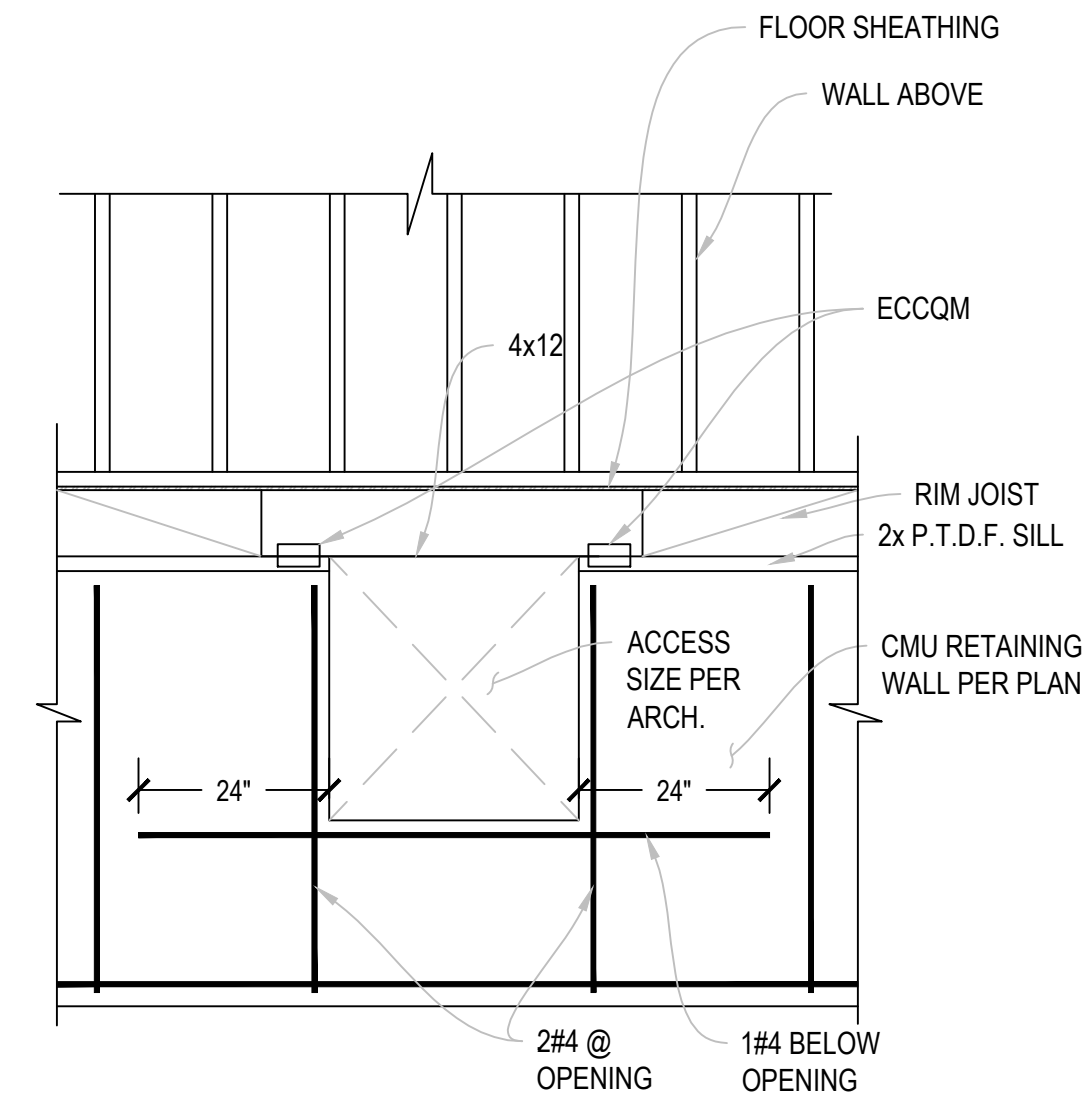
BEAM TO CMU CONNECTION

12



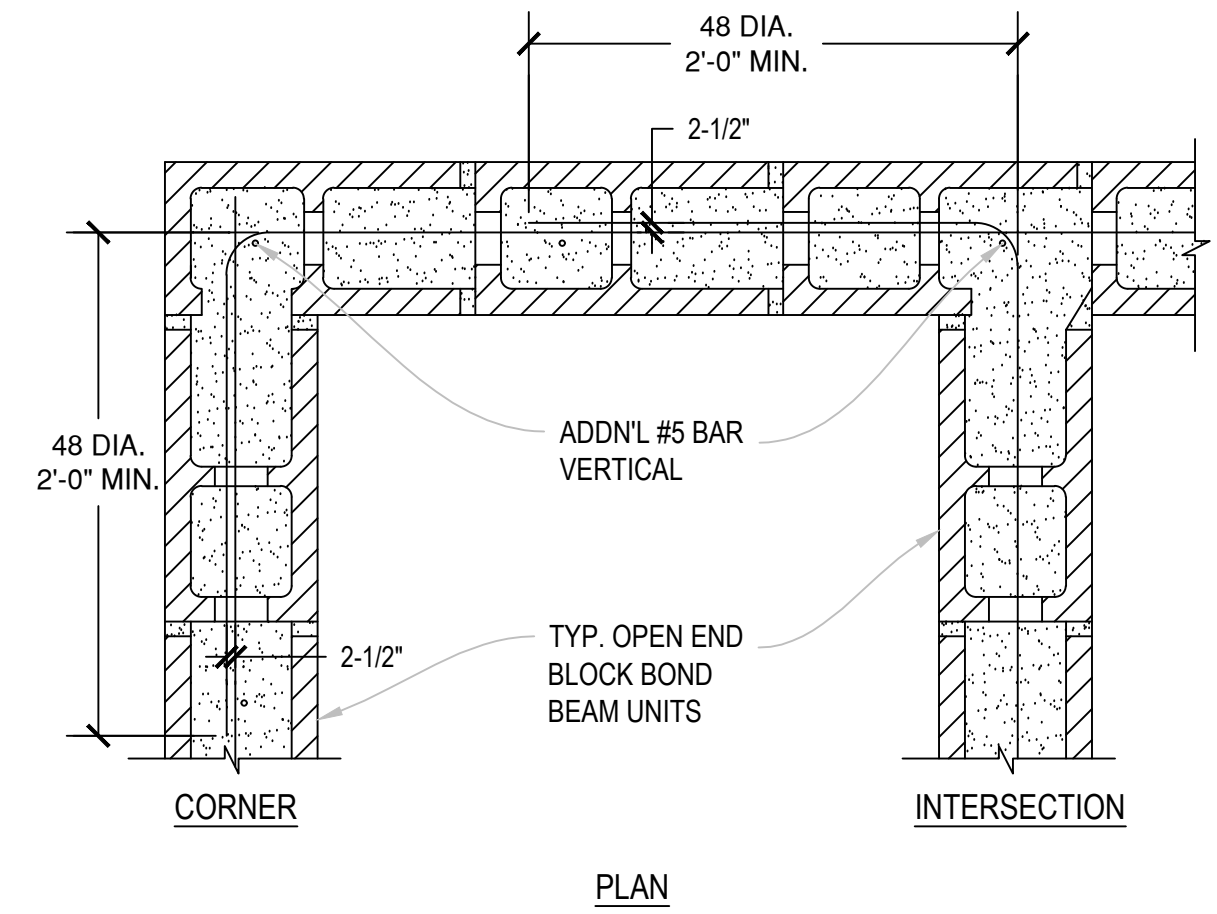
TYP. REINFORCING AT ENDS & OPENINGS IN CONCRETE MASONRY WALLS

6



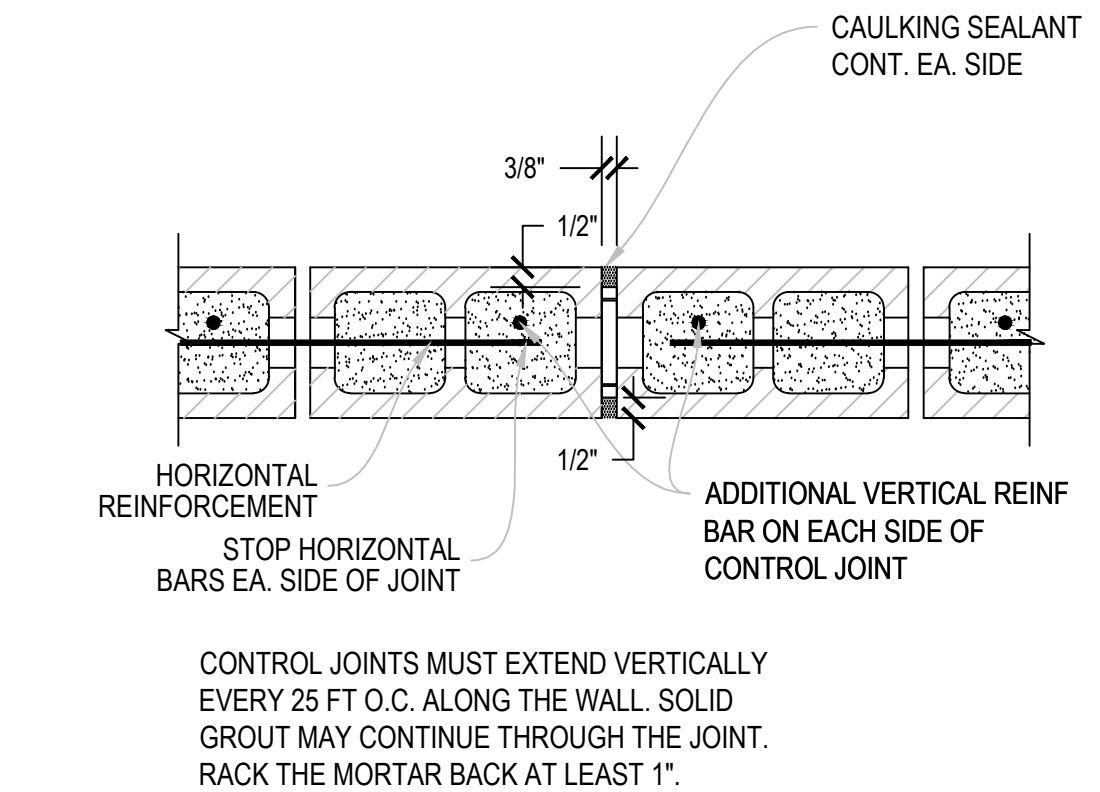
CRAWL SPACE ACCESS DETAIL

4



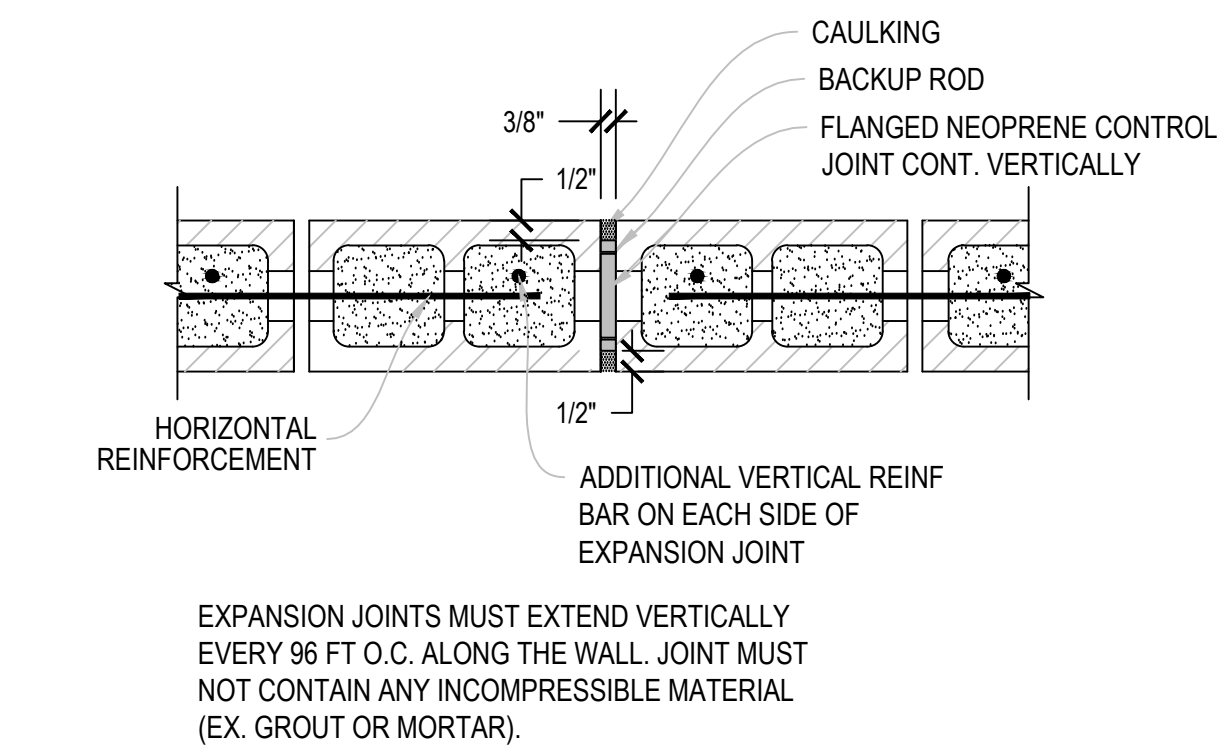
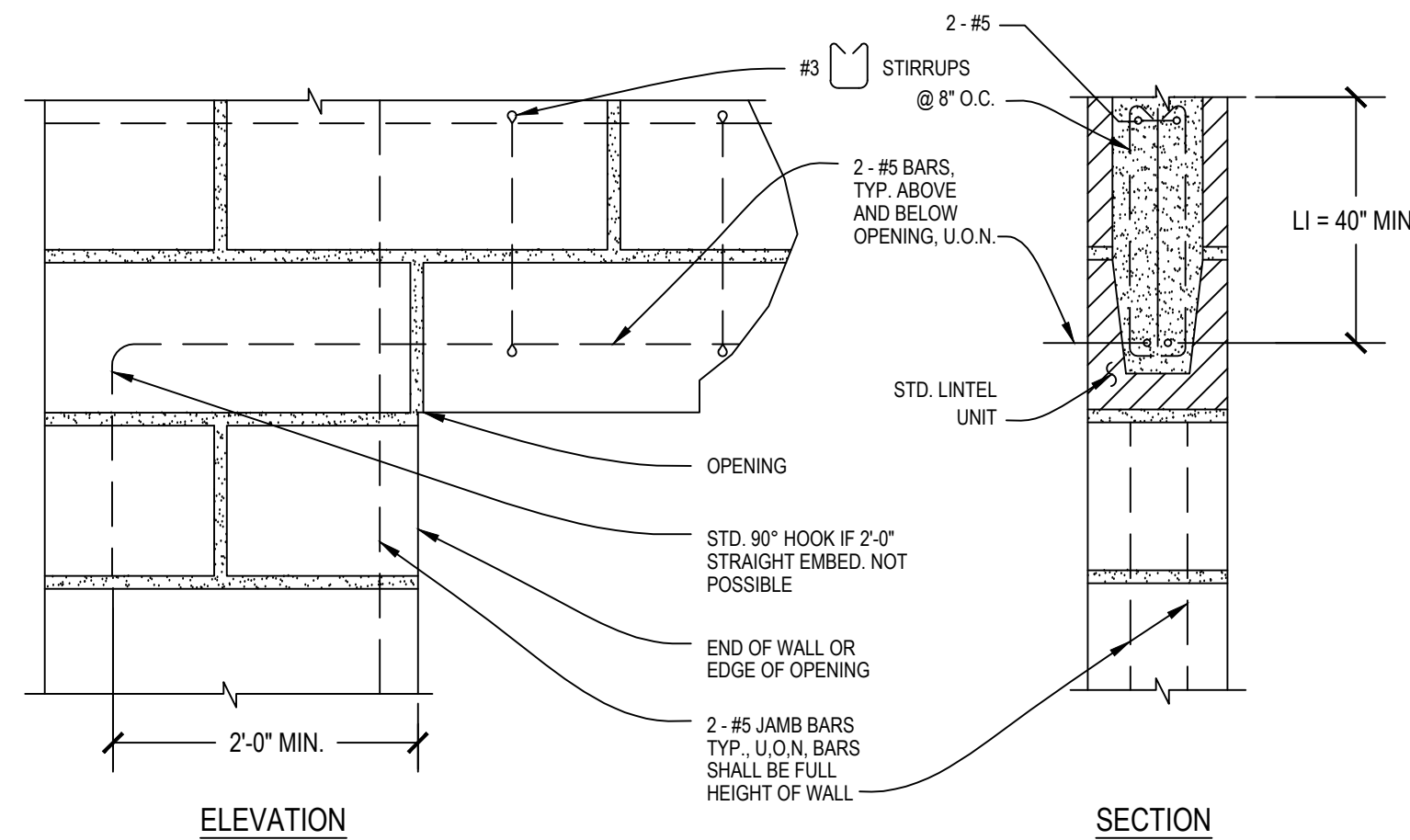
TYPICAL CONCRETE MASONRY WALL CORNERS AND INTERSECTIONS

1



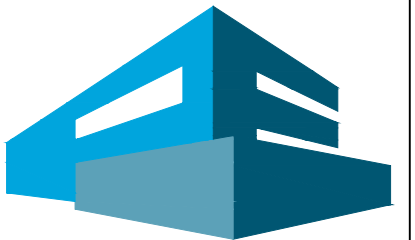
CONTROL JOINT DETAIL

2



EXPANSION JOINT DETAIL

3



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CONTENT

MASONRY
DETAILS

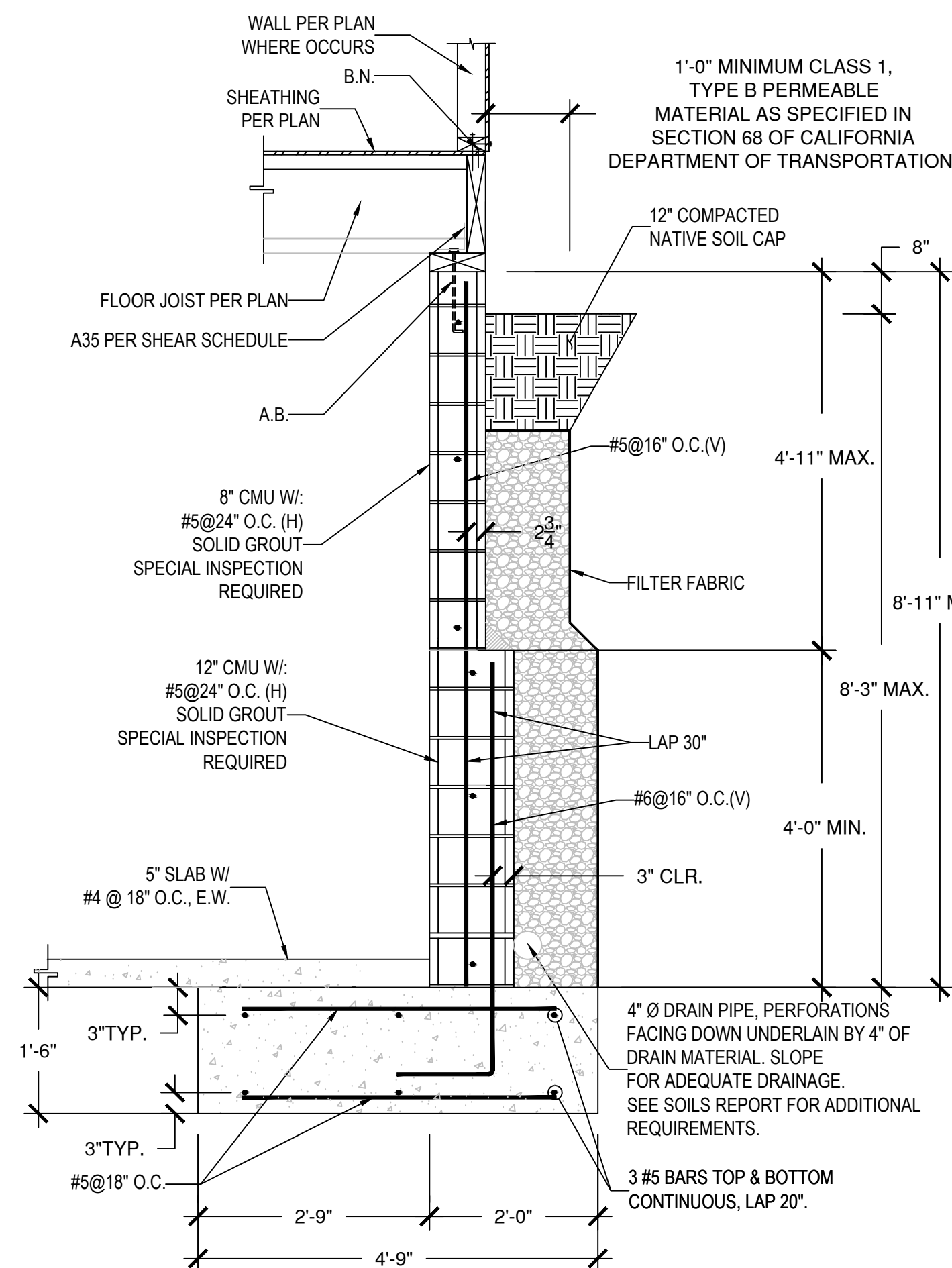
DRAWN BY: CR/CKP

CHECKED BY: CKP

DATE: SEPTEMBER 20, 2013

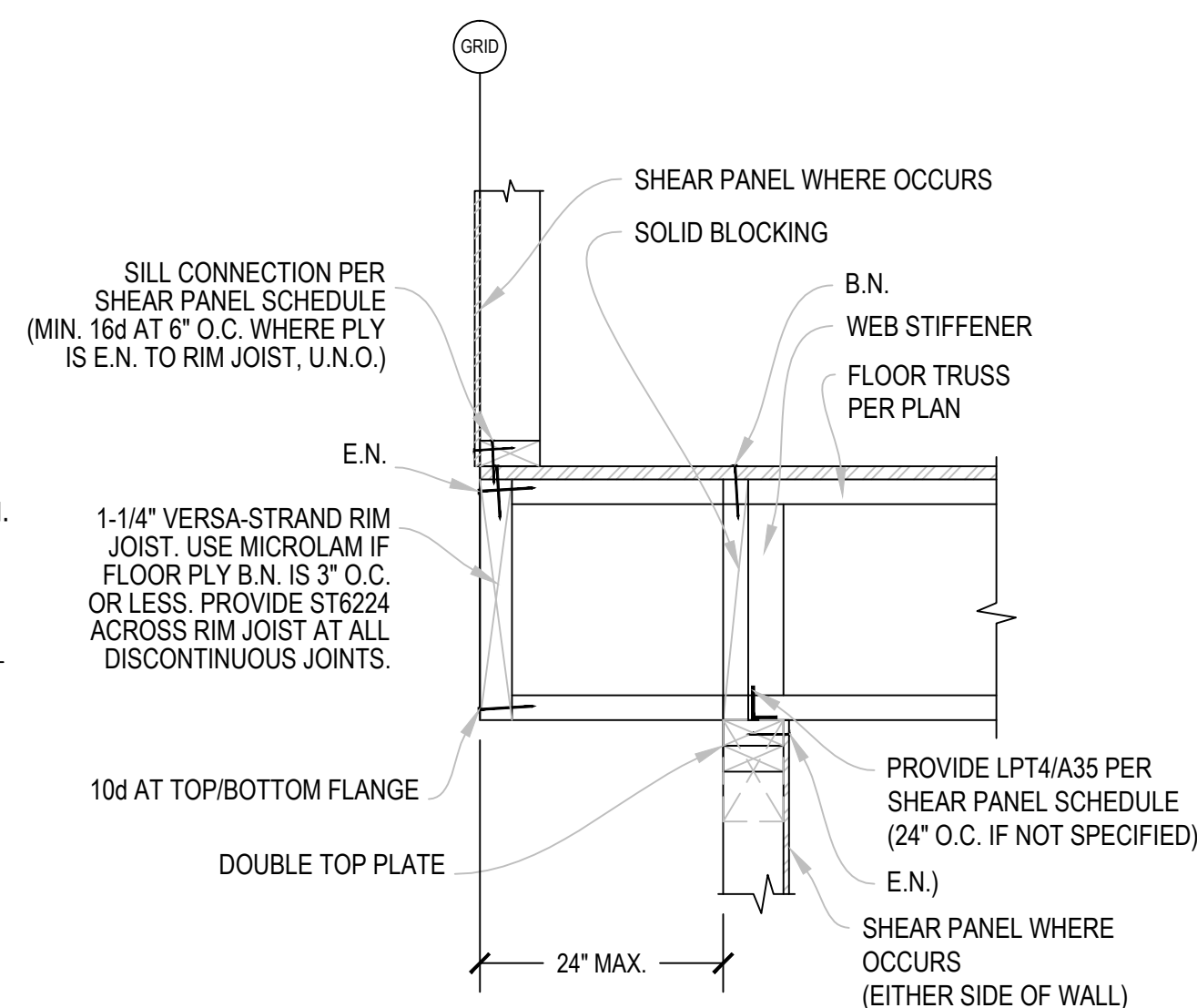
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S4.3



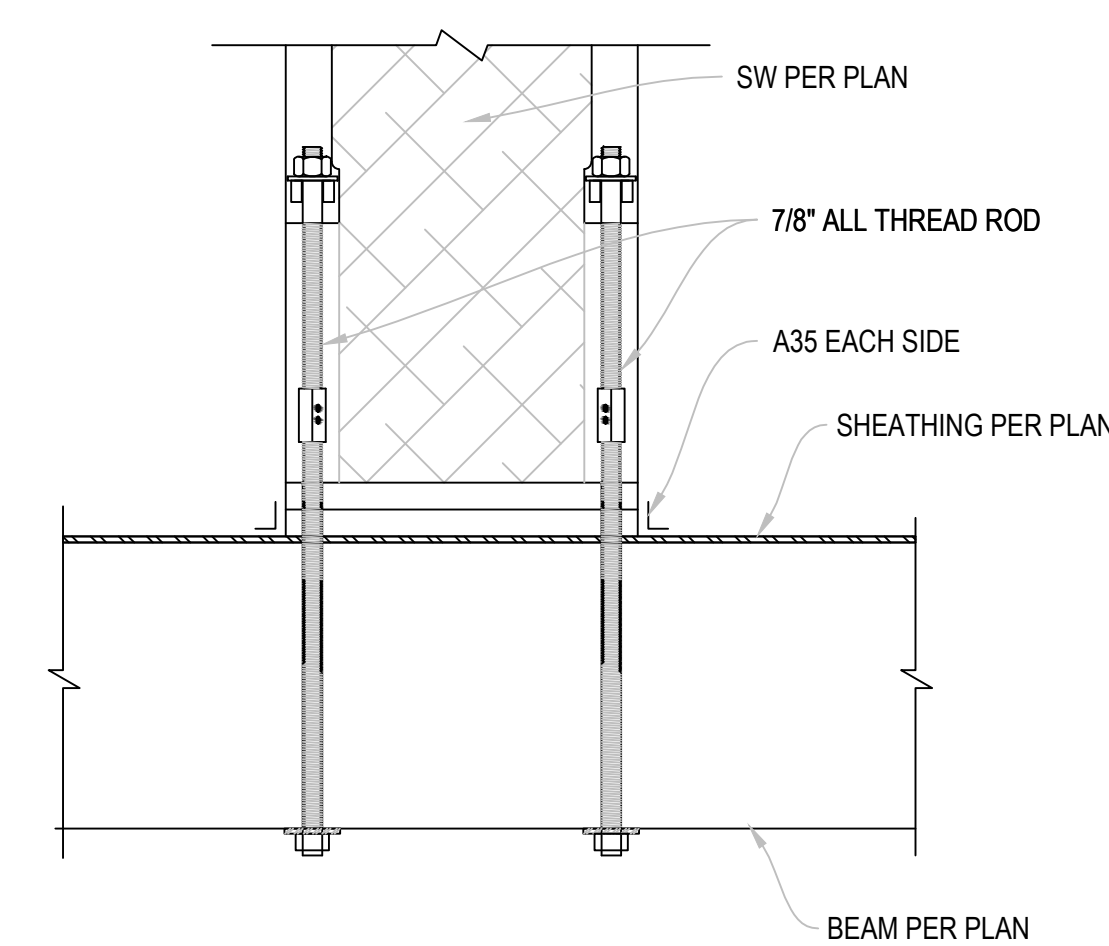
RETAINING WALL DETAIL

4



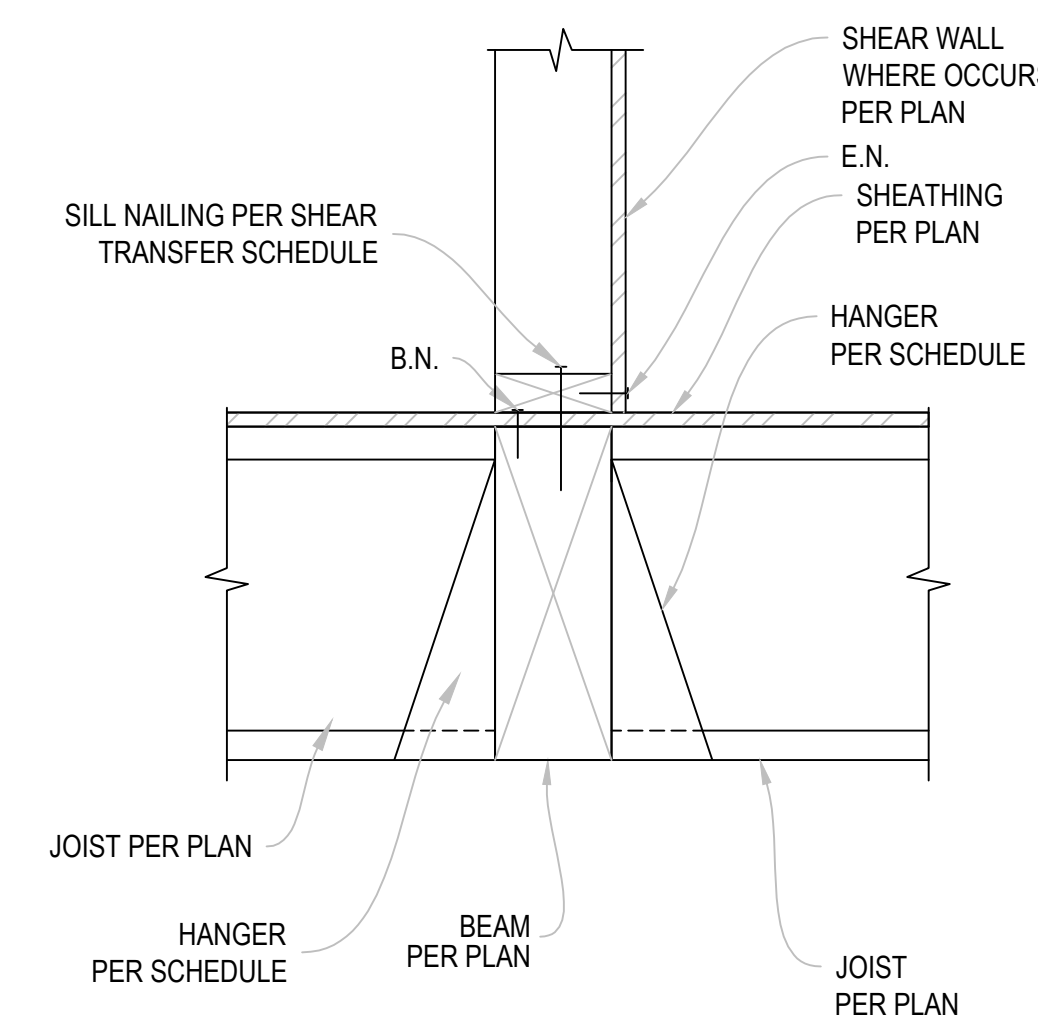
SHEAR TRANSFER AT CANTILEVER JOIST

1



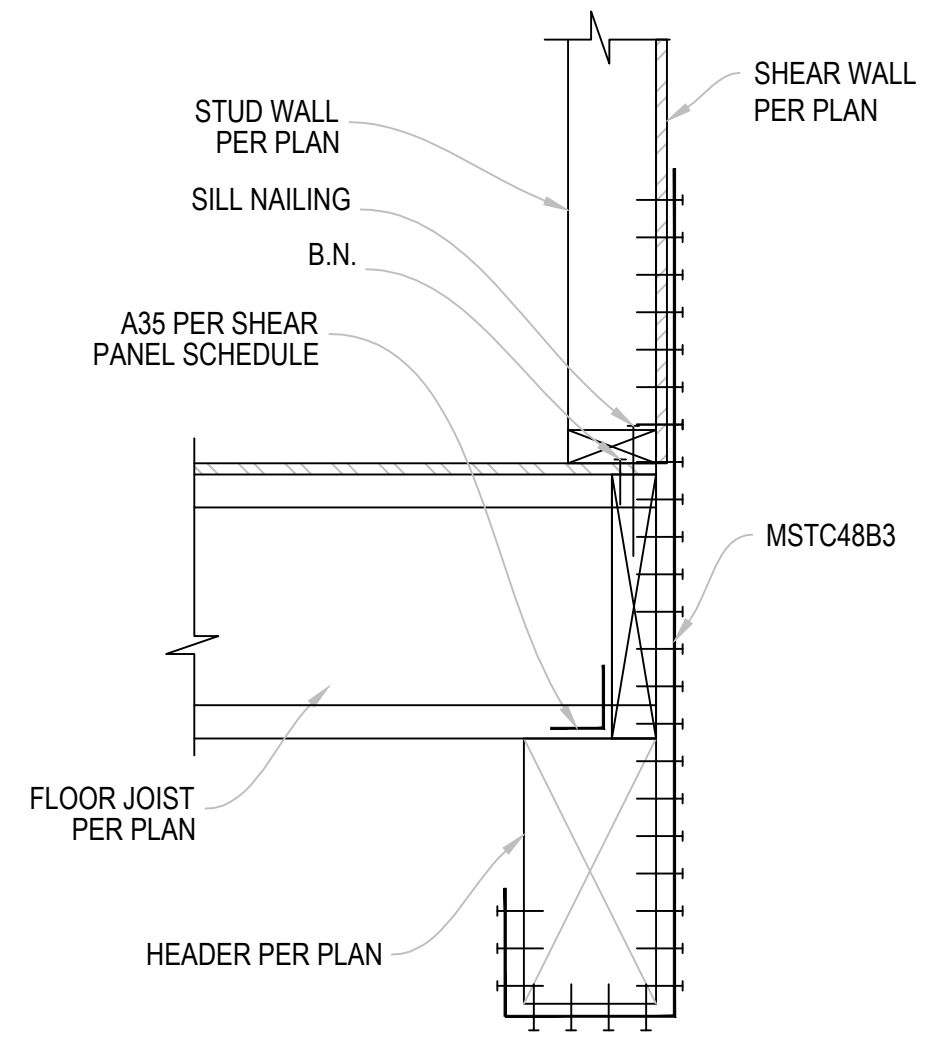
SW TO BEAM DETAIL

2



JOIST TO BEAM CONNECTIONS

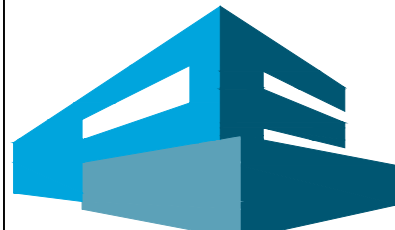
5




SHEAR TRANSFER DETAIL

3

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☐ CONSTRUCTION DOCUMENTS.

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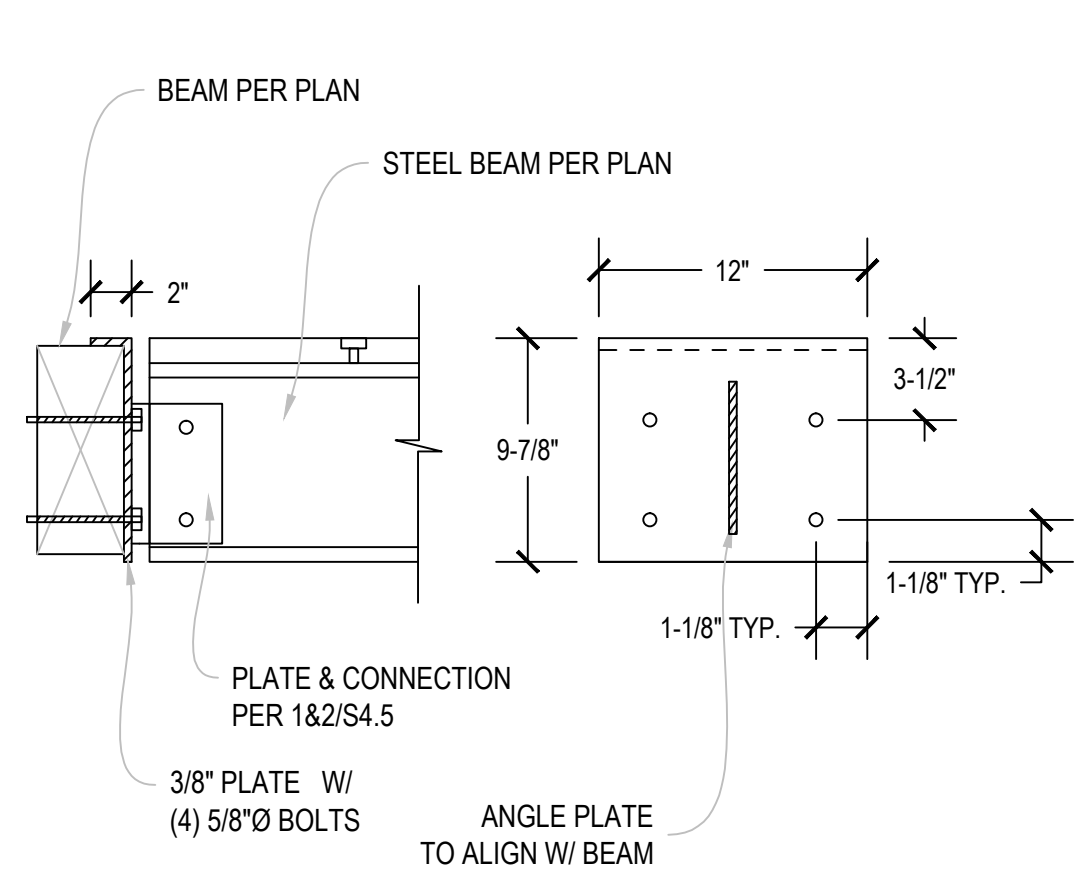
FRAMING
DETAILS

DRAWN BY: CR/CKP

CHECKED BY: CKP

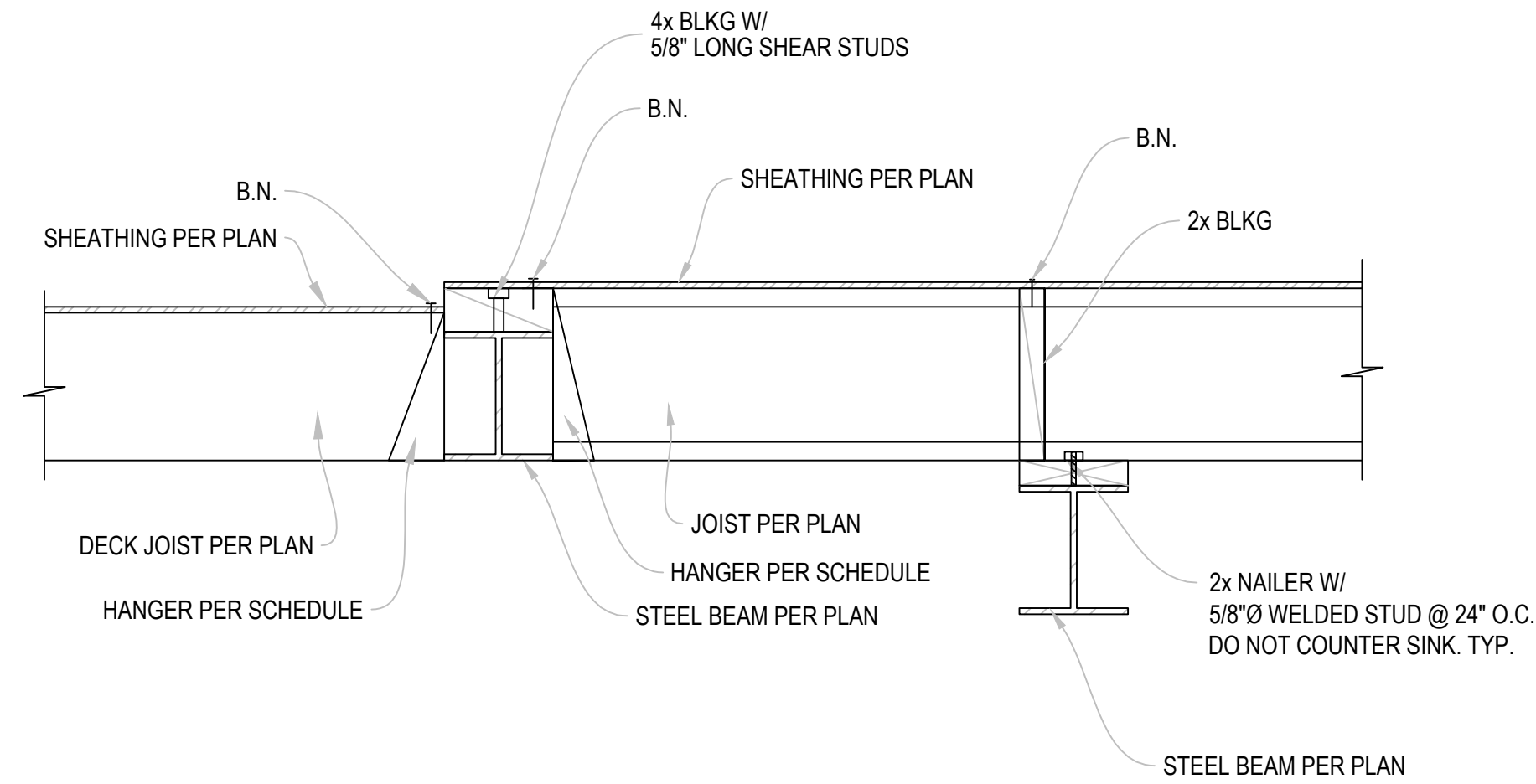
DATE: SEPTEMBER 20, 2013

SHEET NO. S4.4



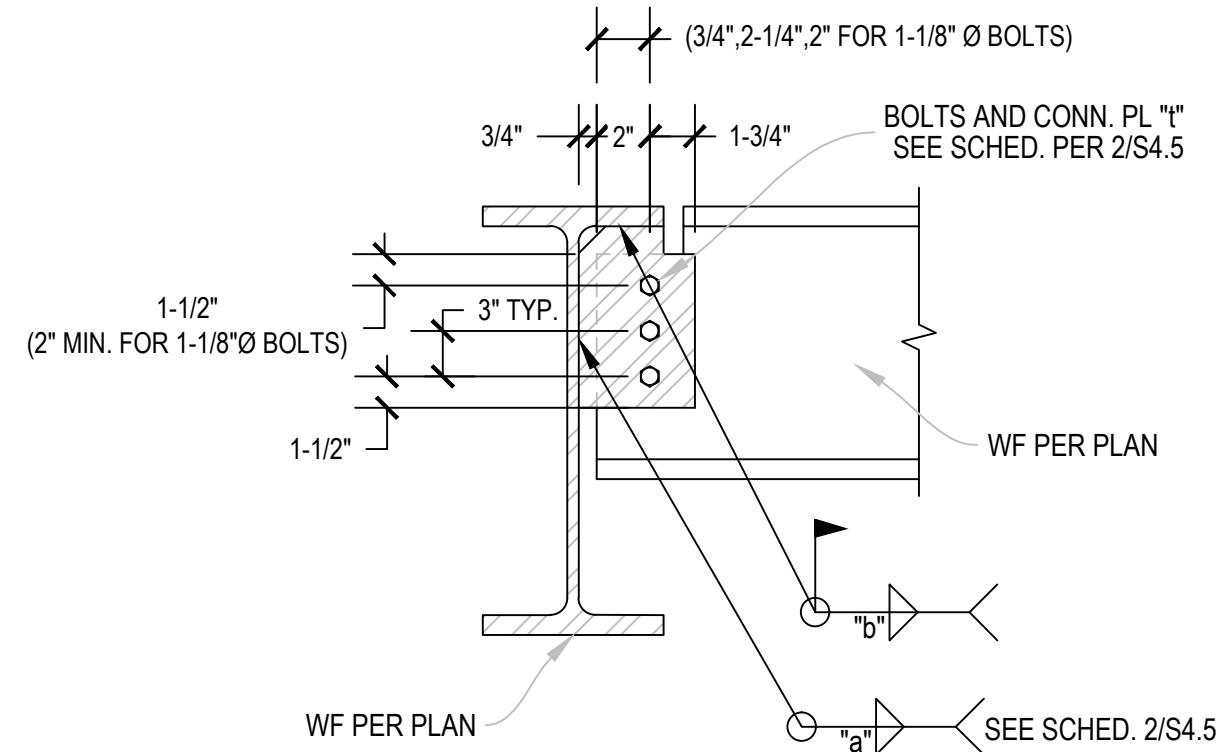
BEAM TO BEAM CONNECTION

10



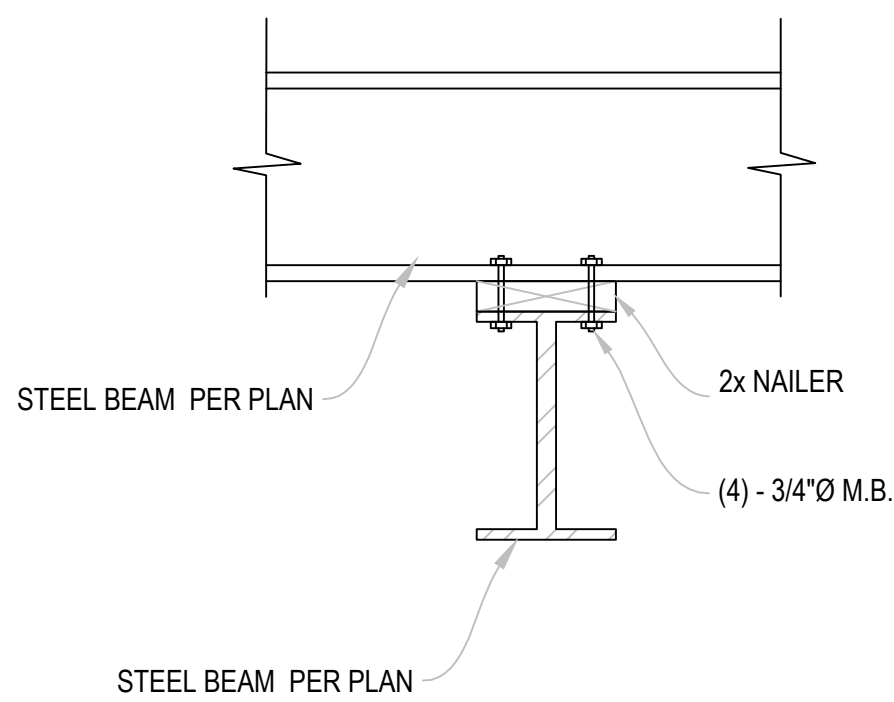
FRAMING TO W BEAM CONNECTION

4



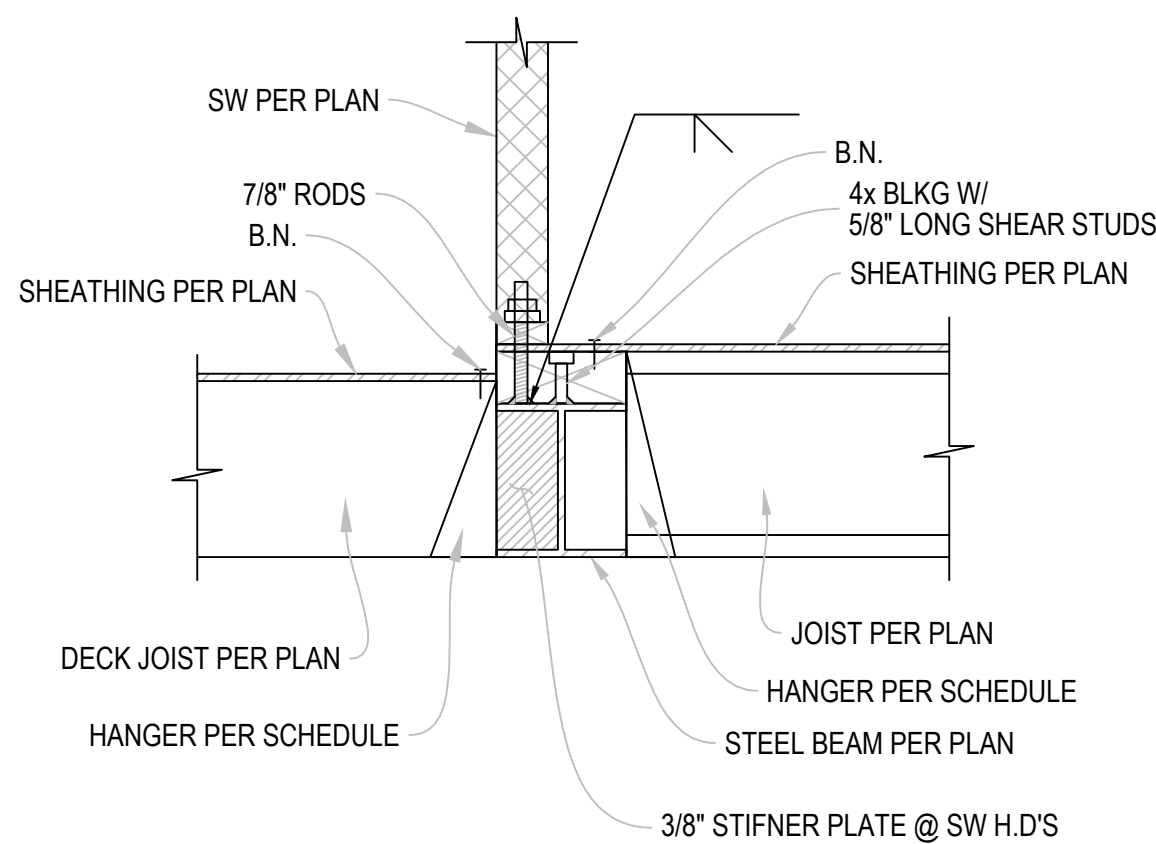
BEAM TO BEAM CONNECTION

1



BEAM TO BEAM CONNECTION

8



SW TO BEAM CONNECTION

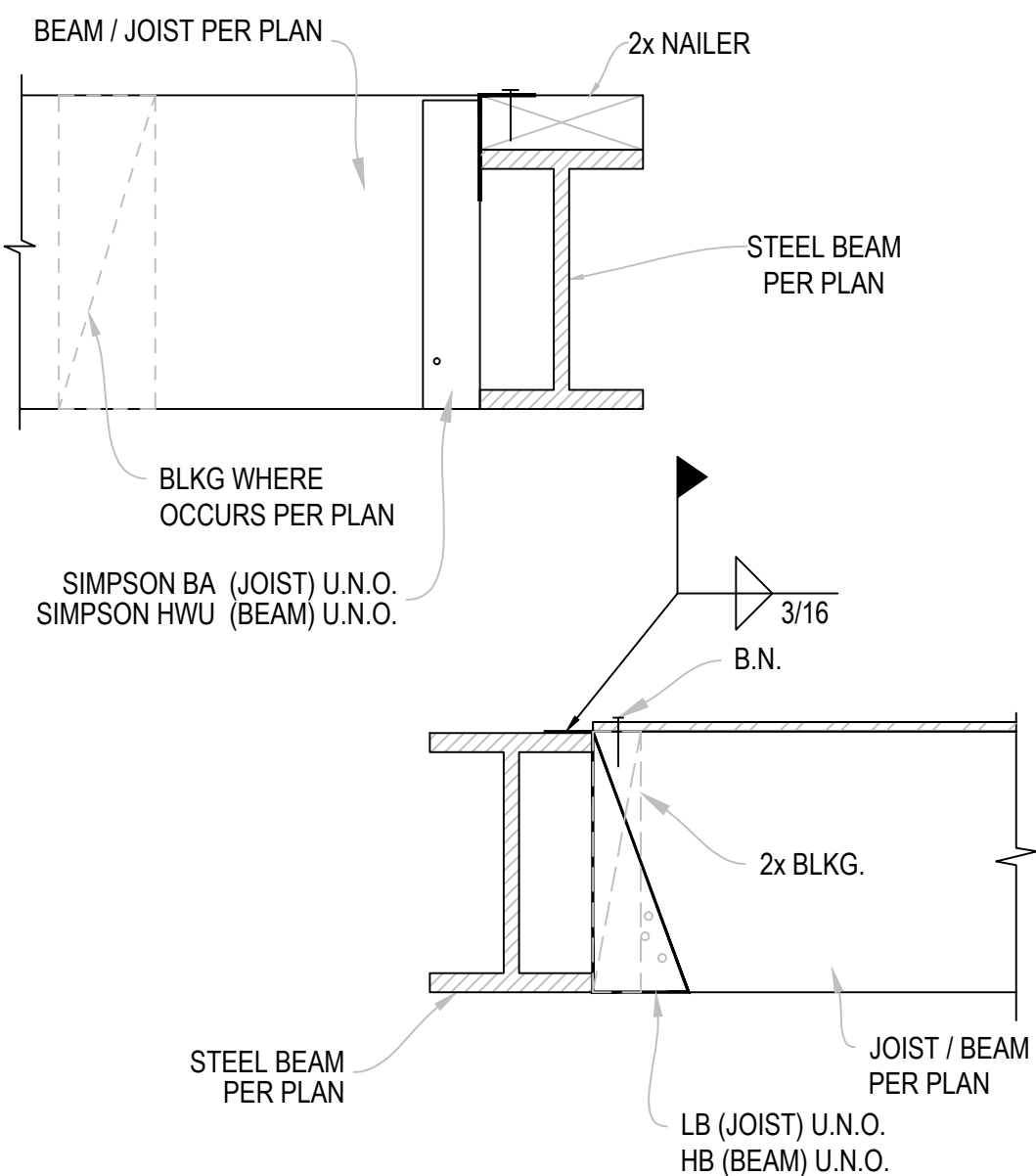
5

BEAM CONNECTION SCHEDULE					
BEAM / GIRDER SIZE	NUMBER & SIZE OF A325X BOLTS U.N.O.		CONNECTION PLATE A-36 U.N.O.		
	ROW 1	ROW 2	PLATE THICKNESS "t"	WELD "a"	WELD "b"
W6x, C6	1-7/8"Ø	1-7/8"Ø	3/8"	1/4"	1/4"
C8x, C10x, W8x, W10x	2-7/8"Ø	—	3/8"	1/4"	1/4"
W12x, W14x, C12x	3-7/8"Ø	—	1/2"	5/16"	5/16"
W16x, W18x	4-7/8"Ø	—	1/2"	5/16"	5/16"
W21x	5-7/8"Ø	—	1/2"	5/16"	5/16"
W24x	6-7/8"Ø	—	1/2"	5/16"	5/16"

NOTE:
1. ALL CONNECTIONS OF PRIMARY FRAMING BEAMS TO COLUMNS ARE TO DEVELOP 50% OF BEAM SHEAR CAPACITY (MIN.) U.N.O.
2. ALL FILLET WELD SIZES SHOWN ARE MINIMUM WELD SIZE, WHERE WELD SIZE SHOWN ARE SMALLER THAN AWS MINIMUM WELD SIZE, AWS MINIMUM WELD SIZE SHALL BE USED.

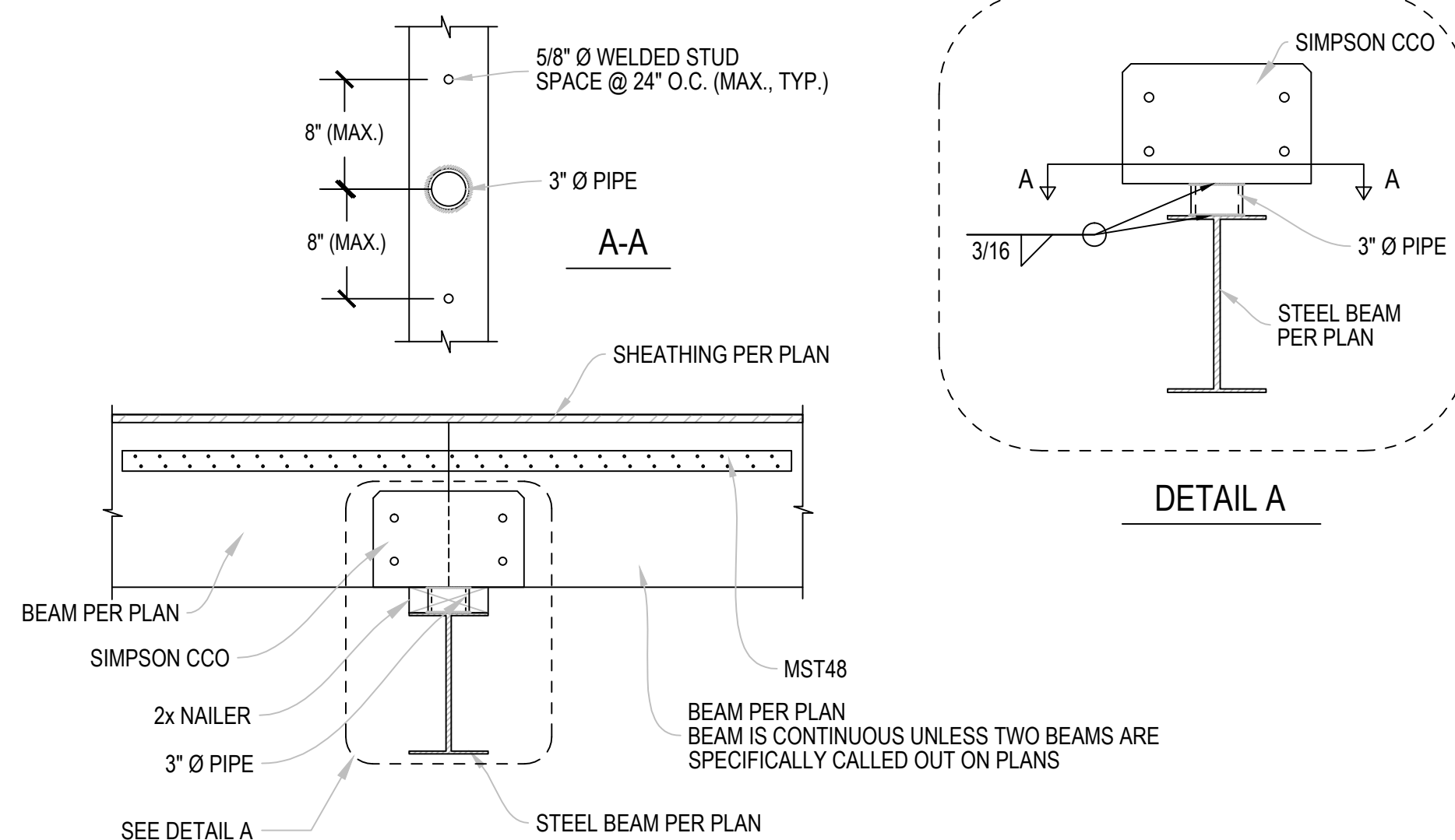
BEAM CONNECTION SCHEDULE

2



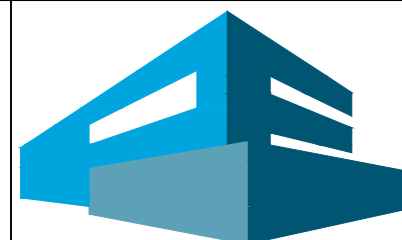
JOIST/BEAM TO STEEL BEAM CONNECTION

9



WOOD BEAM TO STEEL BEAM CONNECTIONS

3



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CONTENT

STEEL DETAILS

DRAWN BY: CR/CKP

CHECKED BY: CKP

DATE: SEPTEMBER 20, 2013

SHEET NO.

S4.5

ENGLISH INSTALL - WESTERN

Western Product Profiles

CATEGORY 1	CATEGORY 2	CATEGORY 3	CATEGORY 4	CATEGORY 5	VERSALAM®	BOISE GULUM®
BCP 5000 1.7	BCP 6000 1.7	BCP 6500 1.8	BCP 60 2.0	BCP 90 2.0	2x 2600	2x 3100
1 1/2" x 16"	1 1/2" x 16"	1 1/2" x 16"	1 1/2" x 16"	1 1/2" x 16"	5 1/2" x 16"	5 1/2" x 16"
2 1/2" x 16"	2 1/2" x 16"	2 1/2" x 16"	2 1/2" x 16"	2 1/2" x 16"	3 1/2" x 16"	3 1/2" x 16"
3 1/2" x 16"	3 1/2" x 16"	3 1/2" x 16"	3 1/2" x 16"	3 1/2" x 16"	2 1/2" x 16"	2 1/2" x 16"
4 1/2" x 16"	4 1/2" x 16"	4 1/2" x 16"	4 1/2" x 16"	4 1/2" x 16"	1 1/2" x 16"	1 1/2" x 16"
5 1/2" x 16"	5 1/2" x 16"	5 1/2" x 16"	5 1/2" x 16"	5 1/2" x 16"	1 1/2" x 16"	1 1/2" x 16"
6 1/2" x 16"	6 1/2" x 16"	6 1/2" x 16"	6 1/2" x 16"	6 1/2" x 16"	1 1/2" x 16"	1 1/2" x 16"
7 1/2" x 16"	7 1/2" x 16"	7 1/2" x 16"	7 1/2" x 16"	7 1/2" x 16"	1 1/2" x 16"	1 1/2" x 16"
8 1/2" x 16"	8 1/2" x 16"	8 1/2" x 16"	8 1/2" x 16"	8 1/2" x 16"	1 1/2" x 16"	1 1/2" x 16"
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26 1/2" x 16"	26 1/2" x 16"	26 1/2" x 16"	26 1/2" x 16"	26 1/2" x 16"	1 1/2" x 16"	1 1/2" x 16"
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58 1/2" x 16"	58 1/2" x 16"	58 1/2" x 16"	58 1/2" x 16"	58 1/2" x 16"	1 1/2" x 16"	1 1/2" x 16"
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76 1/2" x 16"	76 1/2" x 16"	76 1/2" x 16"	76 1/2" x 16"	76 1/2" x 16"	1 1/2" x 16"	1 1/2" x 16"
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78 1/2" x 16"	78 1/2" x 16"	78 1/2" x 16"	78 1/2" x 16"	78 1/2" x 16"	1 1/2" x 16"	1 1/2" x 16"
79 1/2" x 16"	79 1/2" x 16"	79 1/2" x 16"	79 1/2" x 16"	79 1/2" x 16"	1 1/2" x 16"	1 1/2" x 16"
80 1/2" x 16"	80 1/2" x 16"	80 1/2" x 16"	80 1/2" x 16"	80 1/2" x 16"	1 1/2" x 16"	1 1/2" x 16"
81 1/2" x 16"	81 1/2" x 16"	81 1/2" x 16"	81 1/2" x 16"	81 1/2" x 16"	1 1/2" x 16"	1 1/2" x 16"
82 1/2" x 16"	82 1/2" x 16"	82 1/2" x 16"	82 1/2" x 16"	82 1/2" x 16"	1 1/2" x 16"	1 1/2" x 16"
83 1/2" x 16"	83 1/2" x 16"	83 1/2" x 16"	83 1/2" x 16"	83 1/2" x 16"	1 1/2" x 16"	1 1/2" x 16"
84 1/2" x 16"	84 1/2" x 16"	84 1/2" x 16"	84 1/2" x 16"	84 1/2" x 16"	1 1/2" x 16"	1 1/2" x 16"
85 1/2" x 16"	85 1/2" x 16"	85 1/2" x 16"	85 1/2" x 16"	85 1/2" x 16"	1 1/2" x 16"	1 1/2" x 16"
86 1/2" x 16"	86 1/2" x 16"	86 1/2" x 16"	86 1/2" x 16"	86 1/2" x 16"	1 1/2" x 16"	1 1/2" x 16"
87 1/2" x 16"	87 1/2" x 16"	87 1/2" x 16"	87 1/2" x 16"	87 1/2" x 16"	1 1/2" x 16"	1 1/2" x 16"
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92 1/2" x 16"	92 1/2" x 16"	92 1/2" x 16"	92 1/2" x 16"	92 1/2" x 16"	1 1/2" x 16"	1 1/2" x 16"
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97 1/2" x 16"	97 1/2" x 16"	97 1/2" x 16"	97 1/2" x 16"	97 1/2" x 16"	1 1/2" x 16"	1 1/2" x 16"
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Product may not be available. Check with supplier or Boise representative for availability.
Increase in category is an increase in span length. See the BCP Residential Floor Span Tables below.
For further information, refer to VERSA-LAM® 1.7 2650 Header Guide.

Residential Floor Span Tables

About Floor Performance

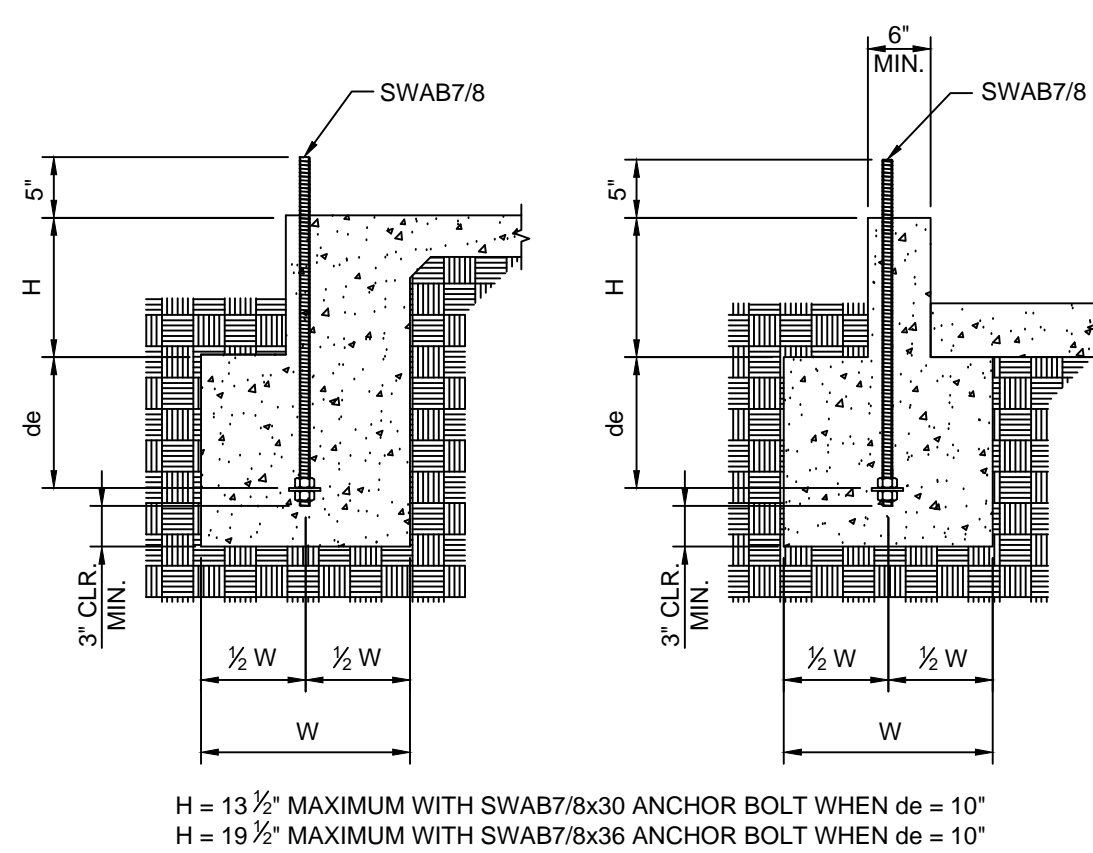
Homeowner's expectations and opinions vary greatly due to the subjective nature of rating a new floor. Communication with the ultimate user and to determine their expectation is critical. Installation is usually the cause of most complaints. Installing lateral bridging may help, however, squeaks may occur if not installed properly. Spacing the joists closer together does little to affect the perception of floor performance. The most common methods used to increase the performance and

reduce vibration of wood floor systems is to increase the joist depth, limit joist deflection, glue and screw a joist, tongue-and-groove subfloor, install the joists vertically plumb with level-bearing supports, and install a direct attached ceiling to the bottom flanges of the joists.

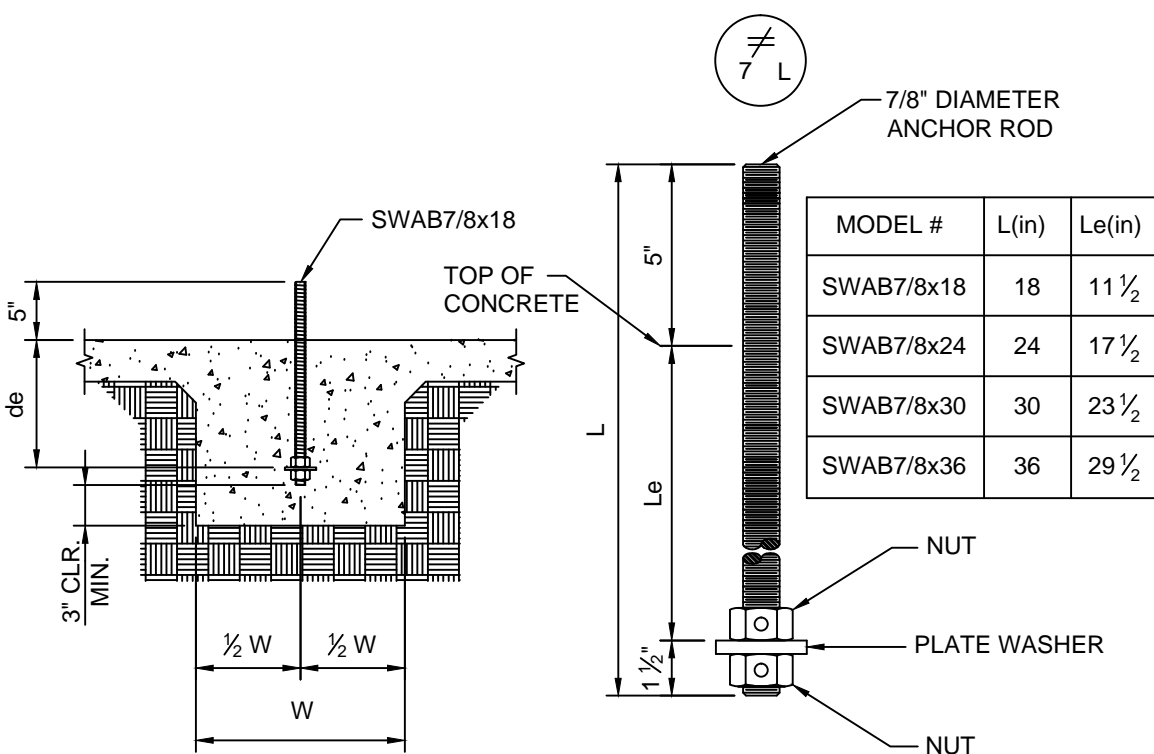
The floor plans details listed below offer three different performance options, based on performance and

*** THREE STAR ***

**** FOUR STAR ****

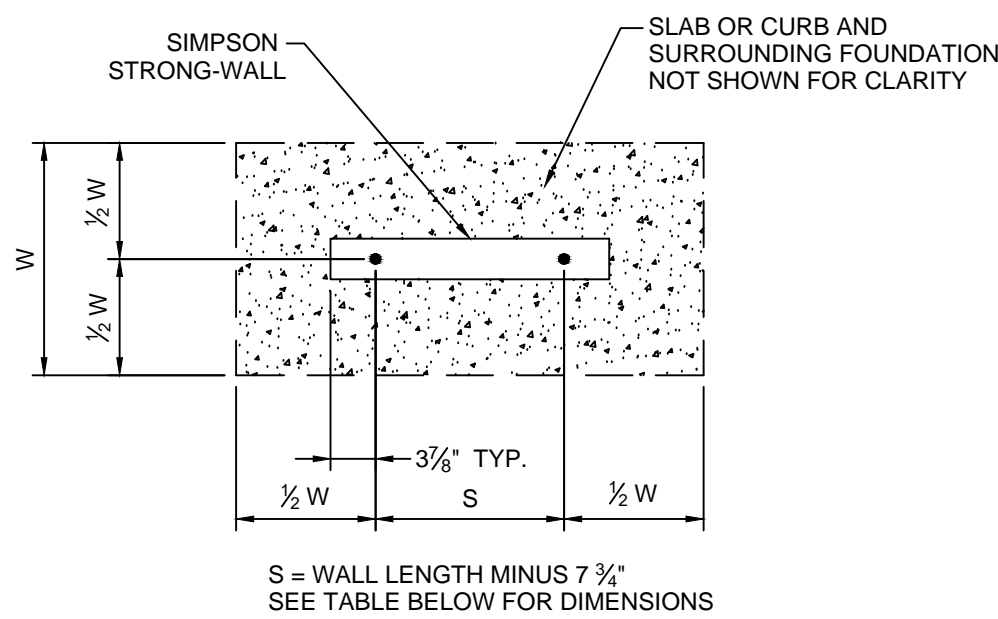


SLAB ON GRADE FOUNDATION CURB OR STEMWALL FOUNDATION



INTERIOR FOUNDATION SWAB7/8 ANCHOR BOLT

NOTES:
1. SEE 2-SW1 FOR DIMENSIONS AND ADDITIONAL NOTES.

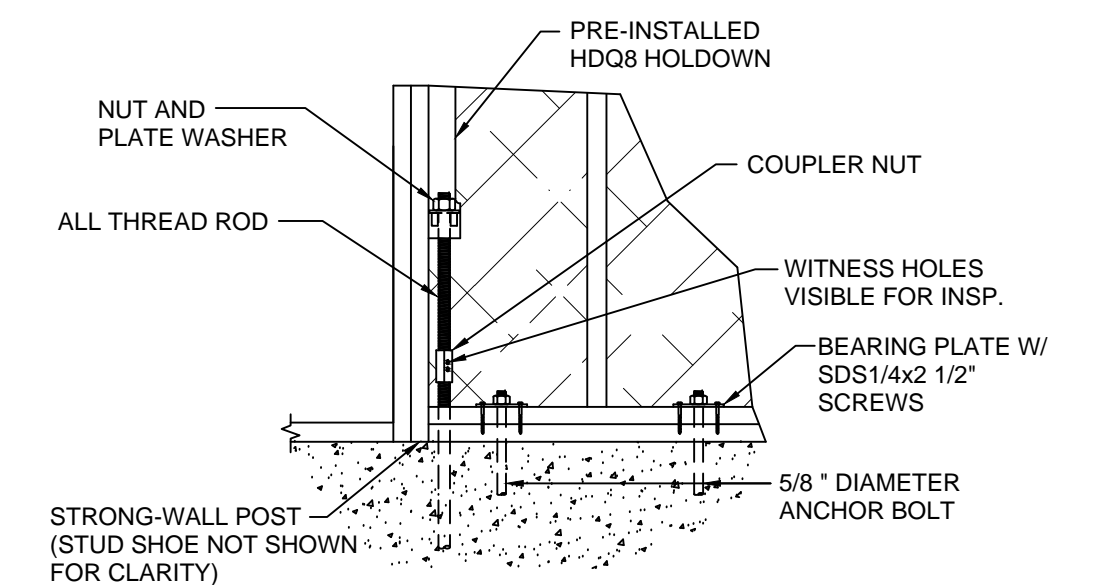


FOUNDATION PLAN VIEW

FOUNDATION DIMENSIONS FOR STRONG-WALL ANCHORAGE				
	CONDITION	ASD ALLOWABLE UPLIFT (lbs)	W (in)	de (in)
SEISMIC	CRACKED	13,000	29	10
	UNCRAKED	13,000	21	7
WIND	CRACKED	6,500	16	6
		9,700	21	7
	UNCRAKED	6,600	14	6
		9,600	18	6
		13,000	22	8

- NOTES:
- ANCHORAGE DESIGNS CONFORM TO ACI 318 APPENDIX D AND ASSUME MINIMUM $f_c=2,500$ PSI CONCRETE, ASTM A307 OR ASTM F1554, GRADE 36 ANCHOR RODS AND NO SUPPLEMENTARY REINFORCEMENT. HIGH STRENGTH ANCHORAGE DESIGN BY OTHERS WHEN REQUIRED.
 - SEISMIC INDICATES SEISMIC DESIGN CATEGORY C THROUGH F. DETACHED 1 AND 2 FAMILY DWELLINGS IN SDC C MAY USE WIND ANCHORAGE SOLUTIONS. SEISMIC ANCHORAGE DESIGNS CONFORM TO ACI 318-05 SECTION D.3.3.4.
 - WIND INCLUDES SEISMIC DESIGN CATEGORY A AND B.
 - FOUNDATION DIMENSIONS ARE FOR ANCHORAGE ONLY. FOUNDATION DESIGN (SIZE AND REINFORCEMENT) BY OTHERS. THE REGISTERED DESIGN PROFESSIONAL MAY SPECIFY ALTERNATE EMBEDMENT, FOOTING SIZE OR ANCHOR BOLT.
 - FOR ANCHORAGE SOLUTIONS USING SSTB, SEE ICC-ES ESR-2611.

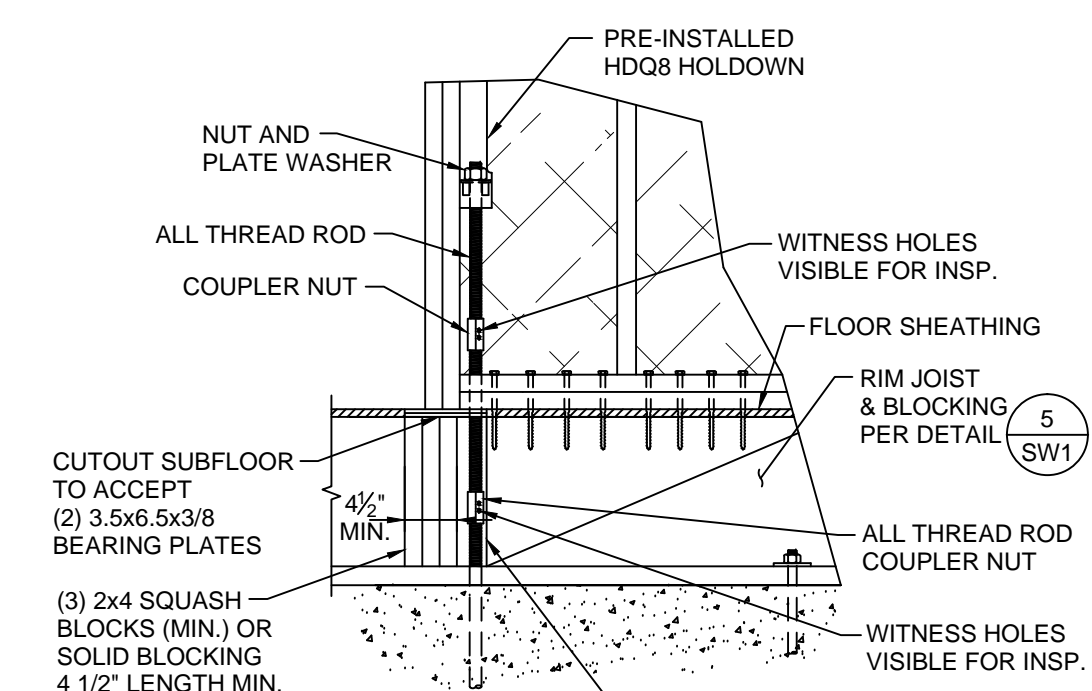
SEE SHEET SW2 FOR STACKED STRONG-WALL REQUIREMENTS.



STANDARD WALL SILL

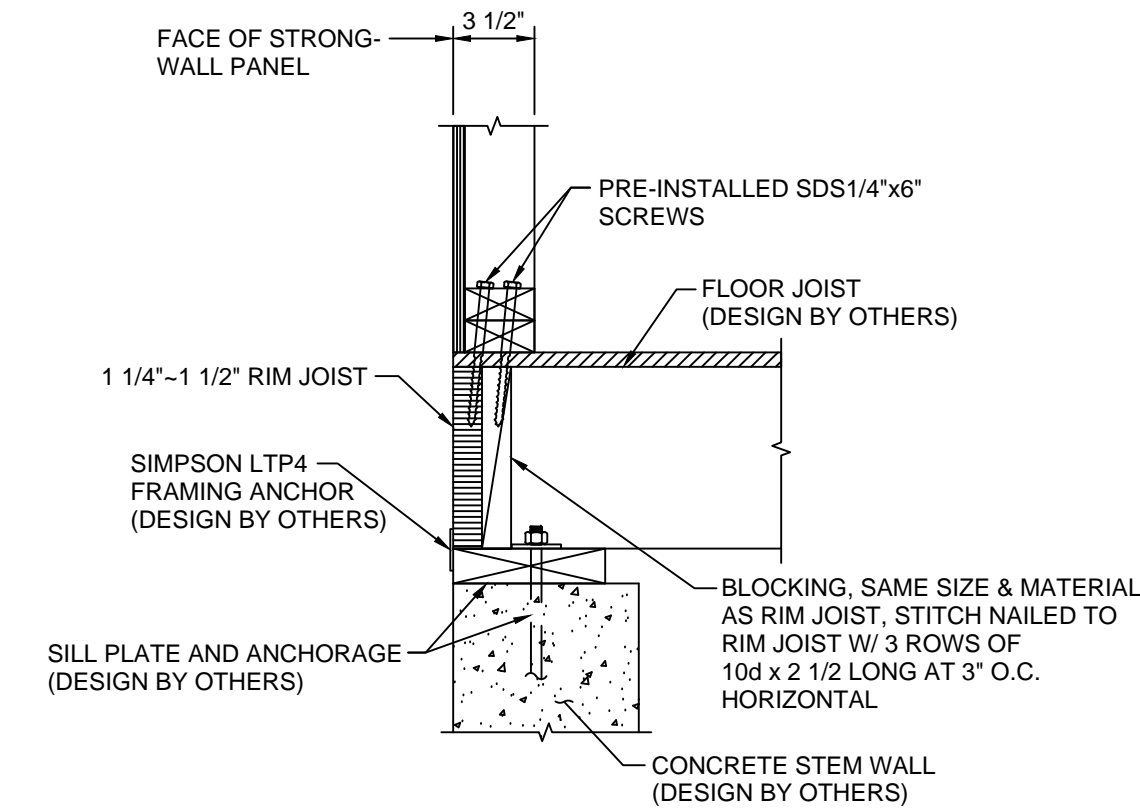
3

SEE SHEET SW2 FOR STACKED STRONG-WALL REQUIREMENTS.



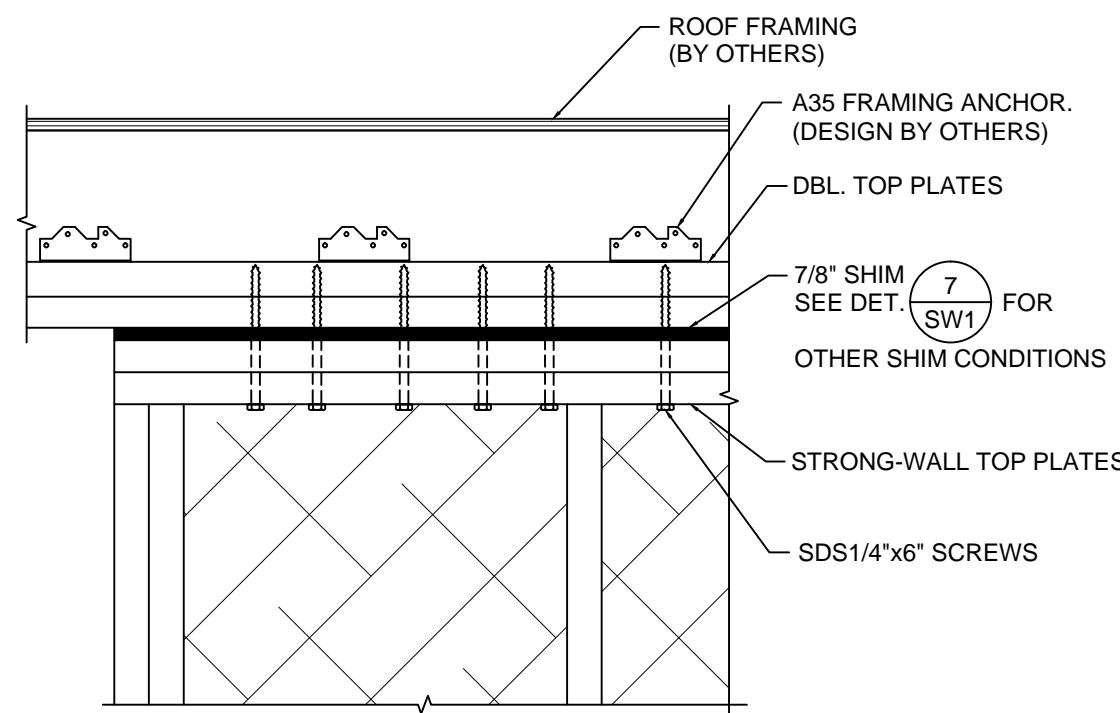
RAISED FLOOR WALL SILL

4



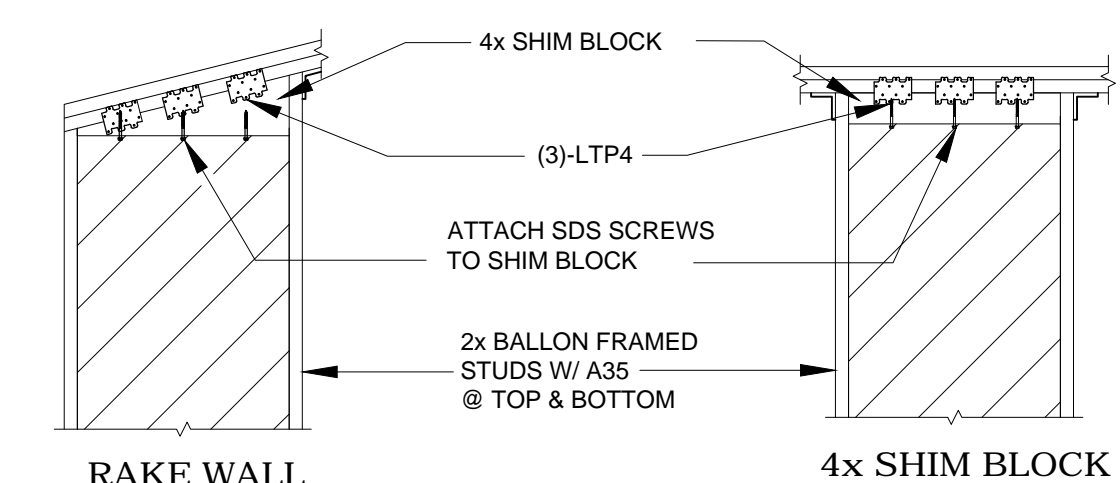
RAISED FLOOR WALL SECTION

5



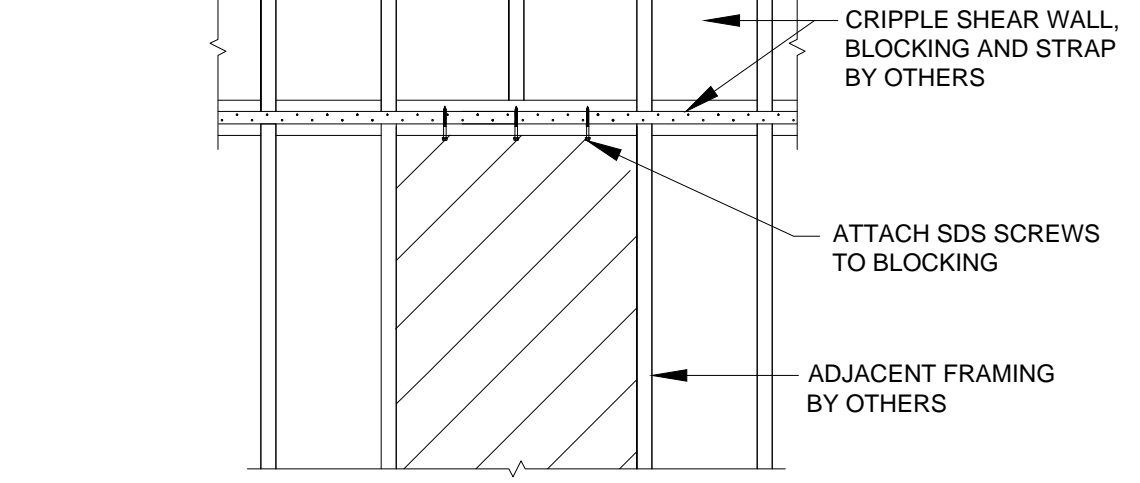
TOP PLATE CONNECTION

6



SHIM BLOCK ON STD. & RF WALLS

7



CRIPPLE WALL ON STD & RF WALLS

8

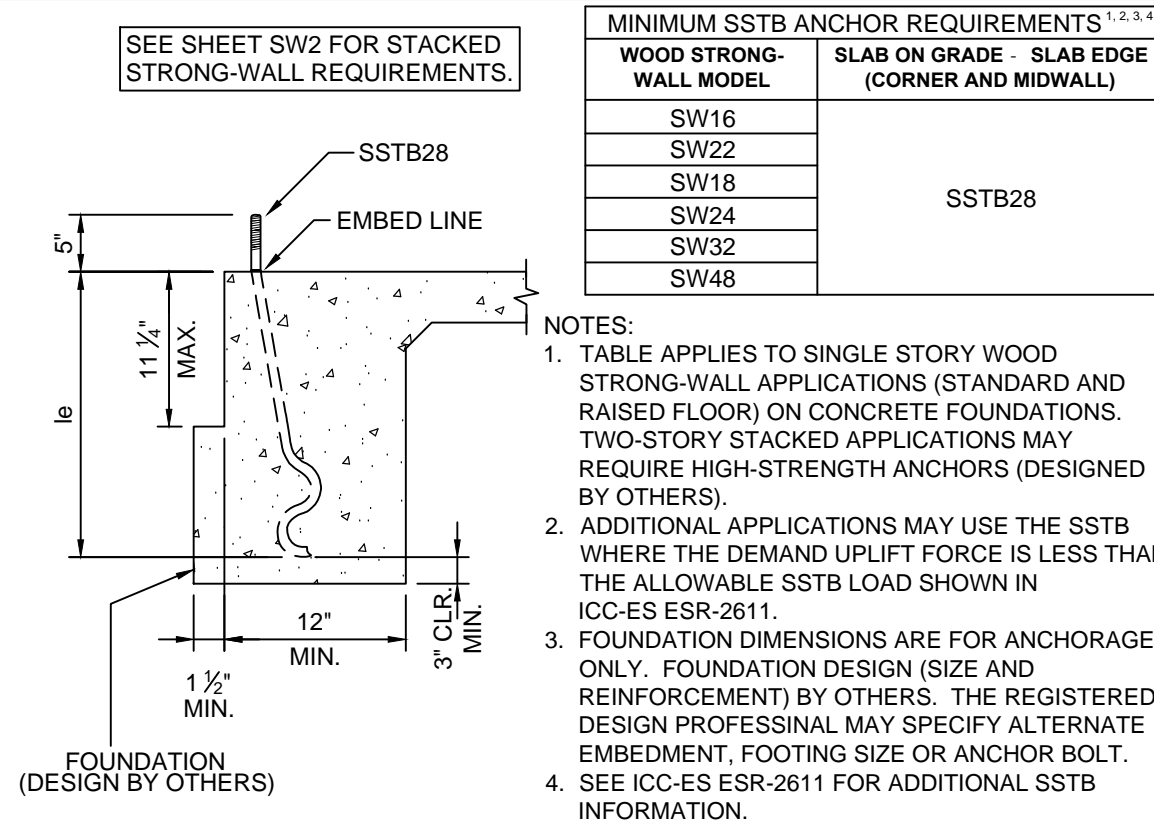
ANCHORAGE - TYPICAL SECTIONS

ANCHORAGE SCHEDULE

TOP PLATE CONNECTION

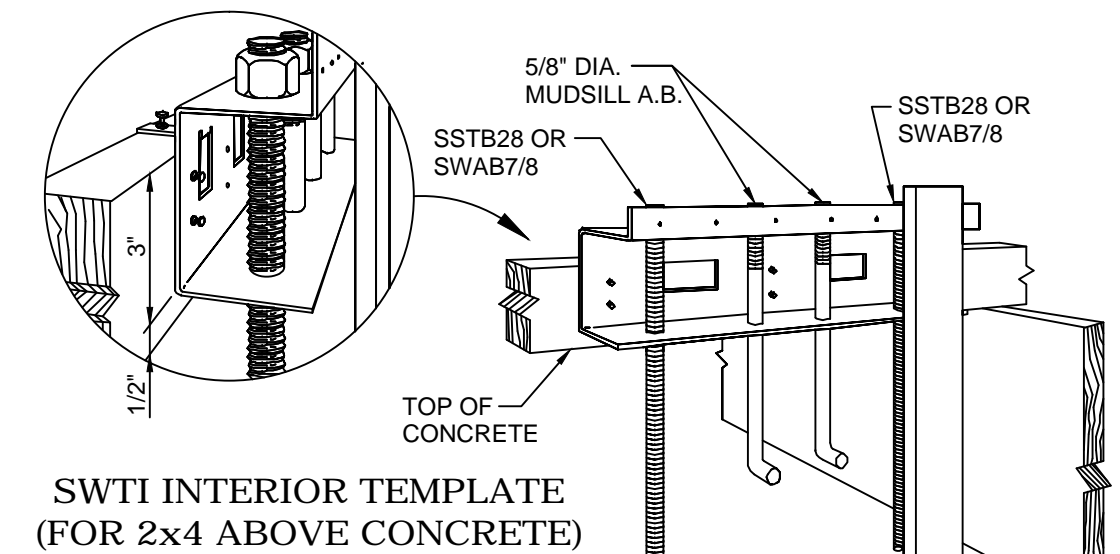
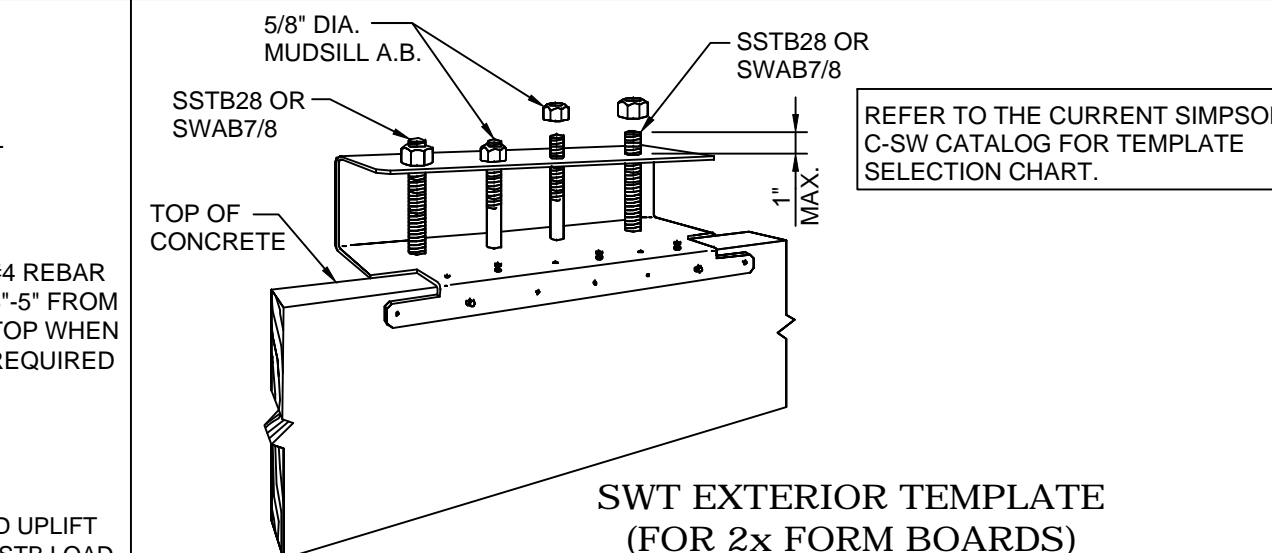
SHIM BLOCK ON STD. & RF WALLS

CRIPPLE WALL ON STD & RF WALLS

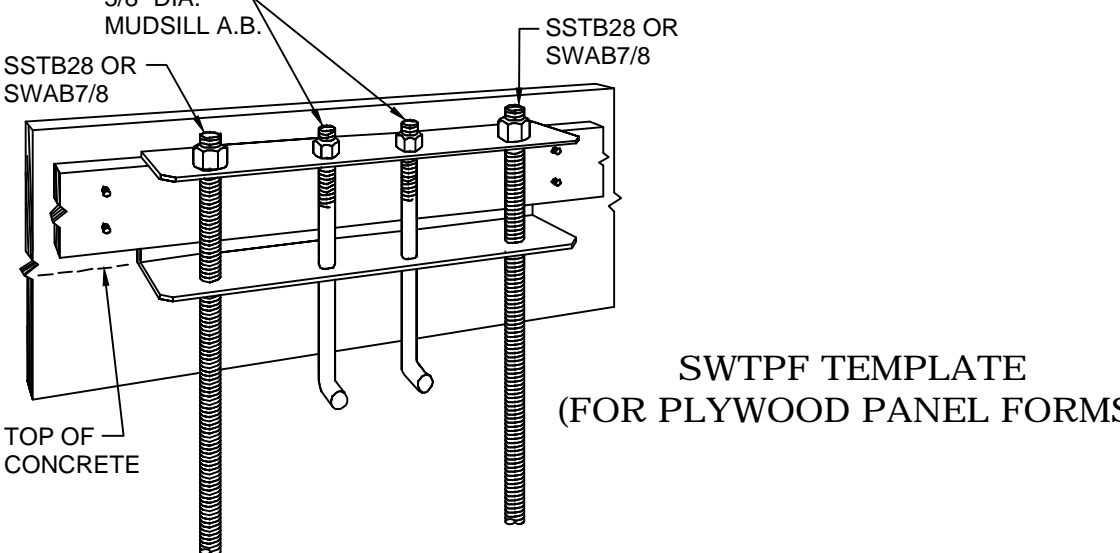


SSTB ANCHORAGE (SLAB ON GRADE)

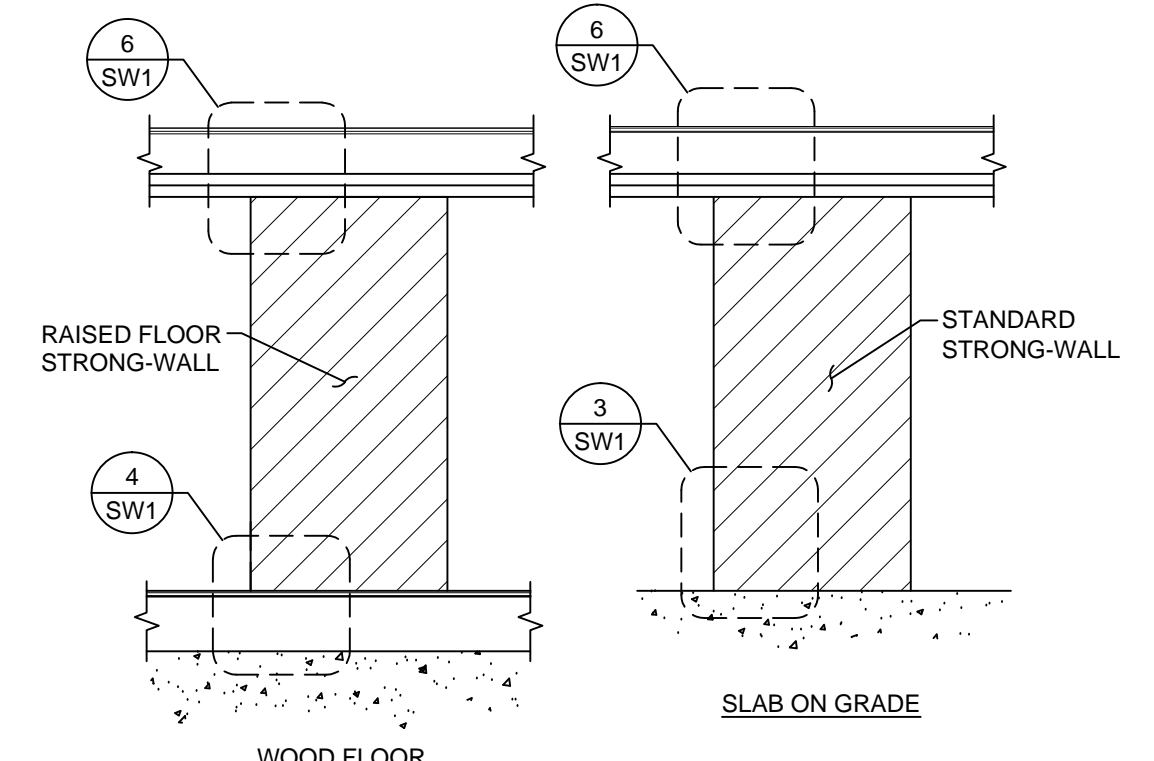
SSTB ANCHORAGE (CONC. STEM WALL)



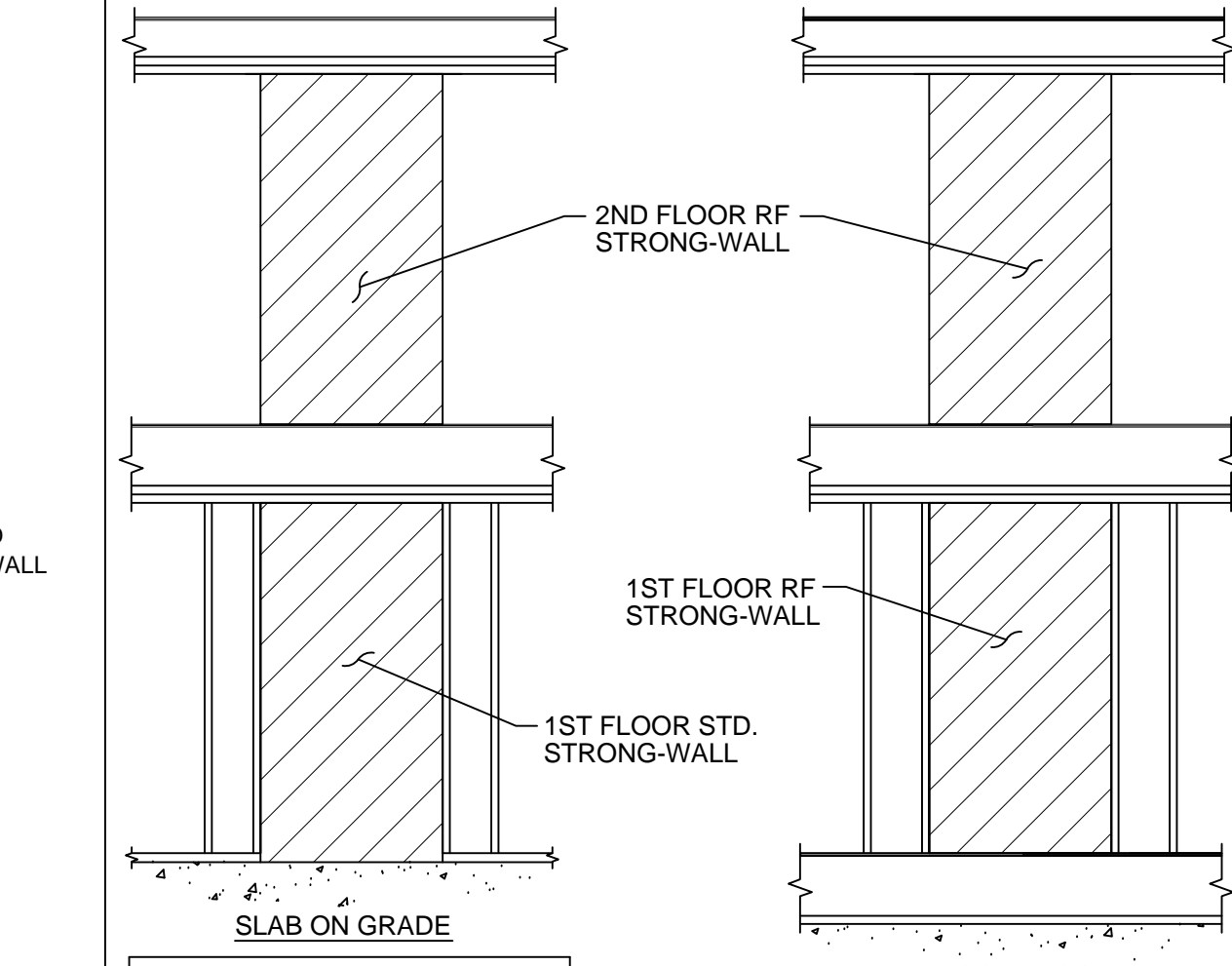
SWTI INTERIOR TEMPLATE (FOR 2x4 ABOVE CONCRETE)



SWTPF TEMPLATE (FOR PLYWOOD PANEL FORMS)

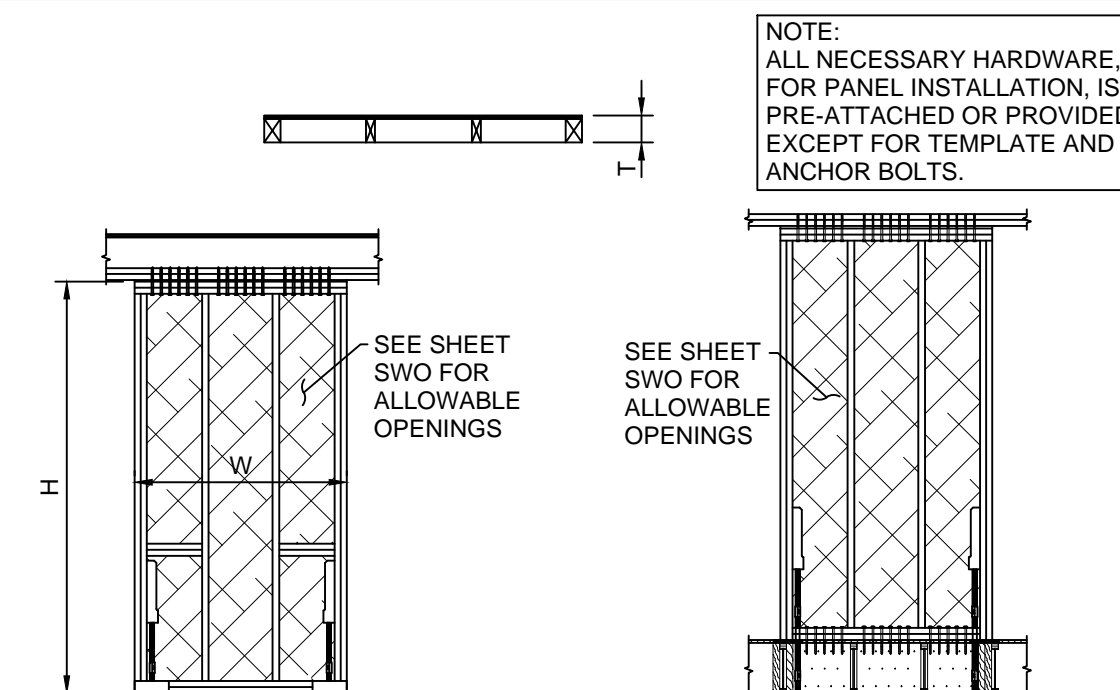


SINGLE STORY STRONG-WALL®



STACKED STRONG-WALL®

STRONG-WALL® TEMPLATES



STANDARD STRONG-WALL RAISED FLOOR STRONG-WALL

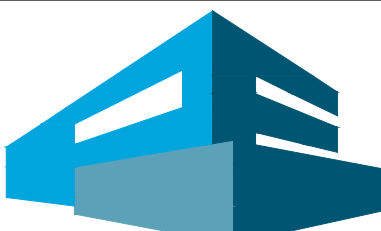
STANDARD STRONG-WALL MODELS						
MODEL SET UP	W (IN)	H (IN)	T (IN)	NUMBER OF MUDDSILL ANCHORS	HOLDOWN ANCHOR BOLTS	ASSEMBLED WALL WEIGHT (lbs)
SW18x8	18	93 1/4	3 1/2	2-5/8"Ø	2-7/8"Ø	85
SW24x8	24	93 1/4	3 1/2	2-5/8"Ø	2-7/8"Ø	91
SW32x8	32	93 1/4	3 1/2	2-5/8"Ø	2-7/8"Ø	116
SW48x8	48	93 1/4	3 1/2	3-5/8"Ø	2-7/8"Ø	149
SW18x9	18	105 1/4	3 1/2	2-5/8"Ø	2-7/8"Ø	94
SW24x9	24	105 1/4	3 1/2	2-5/8"Ø	2-7/8"Ø	101
SW32x9	32	105 1/4	3 1/2	2-5/8"Ø	2-7/8"Ø	128
SW48x9	48	105 1/4	3 1/2	3-5/8"Ø	2-7/8"Ø	165
SW24x10	24	117 1/4	3 1/2	2-5/8"Ø	2-7/8"Ø	111
SW32x10	32	117 1/4	3 1/2	2-5/8"Ø	2-7/8"Ø	134
SW48x10	48	117 1/4	3 1/2	3-5/8"Ø	2-7/8"Ø	171
SW24x12x6	24	141 1/4	5 1/2	2-5/8"Ø	2-7/8"Ø	187
SW32x12x6	32	141 1/4	5 1/2	2-5/8"Ø	2-7/8"Ø	201
SW48x12x6	48	141 1/4	5 1/2	3-5/8"Ø	2-7/8"Ø	256

RAISED FLOOR STRONG-WALL MODELS						
MODEL SET UP	W (IN)	H (IN)	T (IN)	NUMBER OF FASTENERS IN BOTTOM OF WALL	NUMBER OF FASTENERS IN TOP OF WALL	HOLDOWN ANCHOR BOLTS
SW18x8-RF	18	93 1/4	3 1/2	13-SDS 1/4x6	9-SDS 1/4x6	2-7/8"Ø
SW24x8-RF	24	93 1/4	3 1/2	16-SDS 1/4x6	12-SDS 1/4x6	2-7/8"Ø
SW32x8-RF	32	93 1/4	3 1/2	20-SDS 1/4x6	16-SDS 1/4x6	2-7/8"Ø
SW48x8-RF	48	93 1/4	3 1/2	28-SDS 1/4x6	24-SDS 1/4x6	2-7/8"Ø
SW18x9-RF	18	105 1/4	3 1/2	13-SDS 1/4x6	9-SDS 1/4x6	2-7/8"Ø
SW24x9-RF	24	105 1/4	3 1/2	16-SDS 1/4x6	12-SDS 1/4x6	2-7/8"Ø
SW32x9-RF	32	105 1/4	3 1/2	20-SDS 1/4x6	16-SDS 1/4x6	2-7/8"Ø
SW48x9-RF	48	105 1/4	3 1/2	28-SDS 1/4x6	24-SDS 1/4x6	2-7/8"Ø
SW24x10-RF	24	117 1/4	3 1/2	16-SDS 1/4x6	12-SDS 1/4x6	2-7/8"Ø
SW32x10-RF	32	117 1/4	3 1/2	20-SDS 1/4x6	16-SDS 1/4x6	2-7/8"Ø
SW48x10-RF	48	117 1/4	3 1/2	28-SDS 1/4x6	24-SDS 1/4x6	2-7/8"Ø

WALL SPECIFICATIONS

NOTES

- STRONG-WALL® SHEARWALL IS MANUFACTURED AND TRADEMARKED BY "SIMPSON STRONG-TIE COMPANY INC.", HOME OFFICE: 5956 W. LAS POSITAS BOULEVARD, PLEASANTON, CA 94588 TEL: (800) 999-5099, FAX: (925) 875-0826 "SIMPSON STRONG-TIE COMPANY INC." IS AN ISO 9001 REGISTERED COMPANY.
- INSTALLATION OF PRODUCT SHALL BE DONE IN STRICT CONFORMANCE TO THESE DRAWINGS AND THE STRONG-WALL® INSTALLATION GUIDE. MODIFICATIONS TO THIS PRODUCT AND ASSOCIATED SYSTEMS OR CHANGES IN THE INSTALLATION METHODS SHOWN ON THESE DRAWINGS AND THE INSTALLATION GUIDE SHOULD ONLY BE MADE BY A QUALIFIED ARCHITECT, CIVIL OR STRUCTURAL ENGINEER. THE PERFORMANCE OF SUCH MODIFIED PRODUCTS OR ALTERED INSTALLATION PROCEDURES IS THE SOLE RESPONSIBILITY OF THE DESIGNER. REFER TO ICC-ES ESR-1267 FOR FURTHER INFORMATION.
- THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS, CONDITIONS, ELEVATIONS, ETC. PRIOR TO INSTALLATION OF ANY COMPONENTS FOR THE STRONG-WALL® SYSTEM. IF ANY DISCREPANCIES ARE FOUND, THEY SHALL BE BROUGHT TO THE ATTENTION OF THE PROJECT ARCHITECT, PROJECT ENGINEER OR BUILDING DESIGNER FOR CLARIFICATION PRIOR TO CONSTRUCTION.
- THE CONTRACTOR SHALL VERIFY THE POSITION OF THE STRONG-WALL IN RELATION TO THE REST OF THE BUILDING SYSTEM AS SHOWN ON THE PROJECT DRAWINGS.
- USE OF THIS PRODUCT IS SUBJECT TO THE APPROVAL OF THE LOCAL BUILDING OFFICIAL.
- THE BUILDING STRUCTURE SHALL BE DESIGNED IN ACCORDANCE WITH THE LATEST ADOPTED VERSION OF THE BUILDING CODE AND ANY OTHER LOCAL, STATE OR FEDERAL REQUIREMENTS THAT MAY APPLY. VERIFY DESIGN REQUIREMENTS WITH THE LOCAL BUILDING DEPARTMENT.
- THIS PRODUCT IS PART OF THE OVERALL LATERAL FORCE RESISTING SYSTEM OF THE STRUCTURE. DESIGN OF THE BUILDING'S LATERAL FORCE RESISTING SYSTEM, INCLUDING A COMPLETE LOAD PATH NECESSARY TO TRANSFER LATERAL FORCES FROM THE STRUCTURE TO THE GROUND, IS THE RESPONSIBILITY OF THE DESIGNER.
- SIMPSON STRONG-TIE COMPANY INC. RESERVES THE RIGHT TO CHANGE SPECIFICATIONS, DESIGNS AND MODELS WITHOUT NOTICE OR LIABILITY FOR SUCH CHANGES.
- ALL HARDWARE CALLED OUT IS SIMPSON STRONG-TIE®



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MEBUST RESIDENCE
5448 TAFT AVENUE
LA JOLLA, CA 92037
PROJECT #: 2013-0070

DESIGNER:

REVISIONS

REV	REVISION	DATE

DRAWING STATUS

- ☐ INFORMATION DRAWINGS. (NOT FOR CONSTRUCTION)
- ☐ PRELIMINARY DRAWINGS. (NOT FOR CONSTRUCTION)
- ☐ CHECK SET. (NOT FOR CONSTRUCTION)
- ☐ PLAN CHECK DRAWINGS.
- ☐ CONSTRUCTION DOCUMENTS.
- ☐ OTHER AS-BUILT

CONTENT

DRAWN BY: CR/CKP
CHECKED BY: CKP
DATE: SEPTEMBER 20, 2013
SHEET NO.

SD2